

BHP Billiton's Iron Ore Port Hedland Outer Harbour Development

Tidal Flushing of West Creek

Revision – 3

Draft Report 09/10/09

Prepared for: SKM & BHPBIO



Document control form

Document draft	Originated by	Edit & review	Authorized for release by	Date
Draft 1 - Issued for internal review	Marc Zapata	Scott Langtry	Marc Zapata	16/09/09
Draft 2 – Issued for client review	Marc Zapata		Marc Zapata	21/09/09
Draft 3 – Issued for client review	Marc Zapata		Marc Zapata	15/10/09

Document name: APASA report on West Creek tidal inundation V3.docx

APASA Project Number: J0059

APASA Project Manager: Marc Zapata

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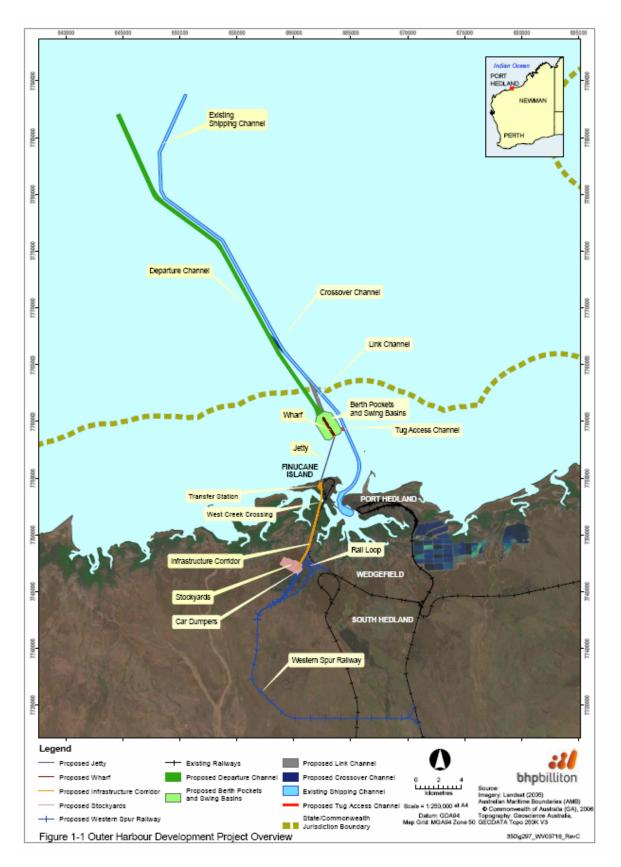
1 INTRODUCTION

BHP Billiton Iron Ore proposes to expand their iron ore operations in the Pilbara by developing a new port. This proposed development is known as the Port Hedland Outer Harbour Development (Outer Harbour Development). The proposed Outer Harbour Development is located on the coast near Port Hedland in the Pilbara region of Western Australia (*Figure 1-1*). Sinclair Knight Merz (SKM) is undertaking an environmental impact assessment of the project's proposed construction and operation activities. This impact assessment is required to facilitate environmental approvals for the project under State and Commonwealth legislation.

Of direct concern are the effects of installation of an infrastructure corridor (West Creek crossing, Figure 1-2) on the tidal water movements of West Creek and surrounding region, including potential impacts upon the flora and fauna of mangrove areas. The potential impacts of the development are required to be assessed in detail.

Asia-Pacific Applied Science Associates (APASA) has conducted hydrodynamic modelling of the proposed West Creek crossing to assess the impact of the development on West Creek and the adjacent mangrove areas. The Environmental Fluid Dynamics Code (EFDC) threedimensional numerical model has been applied for this study. A comparative analysis was conducted using a simulation of the existing configuration of West Creek as the baseline and comparing this to simulations of the proposed creek crossing.

This report presents the findings of the comparative three-dimensional numerical modelling and discusses the likely impacts of the proposed developments on the currents and tidal levels of the system.



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Figure 1-1: Outer Harbour Development project overview.

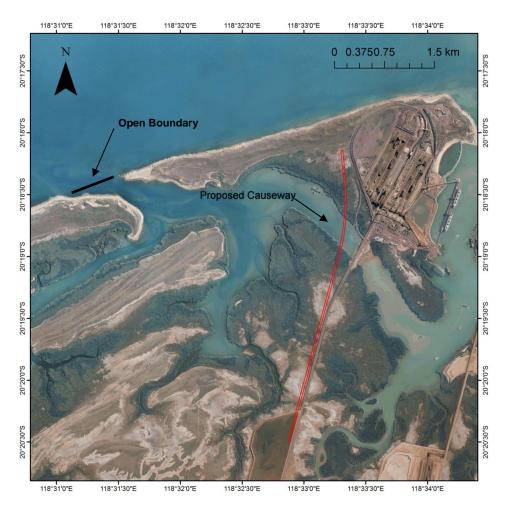


Figure 1-2: Location of proposed West Creek crossing (red).

1.1 Scope

The scope of work included:

- Sourcing and formatting data required for the hydrodynamic models. This includes bathymetry, boundary data and specification of drag coefficients.
- Developing a hydrodynamic model representing the present configuration of West Creek (Base Model) to represent tidal and circulation patterns. A high-resolution wetting and drying model (where the coastline extent varies with the tidal level) was required for the West Creek area due to the extensive area of intertidal mudflats and the large effect these have on water circulation and tidal exchange. The West Creek model was combined with a wider area hydrodynamic model that represented circulation over the coastal waters adjacent to West Creek.
- Validating the hydrodynamic model against available pressure gauges and drogue experiments.

- Modelling of a 29-day operational period. This standard tidal analysis period covered a full spring/neap lunar tidal cycle. The simulations were conducted for the largest tidal variations available in the existing hydrodynamic record, ensuring that a large inundation cycle was sampled and allowing accurate representation of the submergence characteristics.
- Producing submergence times for locations in the inner model domain, which were spatially mapped to show the existing inundation patterns. Submergence curves were plotted at specific reference points of interest.
- Modifying the existing scenario to include the proposed creek crossing. Specifications
 of culverts and other connecting structures were used to specify water exchange
 across the structures. Once prepared and tested, the model was operated over each
 Scenario domain for the same 29 day tidal periods and with common environmental
 conditions to form comparative data sets.
- Differences in the predictions of submergence time, current vectors and inundation from the existing case were highlighted by spatially mapping any differences between each scenario and the existing case over the full domain. Mapping of inundation time changes at the Mean Sea Level (MSL) and Mean High Water Springs (MHWS) water levels will help to define the full extent of any potential impacts. Comparison of the submergence curves at reference points will also serve to highlight changes expected at these locations.

1.2 Study Benchmarks

Water depths and levels presented in this report are with respect to Lowest Astronomical Tide (LAT) unless otherwise stated and are in units of meters.

Positions are satellite derived from the Global Positioning System using the GDA 1994 datum, unless stated otherwise. Position units are in meters.

All units are in International Standard of Units (SI).

2 HYDRODYNAMIC MODEL DEVELOPMENT

Hydrodynamic model development and validation was conducted for the proposed crossing over West Creek. The preliminary hydrodynamic model has been forced by tide and wind only.

2.1 Background and Methods

The domain of the hydrodynamic model needed to encompass the total area required to accurately represent tidal exchange within West Creek and be large enough to properly capture forcing conditions affecting the study site. Fine detail was required within West Creek, to resolve the complex bathymetry. It was important to represent the inundation and drying of the mangrove areas, creeks and intertidal mudflats to correctly represent tidal exchange with the flooding and ebbing tides.

To efficiently meet the dual requirements for a large domain, with detailed representation of wetting and drying within West Creek, two hydrodynamic models were developed:

- An offshore model, developed using the ASA HYDROMAP model; and
- A fine-scale harbour model, developed using the Virginia Institute of Marine Science (VIMS) EFDC model.

The fine-scale harbour model was nested within the wider scale model of the coastal waters, providing tidal boundary conditions predicted by the offshore model, to ensure matching of tidal phase and magnitude. The details of each of the hydrodynamic models are discussed below.

2.2 Offshore Model

2.2.1 Model Description

Three-dimensional currents were generated using a large scale ocean/coastal circulation model, HYDROMAP, which has been successfully applied in many regions around the world (Isaji et al., 2001, Zigic et al., 2003).

HYDROMAP is formulated to describe the flow field that is locally induced when tides and winds are the most important forcing sources. The HYDROMAP model solves for the threedimensional Navier–Stokes equations under an incompressible flow assumption for Newtonian fluids with the inclusion of the Coriolis terms. The equations are solved using a Cartesian coordinate system. The momentum equations retain the non-linear acceleration terms. The dominant frictional forces are assumed to be the vertical diffusion of momentum due to wind shear at the water surface, and friction at the sea bed. The horizontal diffusion of momentum is neglected. Also, vertical accelerations are neglected and the pressure distribution is hydrostatic. These assumptions are valid for modelling the wind and tidal driven circulation over the area of interest.

The hydrodynamic equations of motion are solved using a finite difference grid in the horizontal (x,y) coordinates and the Galerkin method in the vertical (z) coordinate. The numerical solution methodology follows that of Davies (1977 a, b) with further developments

for model efficiency by Owen (1980) and Gordon (1982). The finite difference scheme is fully explicit and uses a space staggered grid scheme with vertical velocities specified at the center of each grid cell, and the horizontal velocities given at the grid cell faces. To allow for time steps larger than the Courant-Friedrichs-Lewy (CFL) condition, the free-surface elevation is treated separately from the internal, three-dimensional flow variables. The scheme has been extensively tested through comparison with analytic solutions.

A detailed presentation of the model and applications can be found in Owen (1980) and Isaji and Spaulding (1984).

2.2.2 Domain and Bathymetry

A large domain was created for the offshore hydrodynamic model (*Figure 2-1*), spanning 131 km in an east west direction (Longitude 117.95 to 119.2E) and 83 km in a north south direction (Latitude 19.72 to 20.47S).



Figure 2-1: Locality map and Hydromap domain shown by the red box.

The hydrodynamic grid used a nested sub-gridding approach, which allowed for higher resolution around areas of specific interest or with complex bathymetry. Computational cells were square, with sizes varied from 1600 m at the outer boundary to 100 m at the finest scale. *Figure 2-2* shows examples of the variable grid resolution employed in the model. The final grid consisted of approximately 55,000 computational water cells.

Bathymetry used in the model was obtained from multiple sources. For the near-shore, high resolution areas, bathymetric data from a Light Detection and Ranging (LiDAR) survey were applied. Areas further offshore were described using 250 m-scale gridded bathymetry developed by Geoscience Australia from hydrographic surveys. *Figure 2-3* shows the bathymetry used in the grid at four different zoom ratios, again highlighting the high resolution in the inshore regions.

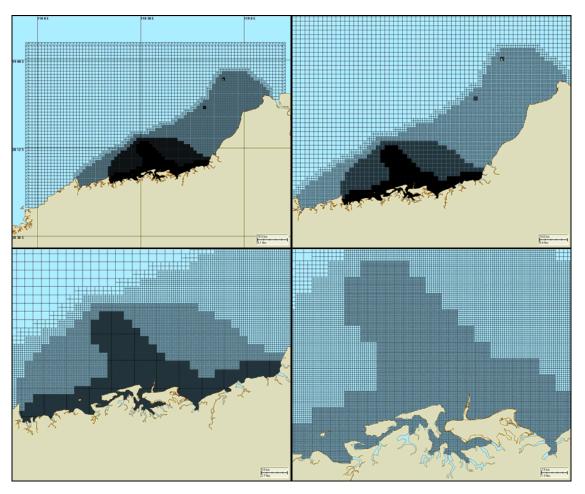


Figure 2-2: Hydrodynamic grid used in HYDROMAP at four different zoom levels indicating the transition from coarse to fine resolution within the domain.

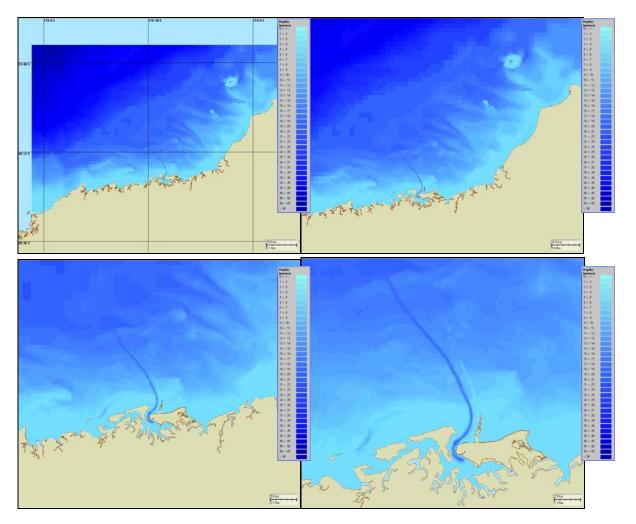


Figure 2-3: Bathymetry integrated within the hydrodynamic grid shown at four zoom levels.

2.2.3 Tidal Forcing

Boundary forcing was provided by calculation of tidal elevations. Astronomical tides were included on all the open boundaries, based on the tidal constituents provided by the latest Topex Poseidon global tidal set (TPX0 version 7.0; source: NOAA). The eight largest tidal constituents were used in the simulations: K2, S2, M2, N2, K1, P1, O1 and Q1. These constituents were selected as they account for the majority of the tidal amplitude. The values of the constituents varied over the latitude and longitude of the model boundary.

2.2.4 Wind Forcing

Two wind records were available for use in this study:

- 1. Long term record at Port Hedland Airport spanning 1993 to 2008, provided by the Bureau of Meteorology (BOM).
- 2. Data collected offshore from Port Hedland (Beacon 15) spanning from May 2000 to August 2007, provided by Metocean Engineers.

Both data sets were inspected for quality assurance/quality control (QA/QC) purposes and were found to contain numerous gaps and erroneous data. While the Beacon 15 data was considered superior in terms of the location of the anemometer (being located offshore) the length of quality records was shorter. The BOM data was of a consistently high quality between 1995 to 2007 inclusive, providing 13 years of reliable data coinciding with the availability of wave hindcast data for the offshore waters, which were required for the wave modelling component.

To optimise the wind data, the BOM records were compared and adjusted to match the distribution of measured conditions at Beacon 15. The wind directions were highly consistent between the two data sets and were not adjusted, however a factor of 1.1 was applied to the measured BOM wind speed to match the distribution of the Beacon 15 data.

Figure 2-4 shows the result of the adjustment process for cumulative occurrence of wind speed. The final result is shown by the green line which confirms a strong match of wind speed over the distribution, particularly for the median and higher values that have more influence on water currents than lighter winds.

Figure 2-5 presents comparative wind roses for the two data sets, showing that the directionality at the locations is consistent. The main variation observed at the two sites is the reduction in the sea breeze component at Beacon 15, due to the distance offshore. In nearshore waters, the sea breeze will be a factor, and it is considered an important inclusion in the modelling.

Overall, the adjusted data provides the most suitable long term data set for modelling.

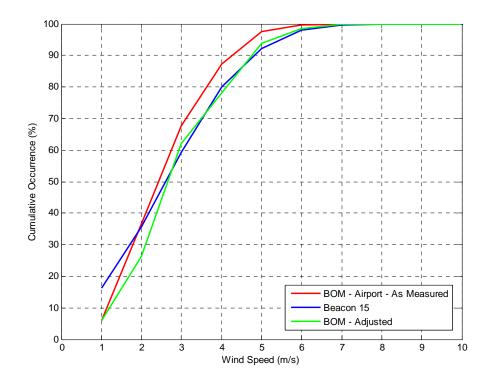


Figure 2-4: Comparison of the cumulative occurrence distribution of wind speed at Beacon 15 (left) and BOM Port Hedland Airport (right).

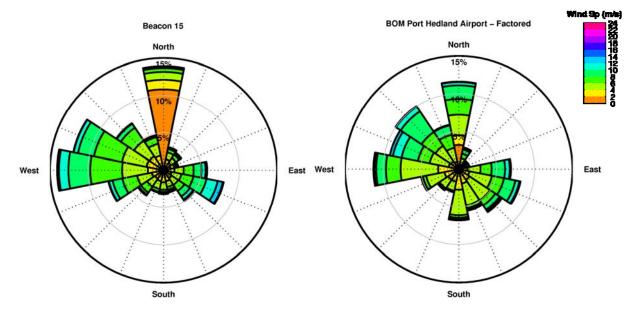


Figure 2-5: Comparative wind roses for Beacon 15 (left) and BOM Port Hedland Airport (right).

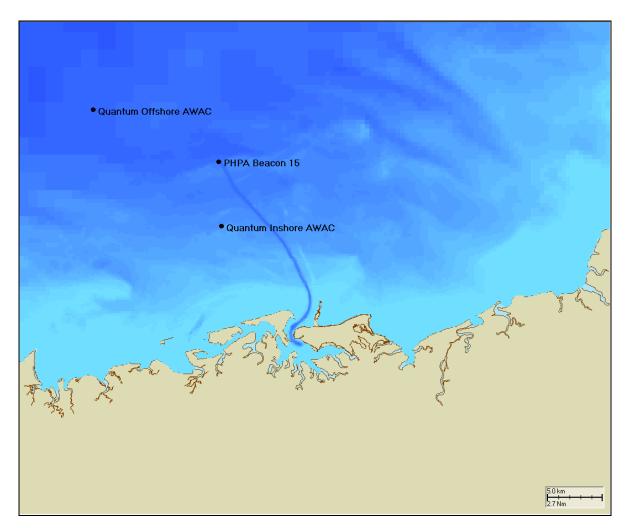
2.2.5 Validation

Current data available from three Acoustic Wave and Current (AWAC) sites (*Figure 2-6*) were used for validating the predictions of the hydrodynamic model. The model was run for a number of periods to allow validation at each of the Quantum AWAC sites for summer, winter and transitional seasons (Table 2-1).

Validation Sites	Period
Beacon 15	February 2006 (Summer)
	April 2006 (Autumn transitional)
	June 2006 (Winter)
	October 2006 (Spring transitional)
Quantum Inshore and Offshore AWAC	February 2008 (Summer)
	April 2008 (Autumn transitional)
	June 2008 (Winter)
	October 2008 (Spring transitional)

Table 2-1: Periods run for each validation set.

The current simulations were conducted once for each period, hence the data from the Quantum Inshore and Offshore AWAC sites were extracted from the same model run for a test of the models stability over space. No adjustments were made to any model coefficients between the periods. Thus, the comparisons also tested the temporal stability and accuracy of the model for that one configuration.



*Note: the shipping channel defined by the clear blue line representing the locally deeper area.

Figure 2-6 Locations of the three current measurement locations.

Model results were assessed in a number of ways. Firstly, surface elevations were compared at the three AWAC sites (*Figure 2-7*). A linear regression analysis was carried out on all data for the periods, yielding a correlation (R^2) value of 0.98 at both AWAC sites, indicating that astronomical tidal variations predicted by the model explained 98% of the elevations observed. This indicates that the model was accurately propagating the tidal waves over the study area using the tidal boundary data for the eight largest tidal constituents extrapolated from the Topex-Poseidon global tidal set. The result indicates that other factors, such as atmospheric pressure variations, explain only a very small proportion of the variations in water elevation at the sites and could be justifiably ignored.

To further validate the model performance, measured and predicted current speeds at the surface, mid-depth and bottom of the water column (seabed) were compared over each period for the two Quantum AWAC sites and the Beacon 15 site. These comparisons are provided in Appendix A (Quantum Inshore AWAC), Appendix B (Quantum Offshore AWAC) and Appendix C (Beacon 15). Figure 2-8 to Figure 2-13 show summary comparisons for the seabed currents over objectively selected summer and winter month samples. Seabed currents are used for summary purposes because sediments, will tend to concentrate within and be transported by currents near the seabed.

Results have been illustrated as rose plots, time-series comparisons of the velocity components, and scatter plots of current speed and direction. Multiple methods were employed due to the inherent biases associated with any one type of analysis or plot. For example, the rose plots have been generated with 11.25 degree directional bins that can create possible discrepancies relating to bin allocation near the defined bin edges. То investigate whether there is any significant discrepancy between the measured and predicted results that may be apparent in the rose plots, the scatter and time series comparison plots may be consulted. It should be noted that positive values in the time-series plots represent currents flowing towards the north and east for the respective current velocity components. The major use of the north-south velocity plots is to confirm that the model is correctly representing the magnitude and timing in the onshore-offshore tidal flow and that there is no loss of match over time. Matches in the east - west current component, which will be primarily due to wind-induced longshore drift, can be observed by comparison of the rose and scatter plots.

The comparisons indicated that the current amplitudes and vertical current profile were predicted to a good level of accuracy by the model for all three locations, indicating that the model has suitable spatial stability. The scatter and rose plots indicated that the magnitude and range in current speeds was well represented. Similarly, the direction of the currents was simulated suitably well by the model for each layer, but with an offset in the tidal current direction between the model and measured data of +5 to +10 degrees. Tidal currents were observed to flow towards the northwest during the ebb and to the southeast during the flood at all three sites, consistent with the observations. The measured data indicated a marginally larger east-west component than that predicted in the model. If this outcome is general for all parts of the model, the comparisons indicate that the model may marginally underestimate migration to the east or west, at least in the deeper parts of the model. The east-west component in the modelled current was a response to the wind data specified over the domain. It was recognised that the wind directions had a small onshore-offshore bias due to the application of a spatially constant wind, using wind directions observed at Port Hedland (with magnitudes alone scaled up to match the offshore measurements). Currents will tend to steer shoreward on approach to the land. Hence, the small shoreward discrepancy observable in the current direction at the offshore sites was considered to be a small and acceptable bias. The effect of this small bias on the sediment modelling would be a minor overstatement of migration onshore for the deeper sections of the model and would be conservative in terms of the concentrations predicted over time over shallower sections of the model domain.

Comparisons for the Beacon 15 data show a higher correlation between measured and modelled data at the seabed than at the surface (*Figure 2-12*, *Figure 2-13*; Appendix C). The surface measurements at Beacon 15 show a number of erratic amplifications in current speed which are likely to be measurement faults. The slight deviation of the model-predicted current direction from the measurements is again apparent at this site, again relating to a moderately lower east-west current velocity being generated in the model.

Further statistical analysis of the model skill at replicating the hydrodynamics was conducted through the use of the Index of Agreement (IOA), presented in Wilmott (1981). This index, commonly used to assess model skill, is calculated as:

$$IOA = 1 - \frac{\sum |X_{model} - X_{obs}|^2}{\sum (|X_{model} - \overline{X_{obs}}| + |X_{obs} - \overline{X_{obs}}|)^2}$$

In this equation, *X* represents the variable being compared. *X* under a bar indicated the mean over time for that variable. A perfect agreement can be said to exist between the model and field observations if the index gives a measure of one, and complete disagreement will produce an index measure of zero (Wilmott 1981). This IOA was applied to each of the current velocity components. The IOA was consistently above 0.9, indicative of a strong correlation between the measured and modelled datasets. For the Quantum Inshore and Offshore AWAC sites, the model produced the highest IOA during February and October, with IOA's consistently above 0.92. The Beacon 15 site indicated strong correlations during February and April, with IOA's above 0.94, reducing during June and October to 0.91.

The results of the combined analysis indicated that the predicted current data was fit for the purpose of calculating the transport and resuspension of sediments.

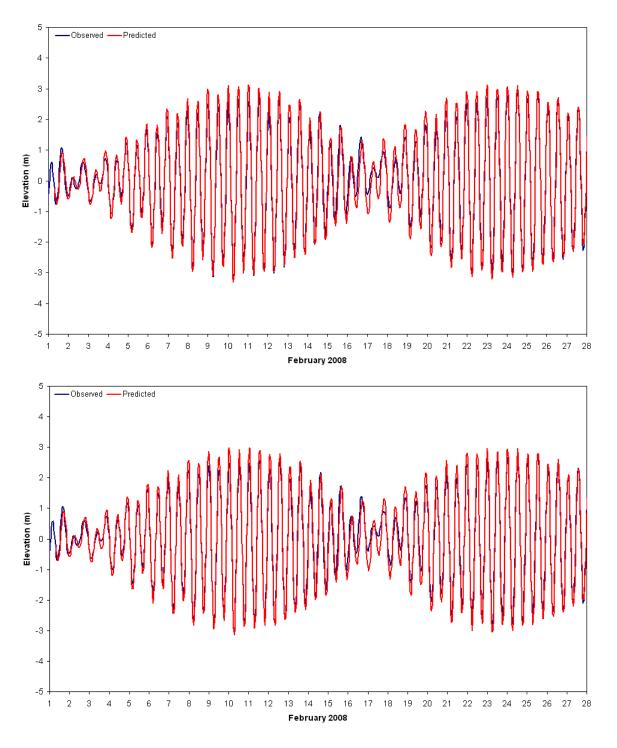
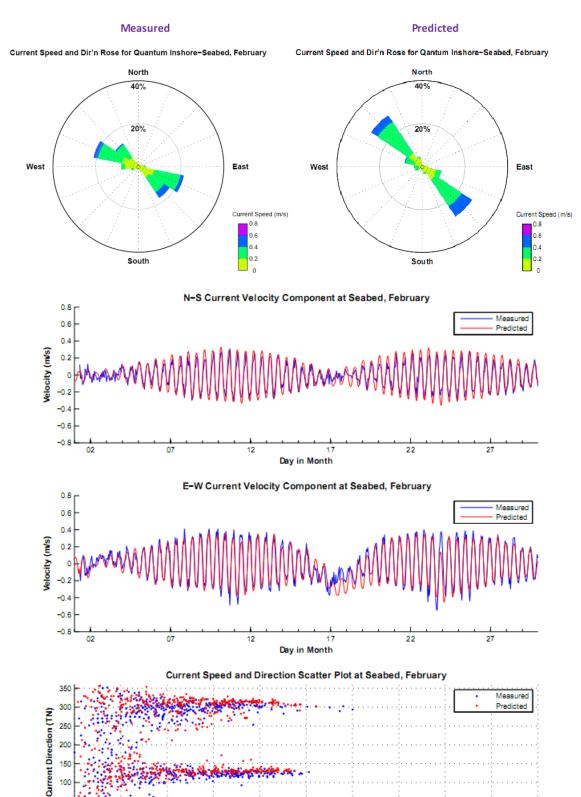


Figure 2-7 Comparative surface elevation during February 2008 for Quantum Inshore AWAC site (top) and Quantum Offshore AWAC site (bottom).



Note: positive values in this graph represent currents flowing to the north.

0.2

0.3

0.4

Figure 2-8 Comparison between current magnitude normalised for the north direction during a representative summer month (February 2008) for the Quantum Inshore AWAC site at the seabed.

0.5

Current Speed (m/s)

0.6

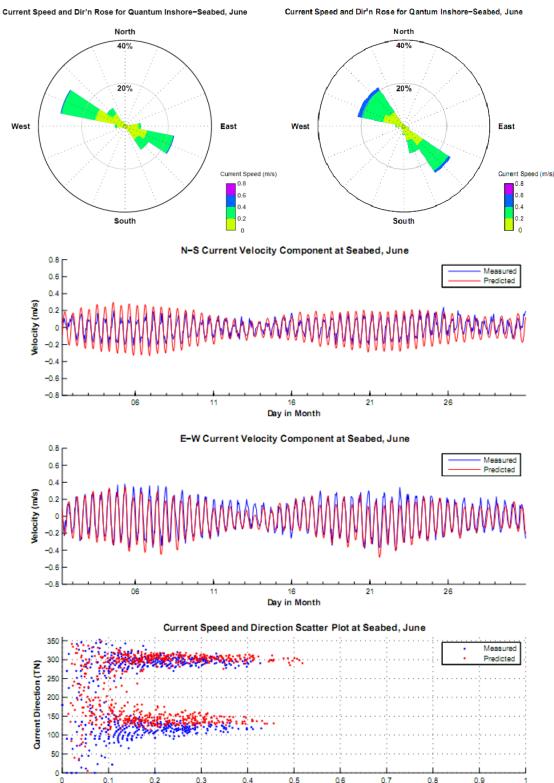
0.7

0.8

0.9

0.1





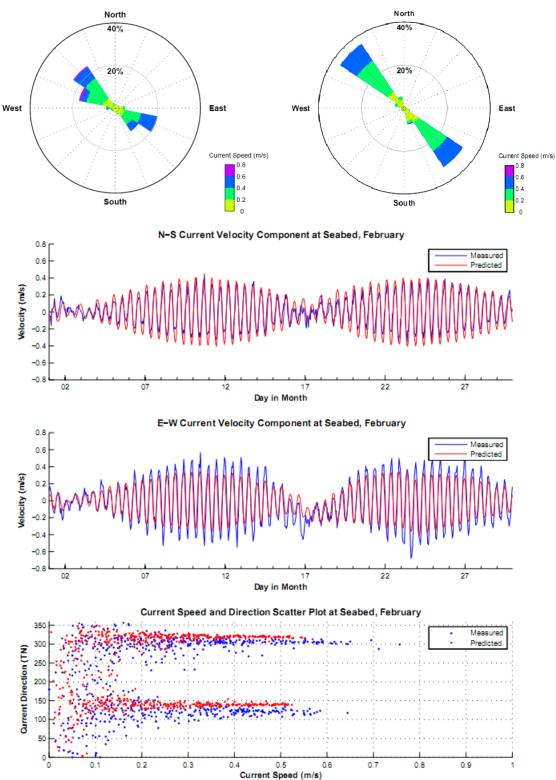
Note: positive values in this graph represent currents flowing to the north.

Figure 2-9 Comparison between current magnitude normalized for the north direction during a representative winter month (June 2008) for the Quantum Inshore AWAC site at the seabed.

Current Speed (m/s)

Current Speed and Dir'n Rose for Qantum Offshore-Seabed, February

Predicted
Current Speed and Dir'n Rose for Qantum Offshore-Seabed, February

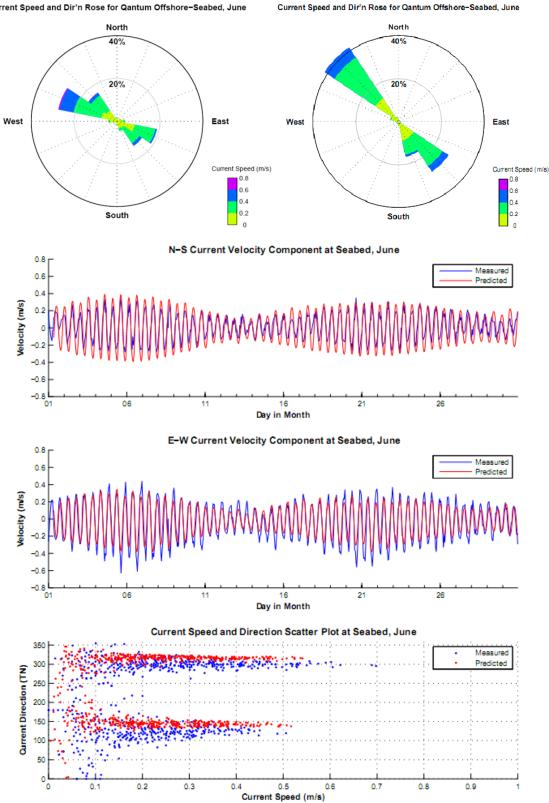


Note: positive values in this graph represent currents flowing to the north.

Figure 2-10 Comparison between current magnitude normalized for the north direction during a representative summer month (February 2008) for the Quantum Offshore AWAC site at the seabed.

Current Speed and Dir'n Rose for Qantum Offshore-Seabed, June

Predicted



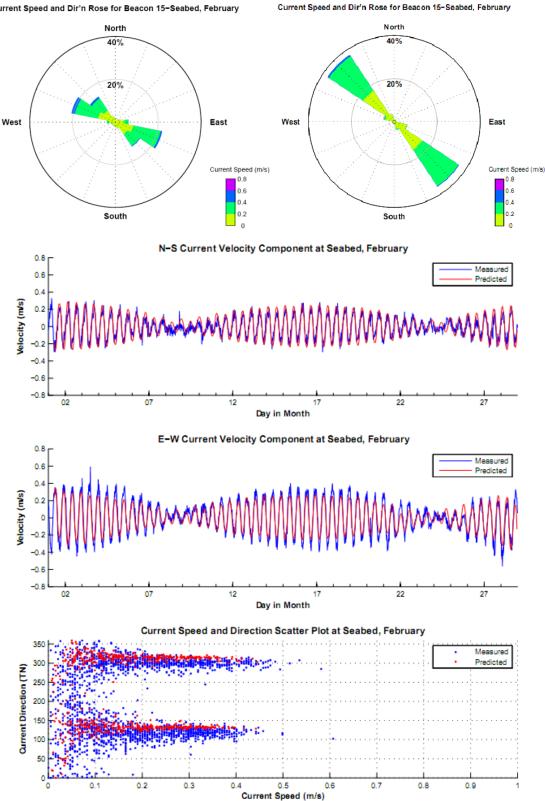
Note: positive values in this graph represent currents flowing to the north.

Figure 2-11 Comparison between current magnitude normalized for the north direction during a representative winter month (June 2008) for the Quantum Offshore AWAC site at the seabed.

Predicted

Measured

Current Speed and Dir'n Rose for Beacon 15-Seabed, February

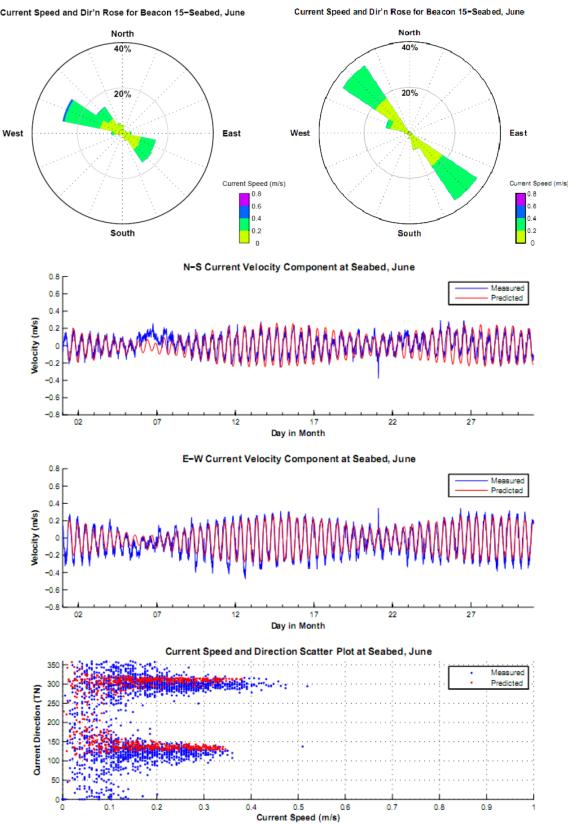


Note: positive values in this graph represent currents flowing to the north.

Figure 2-12 Comparison between current magnitude normalized for the north direction during a representative summer month (February 2006) for the Beacon 15 site at the seabed.

Current Speed and Dir'n Rose for Beacon 15-Seabed, June

Predicted



Note: positive values in this graph represent currents flowing to the north.

Figure 2-13 Comparison between current magnitude normalized for the north direction during a representative winter month (June 2006) for the Beacon 15 site at the Seabed.

2.3 West Creek Model

2.3.1 Model Description

Due to the necessity of accurately simulating inundation and drying of the tidal flats and mangroves in the vicinity of West Creek, the EFDC model was used in wetting and drying mode. EFDC has been widely used in the United States for a variety of environmental assessments, and has gained credibility with the US Environmental Protection Agency (EPA) and other regional authorities, as well as the Western Australian EPA through numerous studies in Western Australia.

The EFDC model was developed in the early 1990's, with development continuing to the present day. EFDC offers both regular Cartesian grids, and a curvilinear orthogonal system, allowing a continuous representation of features such as channels, coastlines and bottom contours. The model has a sigma, or stretched, grid in the vertical that allows a constant proportionate resolution throughout the domain. EFDC has been applied previously by the authors of this report for analysis of the hydrodynamics and dredge fate within Port Hedland Harbour for other similar projects that gained environmental approval (for example, BHP Billiton Iron Ore Nelson Point Dredging).

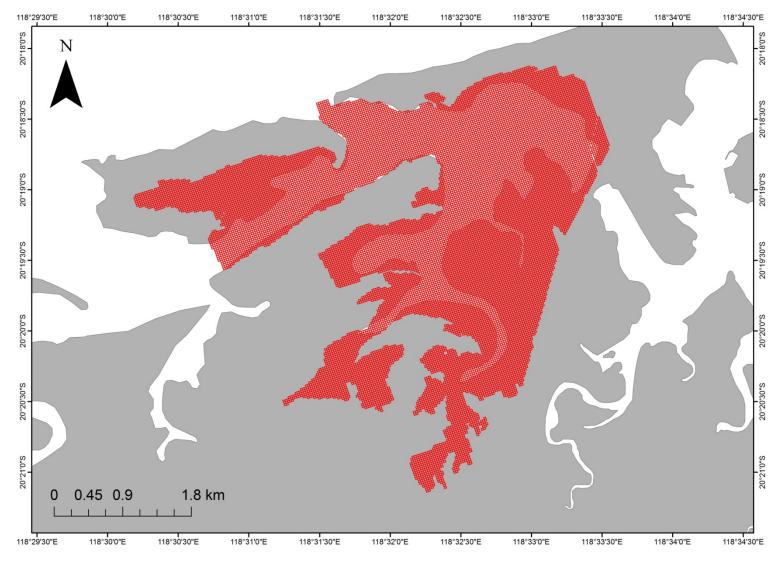
The EFDC model allows forcing by tide, wind, density and discharges, making it an ideal choice for this study. Computationally it is very efficient over small domains; however, the relatively large dispersion zone anticipated from early work suggested that an alternative model would be required for the "offshore" zone, as discussed above.

2.3.2 Domain and Bathymetry

The domain was selected to extend only to the West Creek entrance (*Figure 2-14*). Tidal conditions, including elevations and currents were matched between the two models at the model boundary.

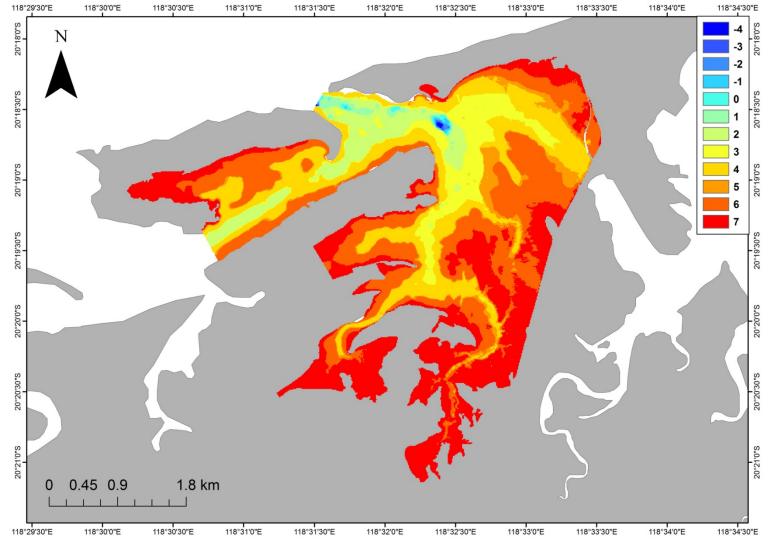
The model was constructed on a rotated, regular, Cartesian grid, with a computational cell size of 25 m x 25 m. Bathymetric data used to define the three-dimensional shape of the seabed within the model was obtained from multiple sources to optimise coverage and resolution. For certain near-shore areas within West Creek, bathymetric data was acquired by BHP Billiton Iron Ore from an air-borne LiDAR survey. LiDAR is an optical remote sensing technology that measures properties of scattered light and can be applied in shallow water to obtain depth estimates.

The data was supplied at a horizontal resolution of 5 m and with a vertical resolution that ranged with the depth, due to increasing interference of turbidity and light scattering with increasing depth (10's of cm in the shallows and degrading to a few metres at the deeper offshore extent). The LiDAR data did not cover the full domain and was augmented by field measurements carried out using a depth sounder. However, because of the incomplete coverage used to generate the data, the accuracy of the data was low over the mangrove areas, sand banks and mudflats; hence interpretation from aerial imagery was required in some areas beyond the extent of the LiDAR data. The resultant model bathymetry is show in Figure 2-15.



Note: Coordinates are in MGA94.

Figure 2-14: EFDC model domain.



Note: Coordinates are in MGA94.

Figure 2-15: EFDC model bathymetry.

Land areas above mean sea level, but below mean high water spring tides within the model domain, were assumed to be mangrove areas, and these correlated well with mapping based on available aerial photography. These areas were prescribed a higher bottom friction coefficient to account for the variable flow retardation due to the vegetation mass. APASA note that this a standard practice for the modelling of inundation within mangrove areas.

It should be noted that although the best available data was used for the study, it was generally lacking in many areas. This included the mangrove areas and particularly the mangrove creek zones.

2.3.3 Tidal Forcing

The harbour model was forced using tidal boundary data extracted directly from the offshore HYDROMAP model. This enabled the tidal phases to be matched ensuring that the two model data sets could be integrated seamlessly.

2.3.4 Wind Forcing

For consistency, the same wind data set used to drive the offshore model was used for the harbour domain. Although this represented a marginal increase in wind speeds (factor of 1.1) based on the Port Hedland Airport BOM data, the wind had negligible impact on circulation within the harbour model domain, which is almost exclusively driven by the tide.

2.3.5 Validation

2.3.5.1 Drogue Validation

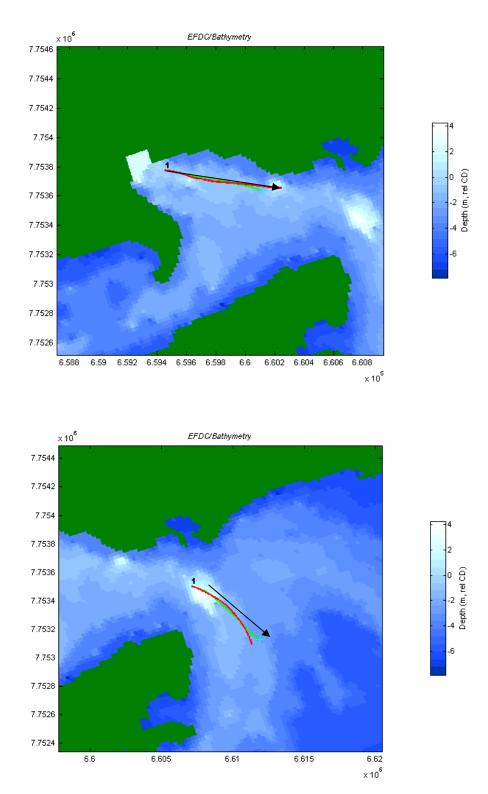
A number of drogue experiments were undertaken by SKM within West Creek from 17 July 2009 to 20 July 2009 to validate current predictions in this area. In total, five drogue experiments of variable duration were used in the model validation (Table 2-2). Experiments on the 17 July 2009 were carried out during a flood (spring) tide while experiments on the 20 July 2009 occurred during the ebbing tide.

In order to reproduce the drogue movements, EFDC was configured to reproduce currents of the same time intervals and used as input to a Lagrangian particle model (EFDC Drogue). EFDC Drogue was used to track particles with transport due to currents (from EFDC). The results of the particle track modelling are presented in Figure 2-16 and Figure 2-17.

Figure 2-16 shows the modelling results for drogue releases carried out on the 17 July 2009 while Figure 2-17 shows drogue releases on the 20 July 2009. From the comparison of the corresponding drogue and particle tracks it clearly follows that the model effectively simulated current speed and directions during the both periods.

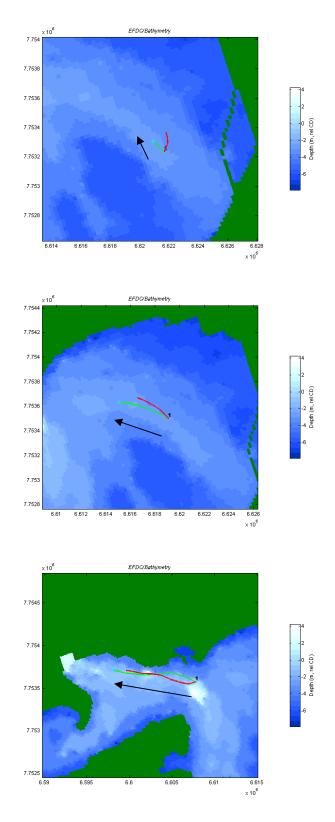
Table 2-2: Characteristics of wate	r currents in	West	Creek	estimated	from	the	results	of
drogue experiments; July 2009 field	campaign.							

Date	Release Number	Time	Distance (m)	Max Speed (km/h)
17/07/09	1	12:46 PM – 1:43 PM	839	1.1
Flood tide	2	2:56 PM – 3:37 PM	619	0.9
20/07/09	3	9:43 AM – 10:30 AM	191	0.2
Ebb tide	4	10:33 AM – 11:35 AM	390	0.4
	5	11:39 AM – 12:18 PM	850	1.3



Note: Black arrows indicate approximate directions of drogue transport (Flood Tide).

Figure 2-16: Observed (green) versus predicted (red) drogue tracks for 17 July 2009 releases.



Note: Black arrows indicate approximate directions of drogue transport (Ebb Tide).

Figure 2-17: Observed (green) versus predicted (red) drogue tracks for 20^t July 2009 releases.

2.3.5.2 Piezometer Validation

Six Odyssey pressure loggers (piezometers) were deployed by SKM within West Creek and along the mangrove areas (Figure 2-18). Sites 2, 3 and 5 were located on the mangrove fringe while sites 1, 4 and 6 were located within mangrove creeks.

Figure 2-19 shows a comparison between the observed and modelled water elevations at the six piezometer locations. From the comparison of the corresponding water levels it clearly follows that the model effectively simulated the tidal inundation in both phase and amplitude at the six sites investigated.



Figure 2-18: Location of piezometer deployments.

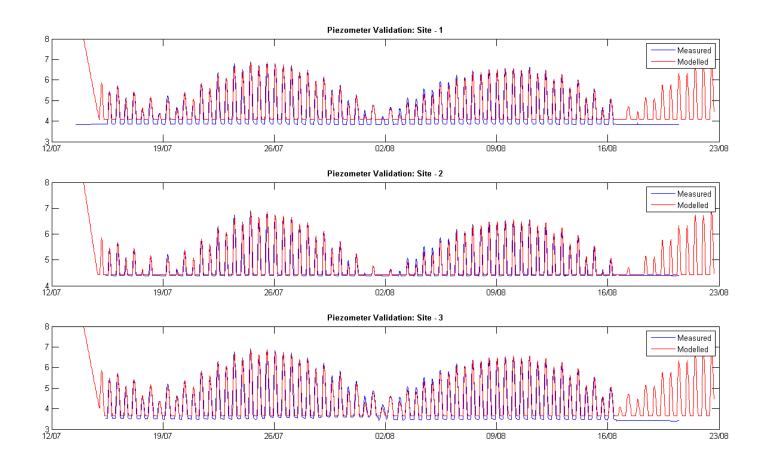


Figure 2-19: Comparison between observed and modelled water elevations at the six piezometer locations.

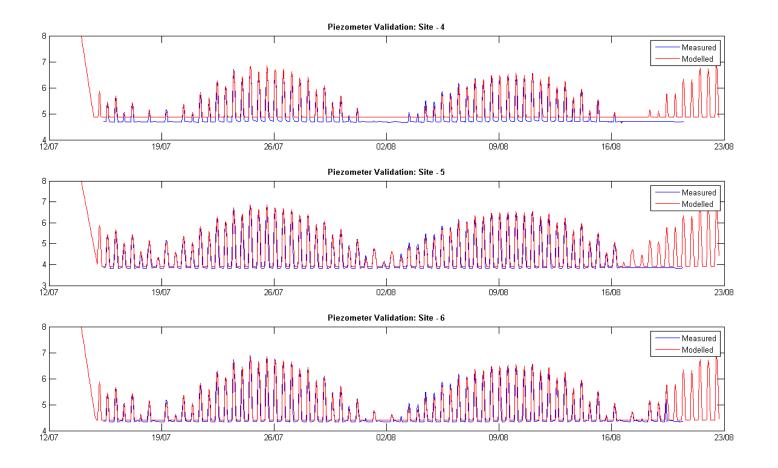


Figure 2-19: Continued.

3 MODELLING METHODS

In order to predict the impact of the proposed West Creek crossing (Figure 1-2) on the surrounding waters and mangrove areas, a detailed assessment of tidal inundation was required. The large tidal range in West Creek and the complexity of the local topography required the use of numerical modelling. Preliminary tidal inundation modelling was undertaken for the existing base case simulations and with the proposed embankment. Given the requirements of this study, the EFDC model was selected.

3.1 Simulation Scenarios

A comparative study was conducted to ascertain the impact of the proposed solid embankment on circulation and water levels within West Creek. Four scenarios have been modelled including a simulation of the existing configuration before any development works to establish baseline conditions and three comparative simulations representing the inclusion of the proposed embankment with 5, 10 and 15 culverts placed along the main drainage channels where they intersect with the proposed causeway (Figure 3-1). Data for the drainage channels was obtained from Qantum area 1 contour and detail data prepared by AAMHATCH. While this data did not extend fully into the mangrove zone it was sufficient to detail where the main channels intersected with the proposed structure. This data correlated well when compared with aerial imagery.

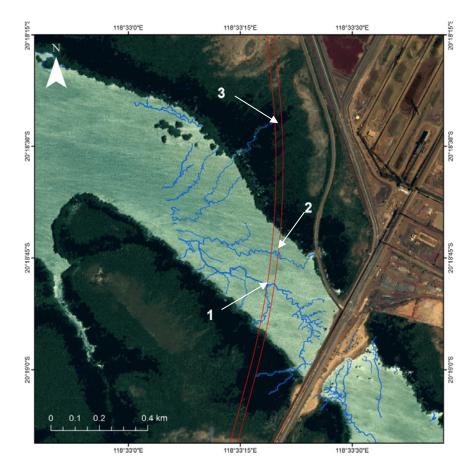


Figure 3-1: Location of existing drainage channels.

The changes to the existing environment are outlined below:

3.1.1 Scenario 2 – 5 Culvert Simulation

The infrastructure corridor is proposed to comprise conveyors and an access road, transporting ore from the proposed Boodarie stockyards to the Finucane Island transfer station where it is then conveyed to the offshore wharf, for ship loading.

The design of the infrastructure corridor investigated in this study was a solid causeway with culverts to allow tidal flow through the causeway. Preliminary culvert designs have been developed and have been incorporated in the modelling (Appendix A). The dimensions of the 2 culvert locations (Figure 3-2 (B)) are presented in Table 3-1. In this simulation 5 culverts have been included with 3 culverts placed at location 2 and 3 culverts placed at location 3. It should be noted that culverts have been designed to minimise impediment to tidal flow. Figure 3-2 (B) presents the bathymetry representing the 5 culvert simulation.

Culvert Location	Design Diameter of Culvert (mm)	Number of Culverts	Cross-sectional Width in Model (m)
2	2000	3	7
3	2000	2	5

Table 3-1: Culvert design dimensions used for modelling.

3.1.2 Scenario 3 – 10 Culvert Simulation

For the 10 culvert case the number of culverts were increased at both locations 2 and 3, with 5 culverts placed at each location. The dimensions of the 2 culvert locations (Figure 3-2 (A)) are presented in Table 3-2. Figure 3-3 (A) presents the bathymetry representing the 10 culvert simulation.

Table 3-2: Culvert design dimensions used for modelling.

Culvert Location	Design Diameter of Culvert (mm)	Number of Culverts	Cross-sectional Width in Model (m)
2	2000	5	12
3	2000	5	12

3.1.3 Scenario 4 – 15 Culvert Simulation

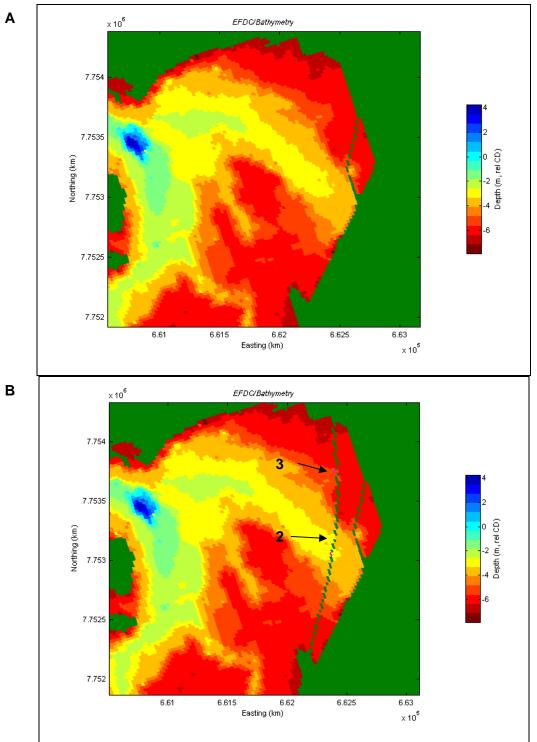
The fourth scenario involved increasing the number of culverts to 15 and placing them at locations 1, 2 and 3, with 5 culverts at each location. The dimensions of the 2 culvert locations (Figure 3-2 (B)) are presented in Table 3-3. Figure 3-3 (B) presents the bathymetry representing the 10 culvert simulation.

Culvert Location	Design Diameter of Culvert (mm)	Number of Culverts	Cross-sectional Width in Model (m)
1	2000	5	12
2	2000	5	12
3	2000	5	12

3.2 Model Parameters

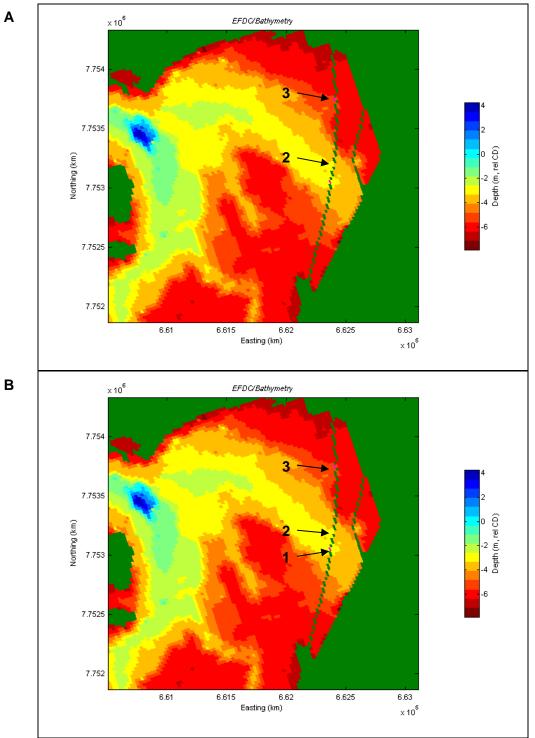
Details of important model parameters are presented below:

- The model was run with a 2.5 second-time step, for a length of 40 days for each of the simulations.
- The model has one open boundary at the entrance to West Creek (Figure 1-2).
- Variable roughness coefficients were applied to incorporate higher bed friction within the mangrove areas. A roughness coefficient of 0.02 m representative of clean natural channels was applied to the grid and a higher value of 0.1 m representative of friction for the mangrove areas. These values were based on previous modelling experience in mangrove areas of Port Hedland and best fit to validation data.



Note: black numbers represent culvert locations

Figure 3-2: Bathymetric maps of two simulation layouts, existing layout (A) and causeway layout – 5 culverts (B).



Note: black numbers represent culvert locations

Figure 3-3: Bathymetric maps of two simulation layouts, causeway layout – 10 culverts (A) and causeway layout – 15 culverts (B).

4 RESULTS

4.1 General Circulation

Typical peak ebb tide and flood tide currents for the existing configuration, 5 culvert, 10 culvert and 15 culvert simulations are presented in Figure 4-1, Figure 4-2, Figure 4-3 and Figure 4-4 respectively. The peak currents in the model domain ranged up to 1 m/s and were slightly larger in the ebb than the flood tide.

On both the ebb and flood tide strong currents are evident along the entrance channel; the current magnitudes reduce as the channel becomes wider and bend into West Creek.

The proposed developments lead to localised changes in the peak current magnitudes due to restriction of flow due to the causeway. The most significant change is a reduction of the currents on the eastern side of the proposed causeway within the main channel. Some localised peak current increases are evident at the culvert sites particularly on the flood tide, which is due to the constriction of the flow at these areas.

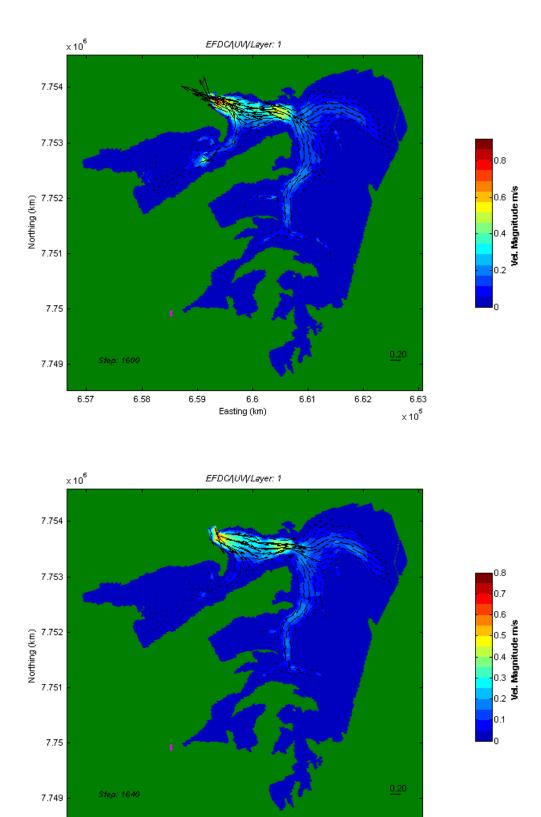


Figure 4-1: Typical peak ebb (top) and peak flood tide (bottom) current magnitudes and directions for existing case.

6.61

6.62

6.63

× 10⁵

6.58

6.59

6.6

Easting (km)

6.57

0.8

0.6

0.4

0.2

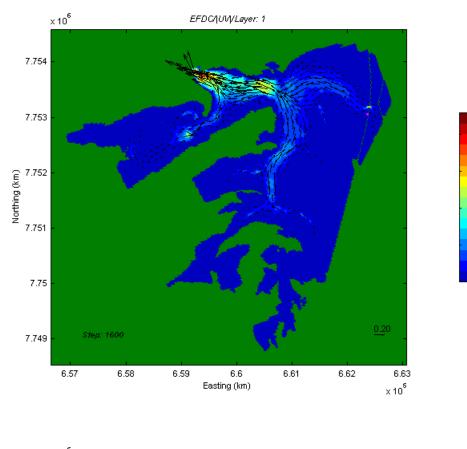
0.7 0.6

> 0.5 0.4 Wadnitude W

0.2 🕏 0.1

0

Vel. Magnitude m/s



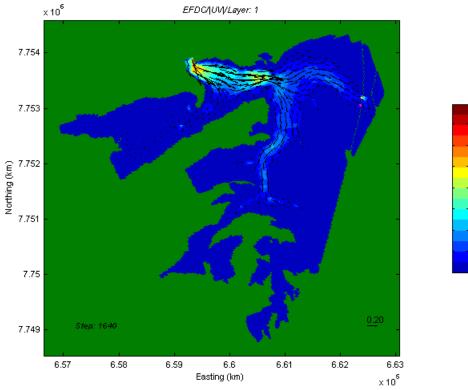


Figure 4-2: Typical peak ebb (top) and peak flood tide (bottom) current magnitudes and directions for 5 culvert case.

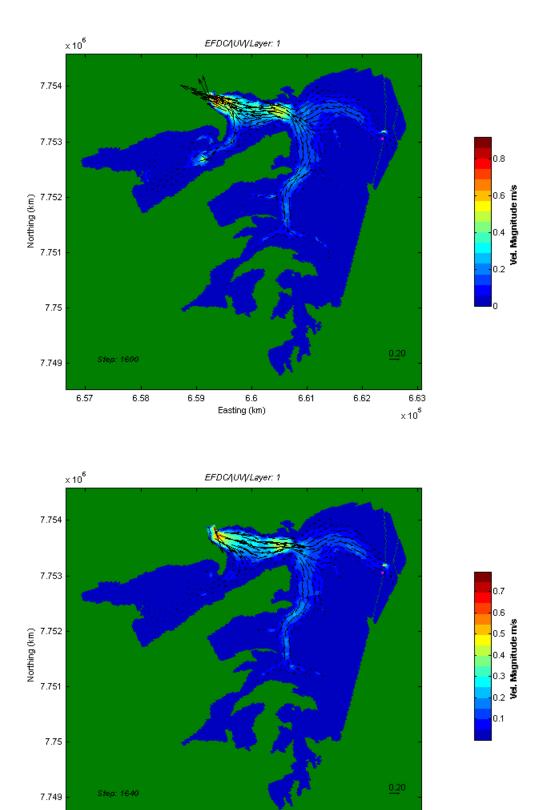


Figure 4-3: Typical peak ebb (top) and peak flood tide (bottom) current magnitudes and directions for 10 culvert case.

6.61

6.62

6.63

× 10⁵

6.58

6.59

6.6

Easting (km)

6.57

0.8

0.6

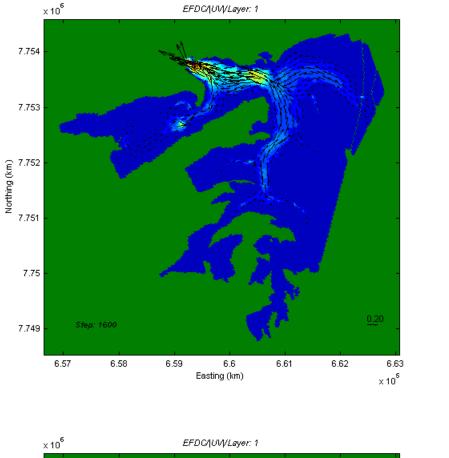
0.4

0.2

Vel. Magnitude m/s

Magnitude m/s

Ϋ́Θ.



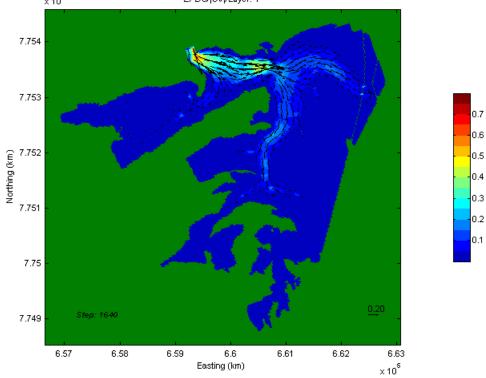


Figure 4-4: Typical peak ebb (top) and peak flood tide (bottom) current magnitudes and directions for 15 culvert case.

4.2 Impact of Infrastructure Corridor on Tidal Currents

The potential for the proposed development to have deleterious flow-on effects over the wider region of West Creek and Mangrove areas was investigated. Although the majority of the impacts should be localised, alterations to flow patterns could have the potential to affect the inundation of the mangrove areas and sedimentation patterns within the West Creek.

4.2.1 Residual Current Patterns

From the model results for the three simulations, residual depth-averaged currents were computed for a 35-day period. This essentially removes the influence of the spring-neap tidal cycle allowing any tidal asymmetry to be revealed. The difference between existing and the post development scenarios was then calculated, and the results are presented in Figure 4-5, Figure 4-6 and Figure 4-7 respectively. The scale shows changes of up to +/- 0.05m/s in magnitude with **positive** values (yellow to red) showing an increase in residual current speed following development and white representing insignificant changes in current magnitude following development.

Impacts on the residual currents from the proposed development are localised to the area surrounding the culverts extending along the main channel of West Creek. The residual currents in these areas are increased but drop quickly with distance from the culvert locations.

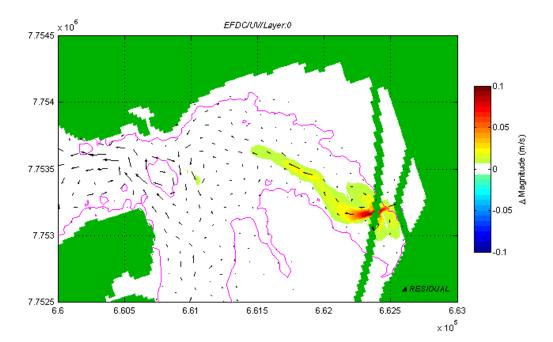


Figure 4-5: Residual depth averaged current differences between the existing and the proposed development with 5 culverts. Positive values represent an increase in residual current magnitude post development.

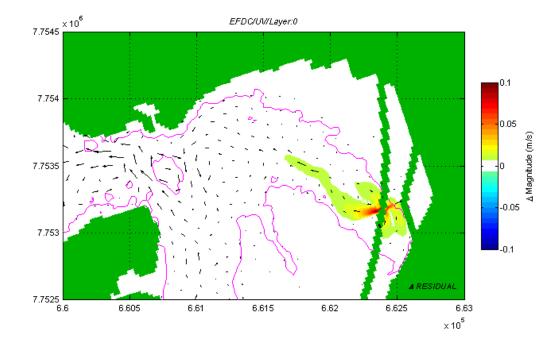


Figure 4-6: Residual depth averaged current differences between the existing and the proposed development with 10 culverts. Positive values represent an increase in residual current magnitude post development.

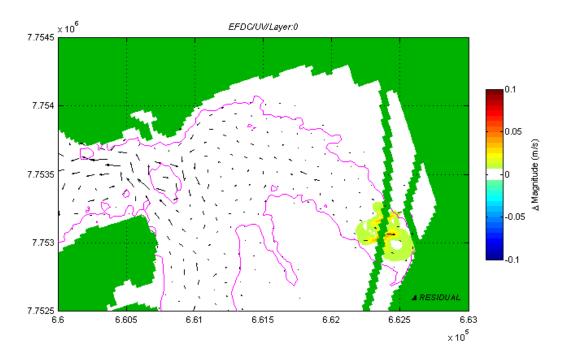


Figure 4-7: Residual depth averaged current differences between the existing and the proposed development with 15 culverts. Positive values represent an increase in residual current magnitude post development.

Maximum Current Speeds

From the model results of the four simulations, maximum depth-averaged currents were computed for a 35 day period over the model domain. The difference between existing and post development scenarios was then calculated and the results are shown in Figure 4-8, Figure 4-9 and Figure 4-10.

The maximum current differences are also localised to the culvert locations for all simulations. Impacts on the maximum currents from the 5 and 10 culvert simulations are localised to one culvert location in the main channel with no significant increases or decreases anywhere else. The 15 culvert simulation shows reduced maximum currents at two lower culvert locations. All simulations indicated no increase or decreases at the upper culvert location.

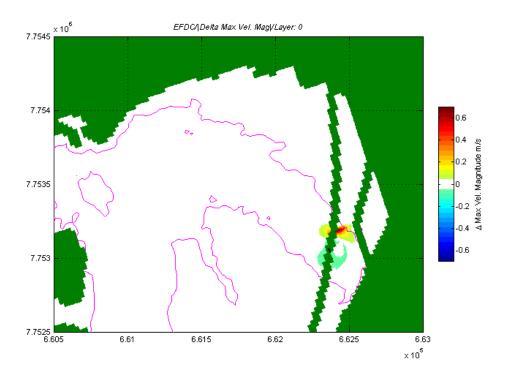


Figure 4-8: Maximum current differences between the existing and the proposed development with 5 culverts. Positive values represent an increase in maximum current magnitude post development.

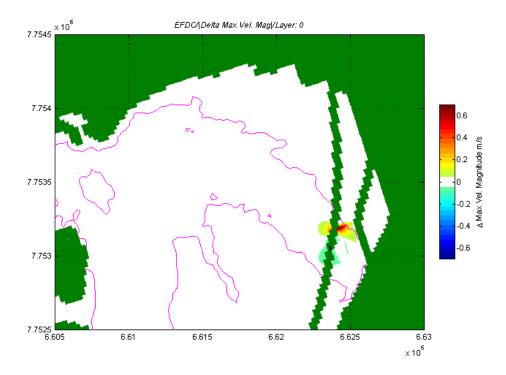


Figure 4-9: Maximum current differences between the existing and the proposed development with 10 culverts. Positive values represent an increase in maximum current magnitude post development.

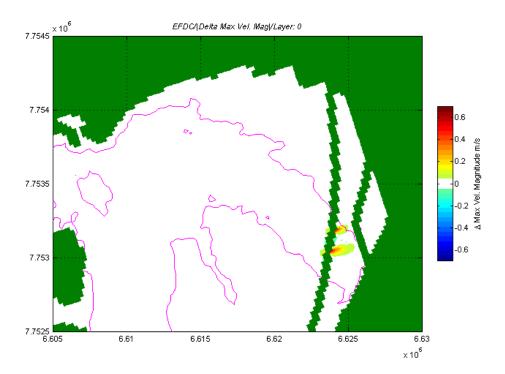


Figure 4-10: Maximum current differences between the existing and the proposed development with 15 culverts. Positive values represent an increase in maximum current magnitude post development.

4.3 Impact of Infrastructure Corridor on Water Levels within West Creek and Mangrove Areas

Submergence curves have been developed for the surface elevation output of the model for 48 locations specified by SKM within the model domain (Table 4-1). These curves have been developed to examine the effect of the proposed West Creek crossing on water levels within West Creek, in particular in the inundation of the mangrove areas. The latitude and longitude of each point is provided in Table 4-1 and the location shown in Figure 4-11. Submergence curves for the 48 points for the existing simulation, 5, 10 and 15 culvert simulations are presented in Appendix B, C, D and E respectively. Comparison plots displaying the submergence curves for all simulations are presented in Appendix F.

	-		· ·		-
Point	Latitude	Longitude	Point	Latitude	Longitude
1	-20.31705131	118.5530547	25	-20.31237354	118.5565993
2	-20.31699567	118.5535268	26	-20.31248969	118.5569716
3	-20.31697433	118.5539991	27	-20.31212737	118.5572225
4	-20.31689702	118.5549799	28	-20.31176634	118.5573279
5	-20.31687632	118.5553796	29	-20.31176214	118.5578005
6	-20.31680354	118.5558515	30	-20.31155186	118.5583073
7	-20.31659423	118.5562493	31	-20.31147923	118.558761
8	-20.31620199	118.556009	32	-20.31142569	118.5589967
9	-20.31639215	118.5519757	33	-20.30888233	118.5538457
10	-20.31655715	118.5527044	34	-20.30877736	118.5540809
11	-20.31619258	118.5532097	35	-20.30861917	118.5545155
12	-20.31577835	118.5535146	36	-20.30827368	118.5548029
13	-20.31546747	118.553766	37	-20.30758334	118.5553049
14	-20.31522533	118.5539999	38	-20.30732034	118.5559566
15	-20.31499937	118.554343	39	-20.30716312	118.5562822
16	-20.31484232	118.5546504	40	-20.30702272	118.5566443
17	-20.31404333	118.5500074	41	-20.30730119	118.5523213
18	-20.31383516	118.550278	42	-20.30699145	118.5524454
19	-20.31345539	118.550565	43	-20.3066817	118.5525696
20	-20.31309227	118.5509068	44	-20.30625048	118.5528561
21	-20.31286616	118.551268	45	-20.3057832	118.5533422
22	-20.31274307	118.5516122	46	-20.30555304	118.5541578
23	-20.3126897	118.5518297	47	-20.30537852	118.5545014
24	-20.31258506	118.5520286	48	-20.30506732	118.5547891

Table 4-1: Coordinates of points where submergence curves have been developed.

* All Coordinates referenced to horizontal datum GDA94.

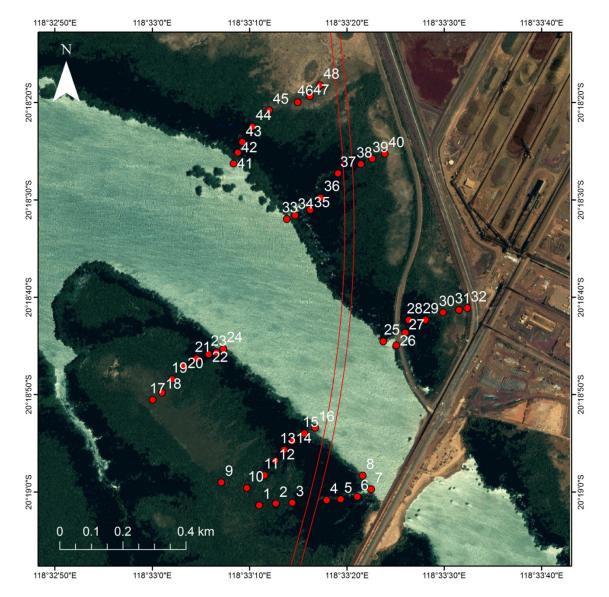


Figure 4-11: Location of points where submergence curves have been developed.

Figure 4-13 and Figure 4-14 show two examples of comparison submergence curves for all simulations (existing (blue line), 15 culvert (red line), 10 culvert (green line) and 5 culvert (black line)) for location 25 and 28 respectively. These figures show the percentage of time that a particular location is submerged. For example Figure 4-14 indicates that location 28 will be submerged for 18 percent of the time for all simulations modelled. This indicates that there would be no change in frequency and duration of inundation at location 28. Location 25 (Figure 4-13) on the other hand is located in the main channel and is lower with respect to CD therefore the percentage of time that this location is submerged is much greater. Unlike location 28 the comparison of submergence curves at this location indicates that the duration of inundation would change for the different simulations. There was no change between the existing and 15 culvert simulation, but the percentage of time submerged increased from 57 to 67 percent for the 10 culvert simulation and to 93 percent for the 5 culvert simulation. This

indicates that the 5 and 10 culvert options don't allow sufficient drainage as the water levels approach MSL.

Figure 4-12 shows a time series of elevation (CD) at location 25 for the existing (blue line), 5 culvert (red line), 10 culvert (green line) and 15 culvert (purple line) simulations. Figure 4-12 serves to reinforce that on the ebbing tide the retardation of flow caused by the 5 and 10 culvert simulations is enough to significantly slow the drainage of certain areas east of the proposed structure. Modelling also predicted that for the 5 culvert simulation and under certain spring tide conditions areas within the main channel do not fully drain.

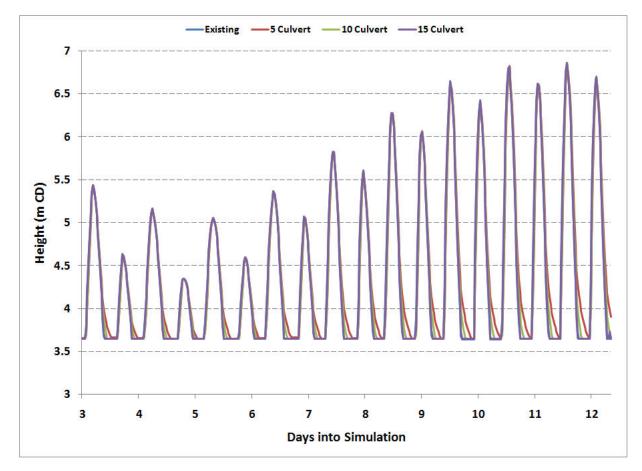
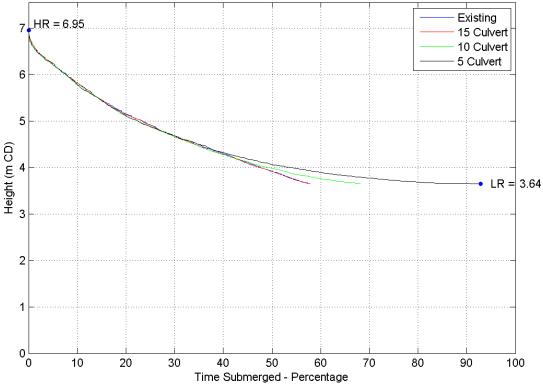


Figure 4-12: Time-series of elevation for the four simulations at location 25.

Unlike locations in the main channel and based on the submergence curves, the changes to water levels within the mangrove areas of West Creek were predicted to be small with insignificant changes to MSL, MHWS and MLWS at most of the points investigated. Changes to extreme water levels (levels submerged <10% or levels exposed <10% of the time) were observed; however, these changes were small and in the order of 1 - 5 cm across the four simulations. Based on the magnitude of the observed changes inundation within the mangroves should not significantly change.

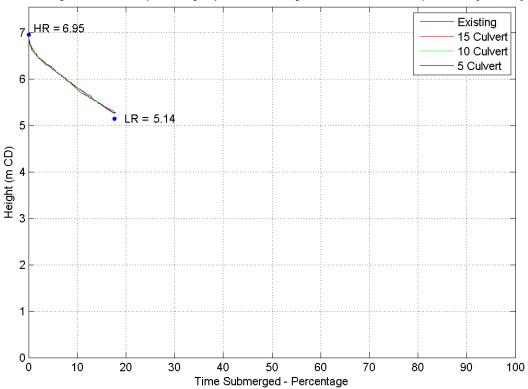
It should be noted that all locations west of the proposed structure also showed approximately no change across all four simulations.



Submergence Curve - Representing Expected Tide Heights Based	d on Model Output - Point [231,188]
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Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	4.92	3.92	Dry	Dry	Dry	Dry	Dry
15 Culvert WL (m CD)	6.63	6.48	6.23	5.81	4.91	3.91	Dry	Dry	Dry	Dry	Dry
10 Culvert WL (m CD)	6.61	6.45	6.23	5.78	4.87	3.98	Dry	Dry	Dry	Dry	Dry
5 Culvert WL (m CD)	6.58	6.46	6.2	5.79	4.88	4.06	3.72	3.66	Dry	Dry	Dry

Figure 4-13: Submergence curve for all simulations – Point 25



Cubmorgenee Cun/e	 Representing Expected 	d Tida Haiabta Daaad a	n Madal Outnut	Daint [226 400]
Submerdence Curve	- Representing Expected	J THE HEIGHLS BASED O	n Model Ouldul	- POINLIZSE, 1091

Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.63	6.47	6.23	5.82	Dry						
15 Culvert WL (m CD)	6.63	6.48	6.23	5.82	Dry						
10 Culvert WL (m CD)	6.61	6.45	6.22	5.78	Dry						
5 Culvert WL (m CD)	6.58	6.46	6.2	5.79	Dry						

Figure 4-14: Submergence curve for all simulations – Point 28

5 CONCLUSIONS AND RECOMMENDATIONS

Overall the comparative modelling predicts that the impacts of the proposed infrastructure corridor will be localised to areas east of the proposed West Creek development, with minimal impact in the broader creek and mangrove regions. The principal findings of the investigation are as follows:

5.1 Impact on Tidal Currents

- The comparative modelling predicts that the impacts of the proposed developments on both residual and maximum tidal currents within West Creek will be localised to the area surrounding the culverts along the proposed infrastructure corridor.
- Impacts on the residual currents from the proposed development are localised to the area surrounding the culverts extending along the main channel of West Creek. The residual currents in these areas are increased but drop quickly with distance from the culvert locations.
- The maximum current differences are also localised to the culvert locations for all simulations. Impacts on the maximum currents from the 5 and 10 culvert simulations are localised to one culvert location in the main channel with no significant increases or decreases anywhere else. The 15 culvert simulation shows reduced maximum currents at two lower culvert locations. All simulations indicated no increase or decreases at the upper culvert location

5.2 Impact on Water Levels and Inundation of Mangrove Areas

- Changes in duration of inundation for areas east of the proposed development are predicted for the 5 and 10 culvert simulations when compare with the existing simulation. The increase in duration varied based on location and state of the tide. This was limited to the main channel where the tidal rage was larger. This indicates that the 5 and 10 culvert options don't allow sufficient drainage as the water levels approach MSL. Areas above MSL in the mangrove zones were predicted to see small changes in extreme water levels (1 5 cm) but no change in the frequency and duration of inundation.
- The 15 culvert simulation was predicted to show no change when compared to the existing case in both the frequency and duration of inundation for all sites investigated; however small changes to extreme water levels were predicted (1 – 5 cm).

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BHP Billiton's Iron Ore Port Hedland Outer Harbour Development

Tidal Flushing of West Creek

APPENDIX Revision – 1 Draft APPENDIX 09/10/09 Prepared for: SKM & BHPBIO



Document control form

Document draft	Originated by	Edit & review	Authorized for release by	Date
Draft 1 - Issued for internal review	Marc Zapata	Marc Zapata		13/10/09

Document name: West Creek tidal inundation APENDIX

APASA Project Number: J0059

APASA Project Manager: Marc Zapata

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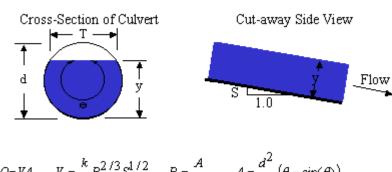
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Figure F-29: Submergence curve for all simulations – Point 29.238Figure F-30: Submergence curve for all simulations – Point 30.239Figure F-31: Submergence curve for all simulations – Point 31.240Figure F-32: Submergence curve for all simulations – Point 32.241Figure F-33: Submergence curve for all simulations – Point 33.242Figure F-34: Submergence curve for all simulations – Point 34.243Figure F-35: Submergence curve for all simulations – Point 35.244Figure F-36: Submergence curve for all simulations – Point 36.245Figure F-37: Submergence curve for all simulations – Point 37.246
Figure F-30: Submergence curve for all simulations – Point 30.239Figure F-31: Submergence curve for all simulations – Point 31.240Figure F-32: Submergence curve for all simulations – Point 32.241Figure F-33: Submergence curve for all simulations – Point 33.242Figure F-34: Submergence curve for all simulations – Point 34.243Figure F-35: Submergence curve for all simulations – Point 35.244Figure F-36: Submergence curve for all simulations – Point 36.245Figure F-37: Submergence curve for all simulations – Point 37.246
Figure F-31: Submergence curve for all simulations – Point 31240Figure F-32: Submergence curve for all simulations – Point 32241Figure F-33: Submergence curve for all simulations – Point 33242Figure F-34: Submergence curve for all simulations – Point 34243Figure F-35: Submergence curve for all simulations – Point 35244Figure F-36: Submergence curve for all simulations – Point 36245Figure F-37: Submergence curve for all simulations – Point 37246
Figure F-32: Submergence curve for all simulations – Point 32
Figure F-33: Submergence curve for all simulations – Point 33.242Figure F-34: Submergence curve for all simulations – Point 34.243Figure F-35: Submergence curve for all simulations – Point 35.244Figure F-36: Submergence curve for all simulations – Point 36.245Figure F-37: Submergence curve for all simulations – Point 37.246
Figure F-34: Submergence curve for all simulations – Point 34
Figure F-35: Submergence curve for all simulations – Point 35
Figure F-36: Submergence curve for all simulations – Point 36
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Figure F-38: Submergence curve for all simulations – Point 38
Figure F-39: Submergence curve for all simulations – Point 39
Figure F-40: Submergence curve for all simulations – Point 40
Figure F-41: Submergence curve for all simulations – Point 41
Figure F-42: Submergence curve for all simulations – Point 42
Figure F-43: Submergence curve for all simulations – Point 43
Figure F-44: Submergence curve for all simulations – Point 44
Figure F-45: Submergence curve for all simulations – Point 45
Figure F-46: Submergence curve for all simulations – Point 46
Figure F-47: Submergence curve for all simulations – Point 47

Figure F-48: Submergence curve for	r all simulations – Point 4	8257
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A. APPENDIX - CULVERT DESIGN

Culvert design for a single 2m circular culvert.



$$Q = VA \qquad V = \frac{1}{n}R^{n+1}S^{n+1} \qquad R = \frac{1}{p} \qquad A = \frac{1}{8}(\theta - \sin(\theta))$$
$$P = \frac{\theta d}{2} \qquad y = \frac{d}{2}\left[1 - \cos\left(\frac{\theta}{2}\right)\right] \qquad T = 2\sqrt{y(d-y)} \qquad F = V\sqrt{\frac{T}{gA\cos(Tan^{-1}S)}}$$

Parameter	Value
A = Flow cross-sectional area, determined normal (perpendicular) to the bottom surface $[L^2]$.	Calculated
d = Culvert diameter [L].	2 m
F = Froude number. F is a non-dimensional parameter indicating the relative effect of inertial effects to gravity effects. Flow with F<1 are low velocity flows called subcritical. F>1 are high velocity flows called supercritical. Subcritical flows are controlled by downstream obstructions while supercritical flows are affected by upstream controls. F=1 flows are called critical.	Calculated
g = acceleration due to gravity. g is used in the equation for Froude number.	9.8066 m/s ²
k = unit conversion factor	1.0 if metric units
n = Manning coefficient. n is a function of the culvert material, such as plastic, concrete, brick, etc.	0.12 (Finished concrete)
P = Wetted perimeter [L]. P is the contact length (in the cross-section) between the water and the culvert.	Calculated
Q = Discharge or flowrate [L ³ /T]	Calculated
R = Hydraulic radius of the flow cross-section [L].	
S = Slope of channel bottom or water surface $[L/L]$. Vertical distance divided by horizontal distance.	0.02
T = Top width of the flowing water [L].	Calculated
V = Average velocity of the water [L/T].	Calculated
y = Water depth measured normal (perpendicular) to the bottom of the culvert[L]. If the culvert has a small slope (S), then entering the vertical depth introduces only minimal error	Calculated
\emptyset = Angle representing how full the culvert is [radians]. A culvert with \emptyset =0 radians (0°) contains no water, a culvert with \emptyset =pi radians (180°) is half full, and a culvert with \emptyset =2 pi radians (360°) is completely full.	Calculated

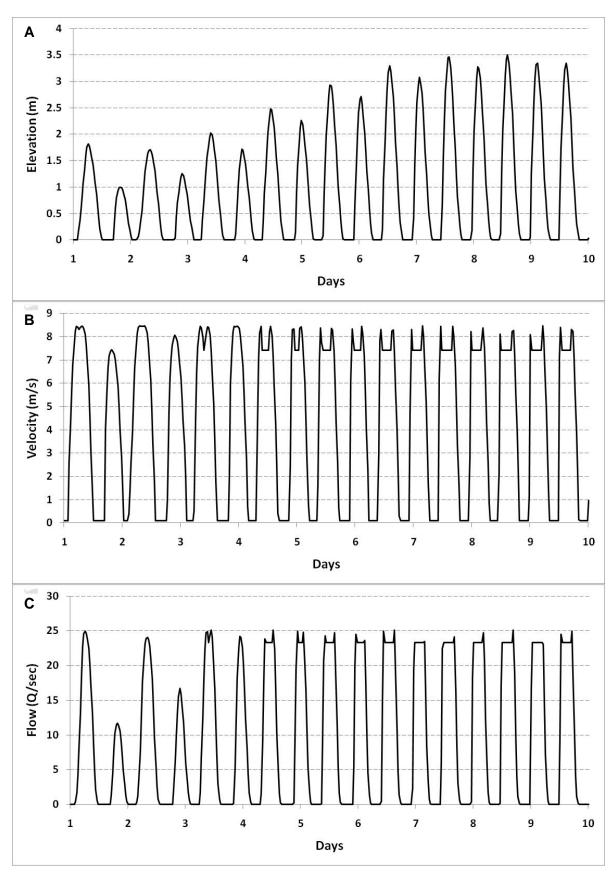
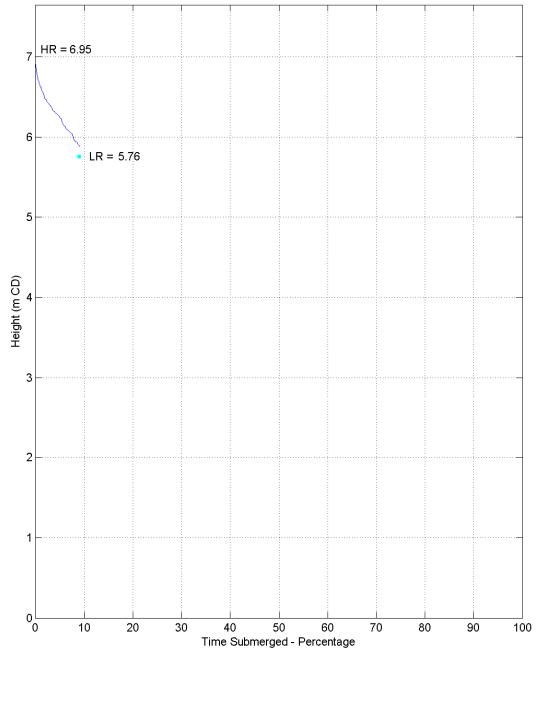


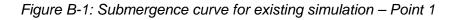
Figure A-1: Time-series of velocity (B) and flow rate (C) as a result of elevation (A).

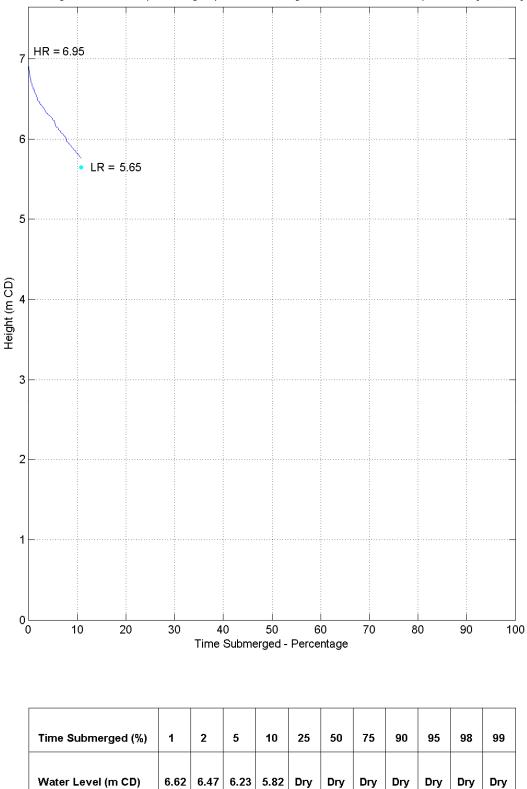
B. APPENDIX – SUBMERGENCE CURVES FOR EXISTING LAYOUT SIMULATION



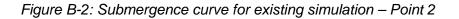
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [210,173]

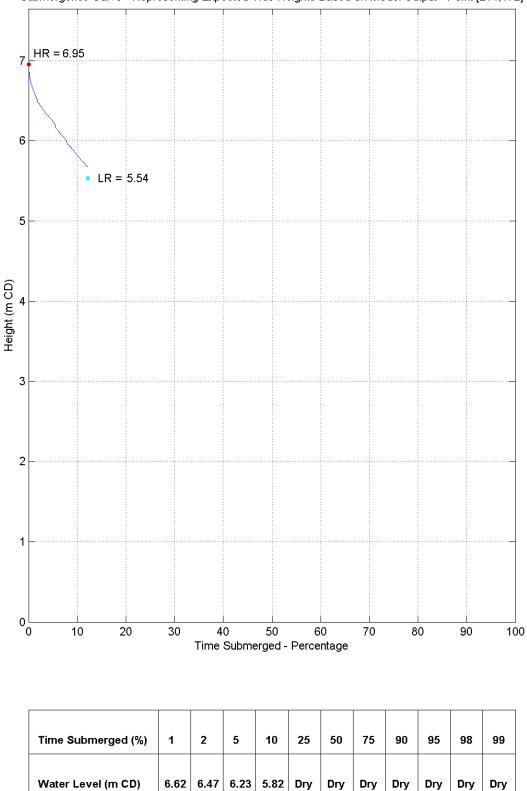
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Water Level (m CD)	6.62	6.47	6.23	Dry							



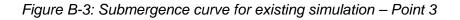


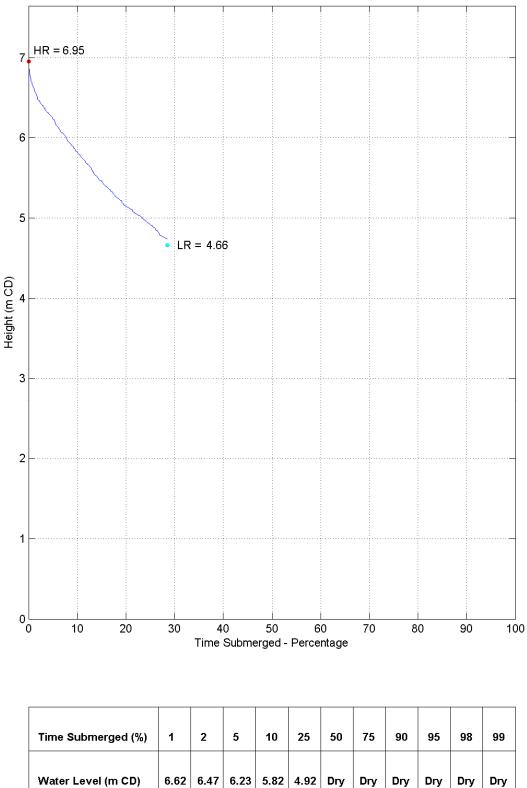
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [212,173]

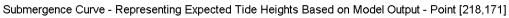


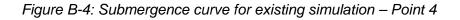


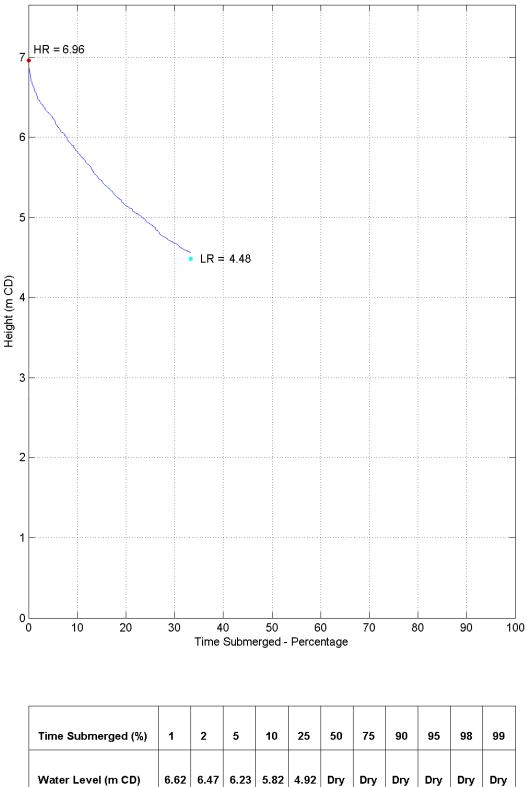
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [214,172]



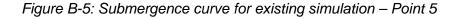


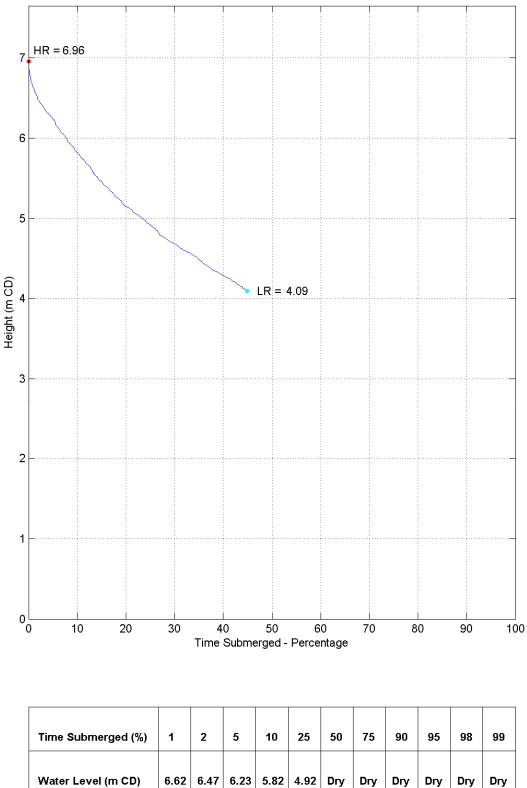




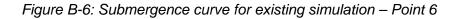


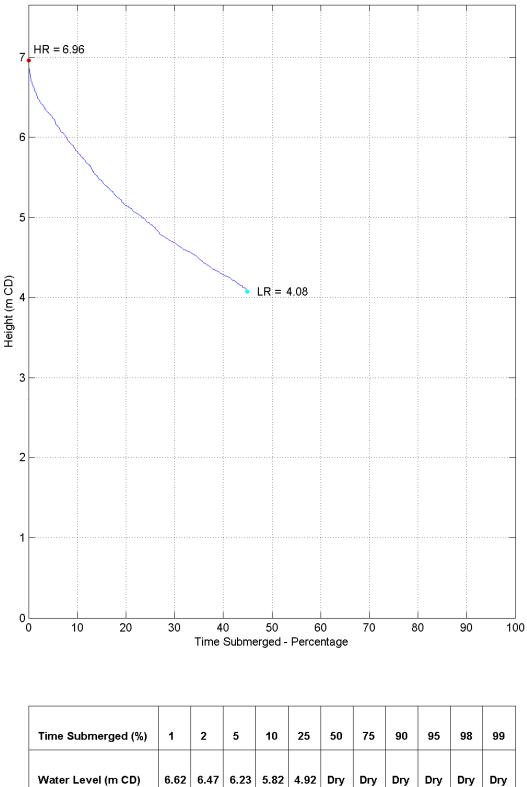
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [220,171]

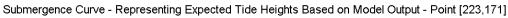


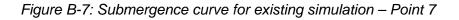


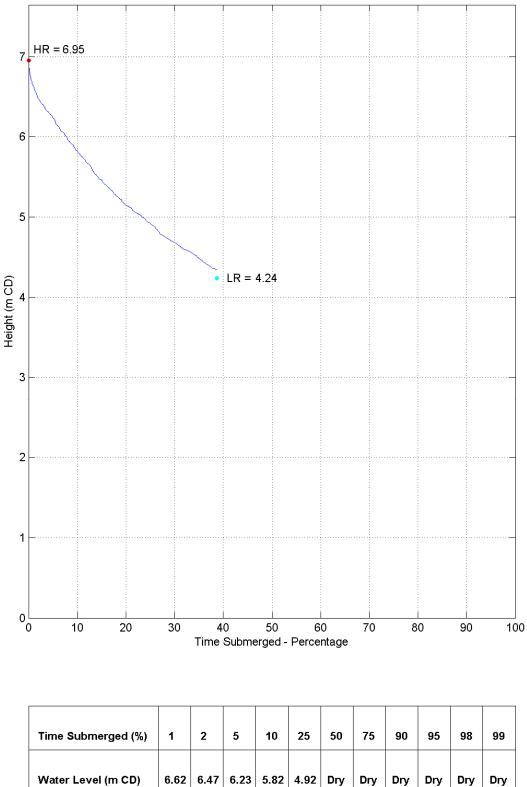
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [222,171]



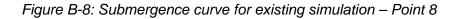


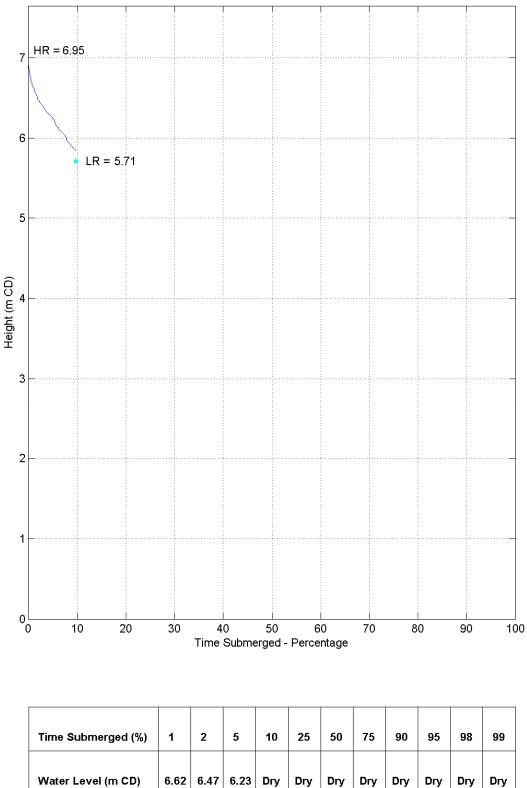




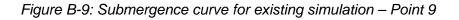


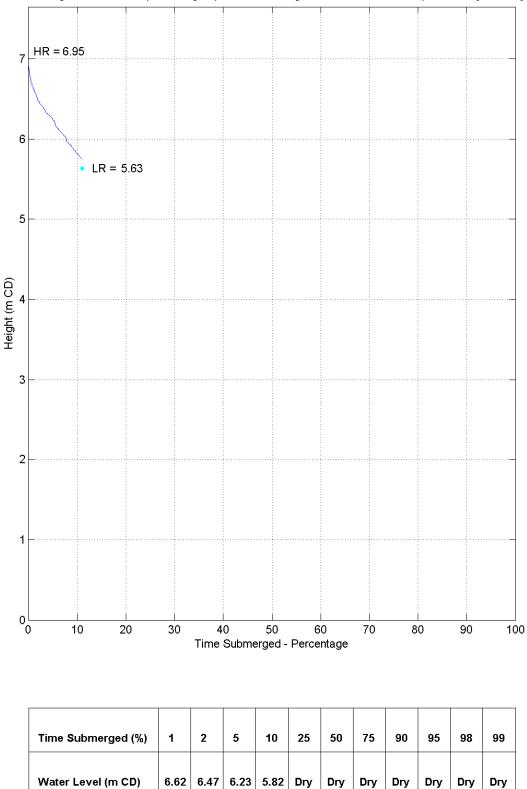
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [223,173]





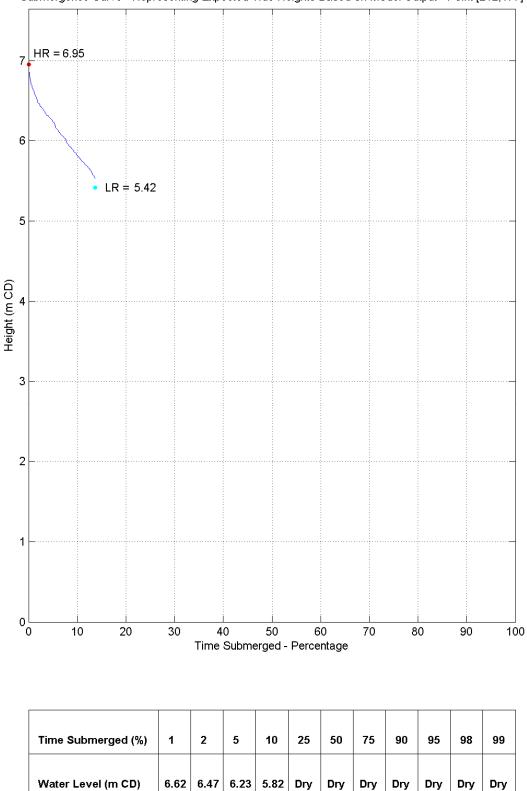
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [207,177]





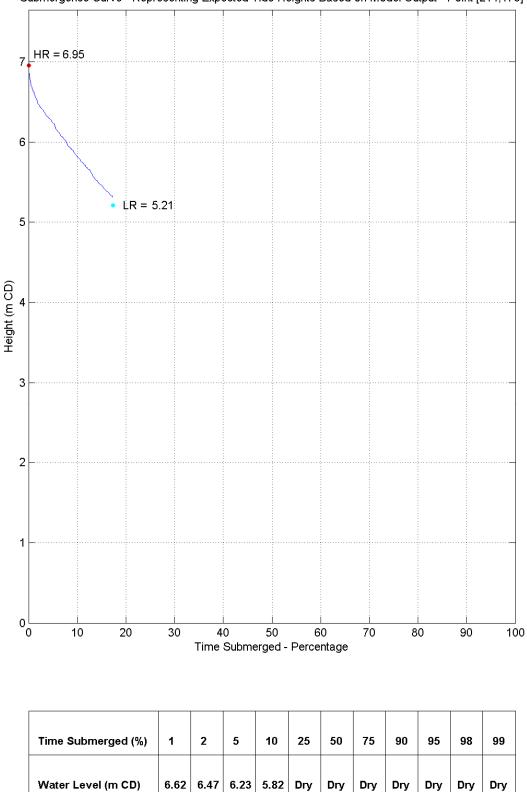
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [209,176]

Figure B-10: Submergence curve for existing simulation – Point 10

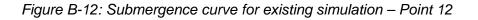


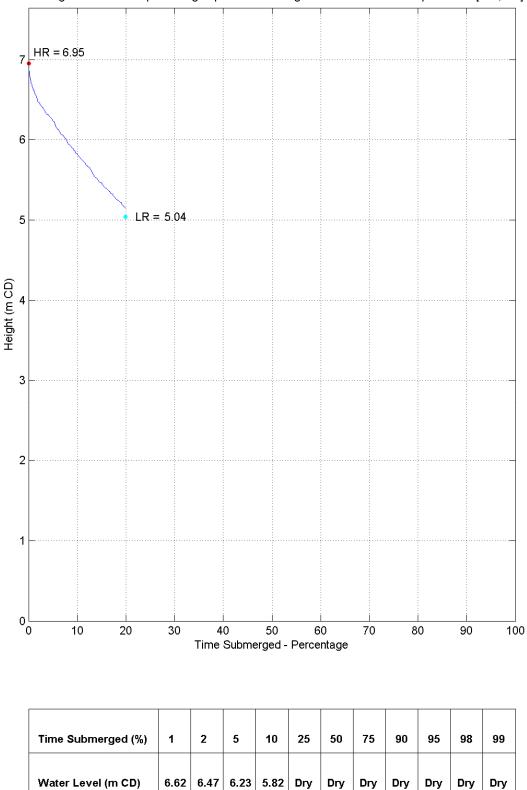
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [212,177]

Figure B-11: Submergence curve for existing simulation – Point 11



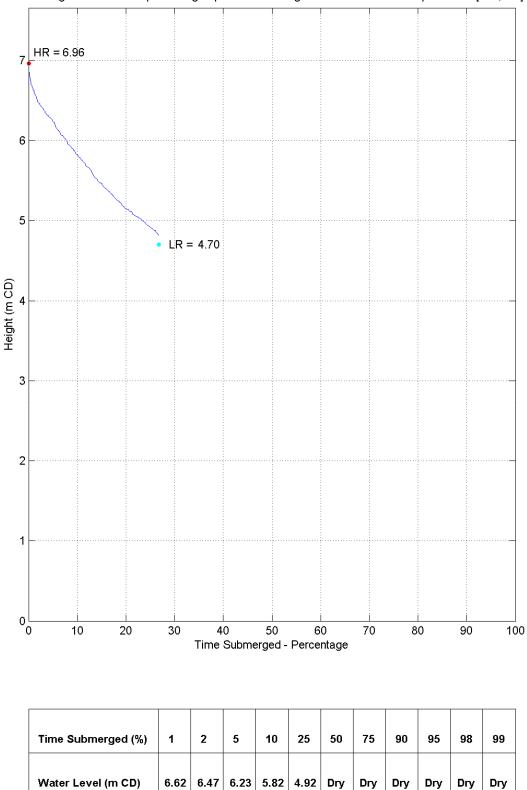
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [214,178]



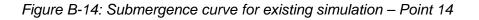


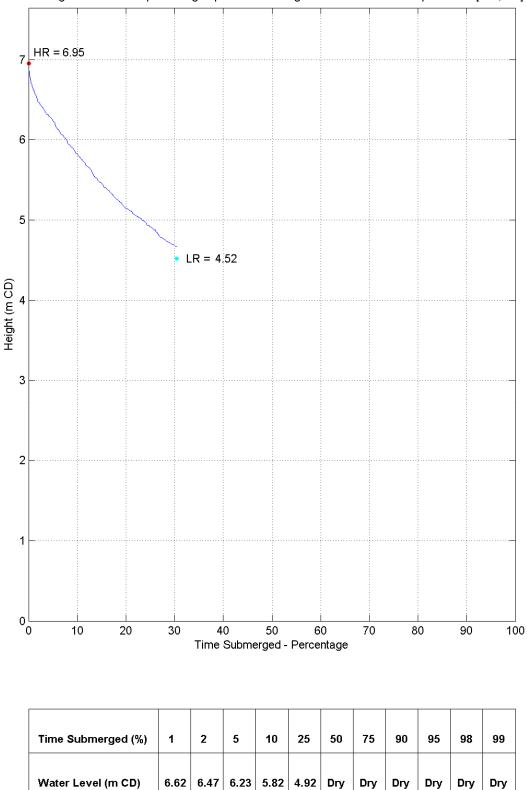
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [215,179]

Figure B-13: Submergence curve for existing simulation – Point 13

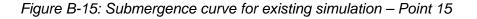


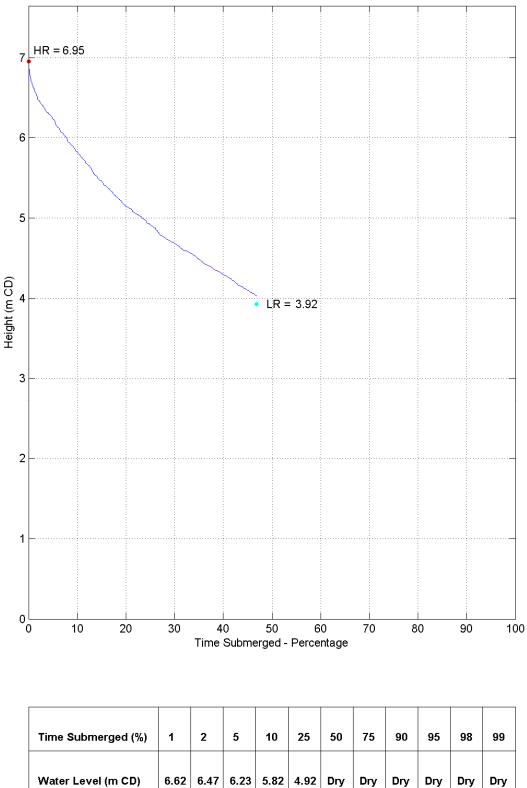
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [217,180]





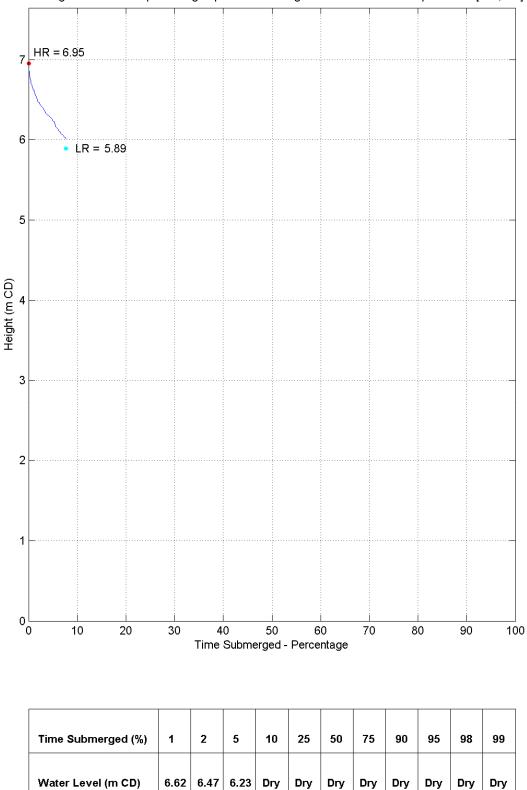
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [218,180]





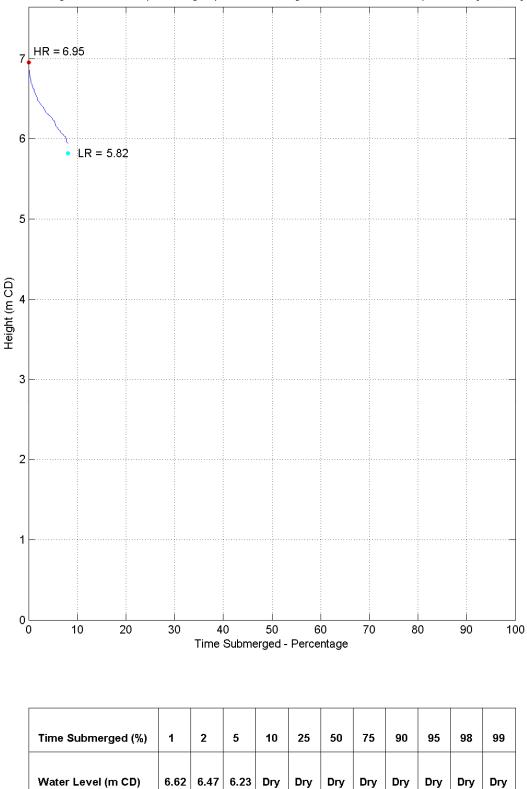
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [220,180]

Figure B-16: Submergence curve for existing simulation – Point 16



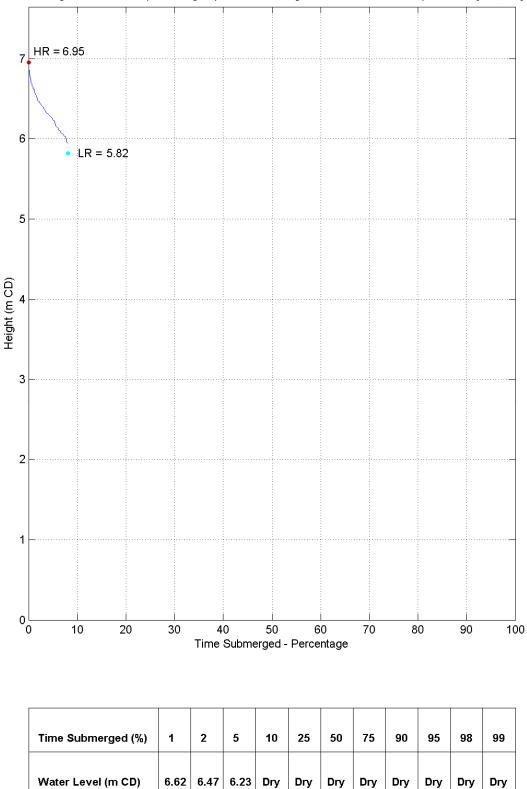
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [202,190]

Figure B-17: Submergence curve for existing simulation – Point 17



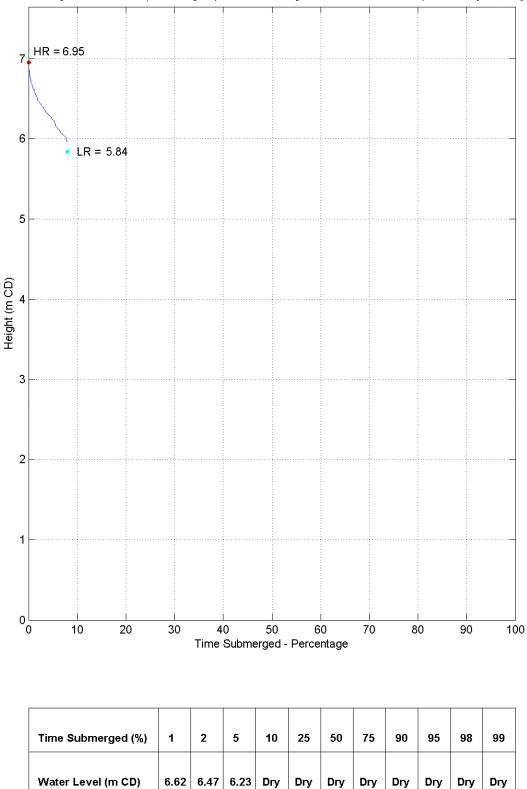
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [204,191]

Figure B-18: Submergence curve for existing simulation – Point 18



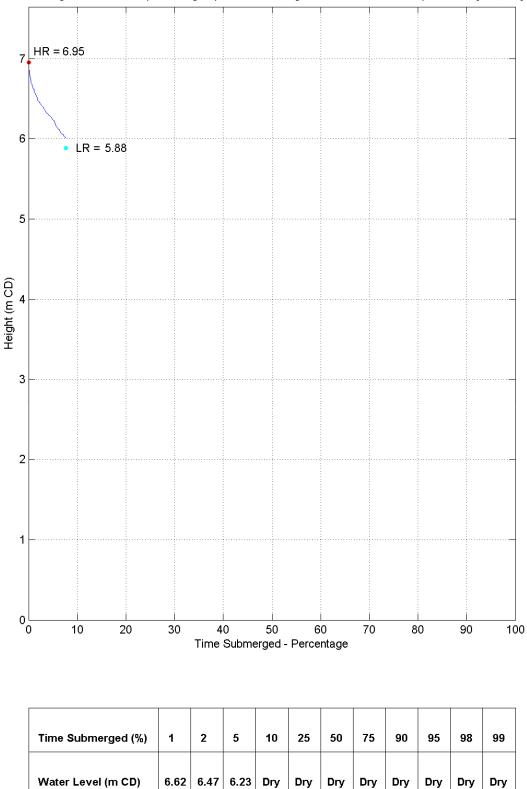
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [205,192]

Figure B-19: Submergence curve for existing simulation – Point 19



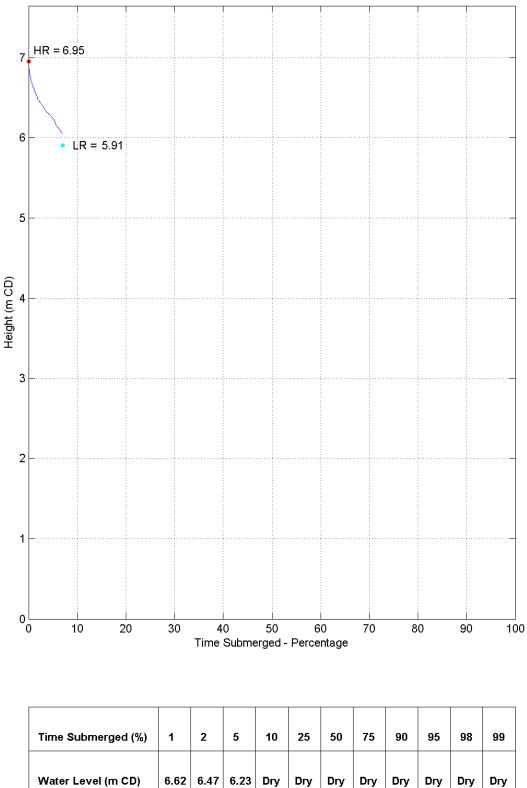
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [208,193]

Figure B-20: Submergence curve for existing simulation – Point 20



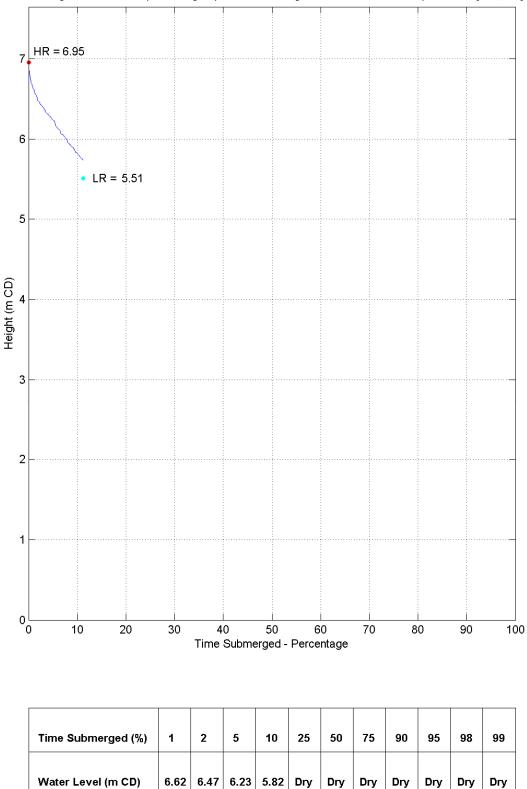
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [209,194]

Figure B-21: Submergence curve for existing simulation – Point 21

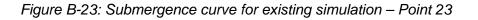


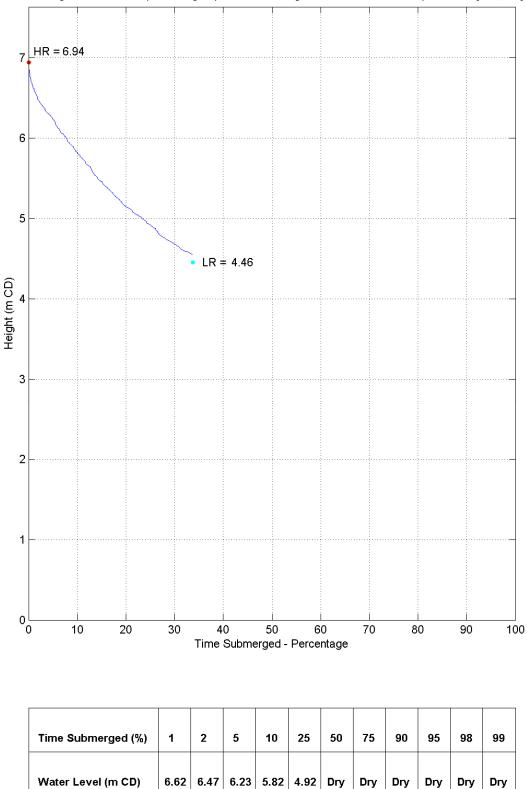
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [210,193]

Figure B-22: Submergence curve for existing simulation – Point 22

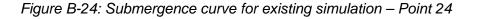


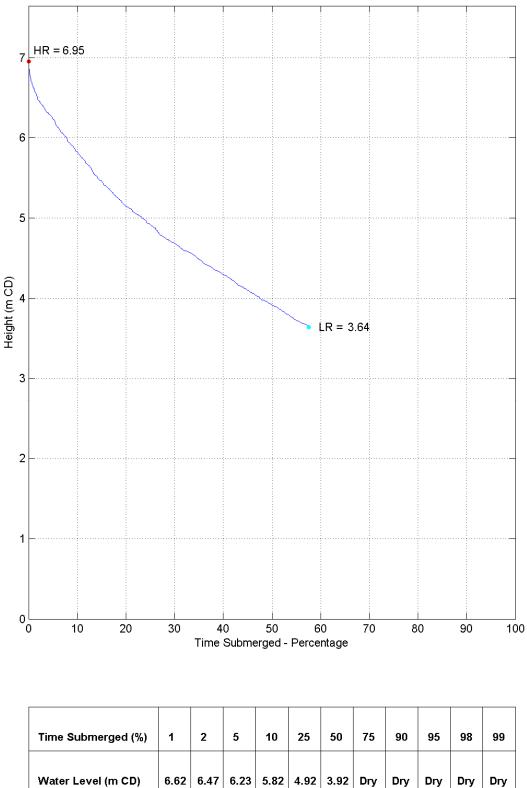
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [211,193]



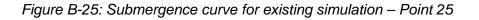


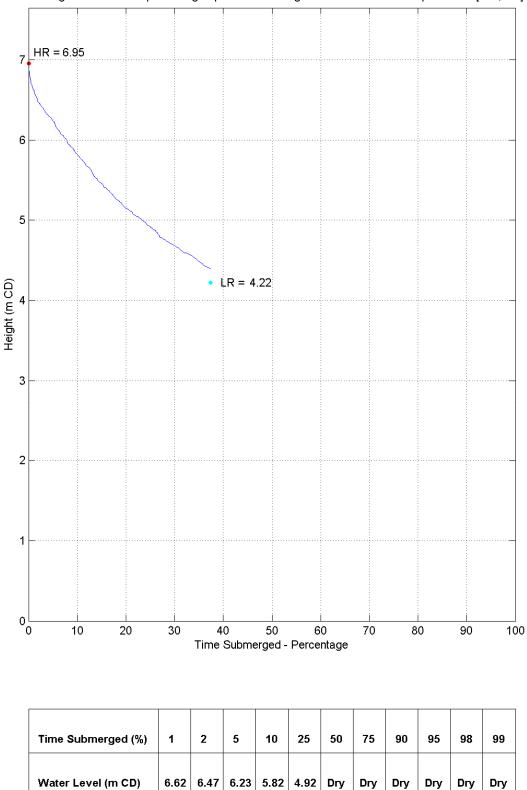
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [212,193]



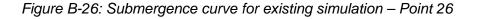


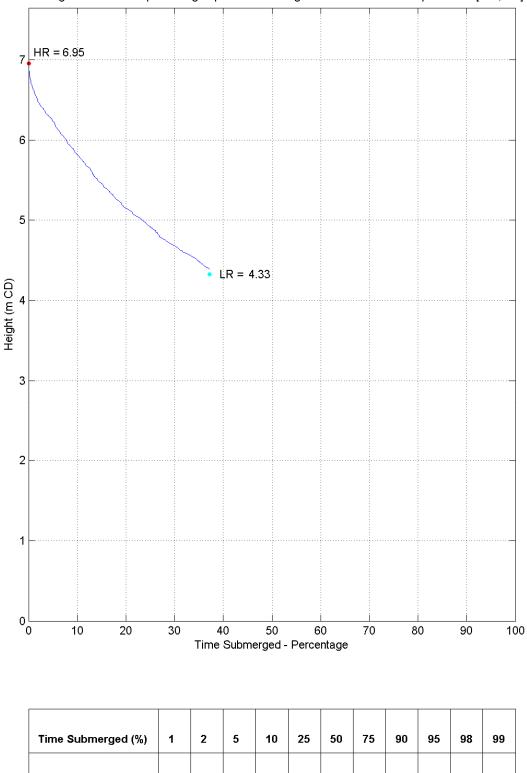
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [231,188]





Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [232,187]





Dry

Dry

Dry

Dry

Dry

Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [233,188]

Water Level (m CD)

6.62

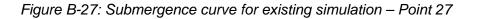
6.47

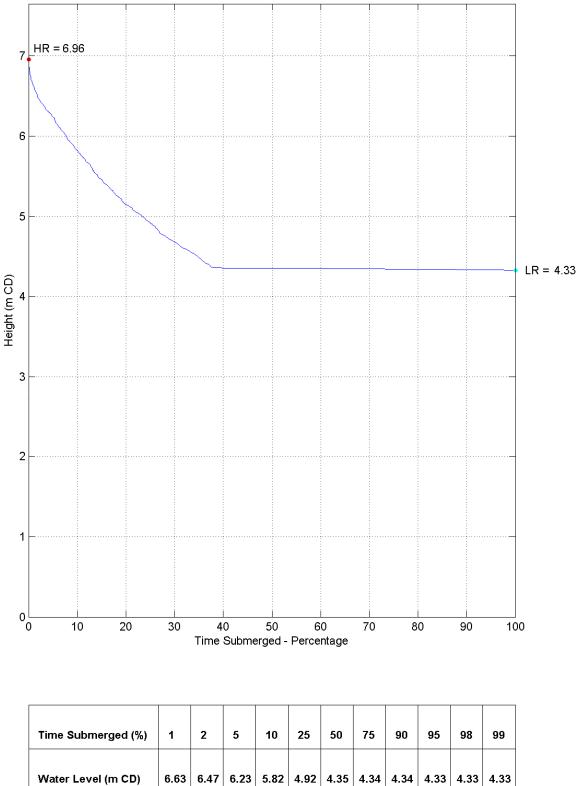
6.23

5.82

4.92

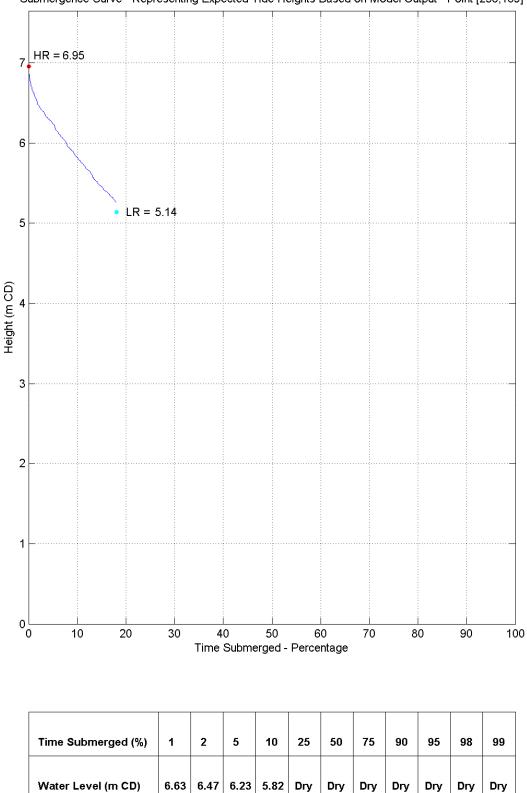
Dry





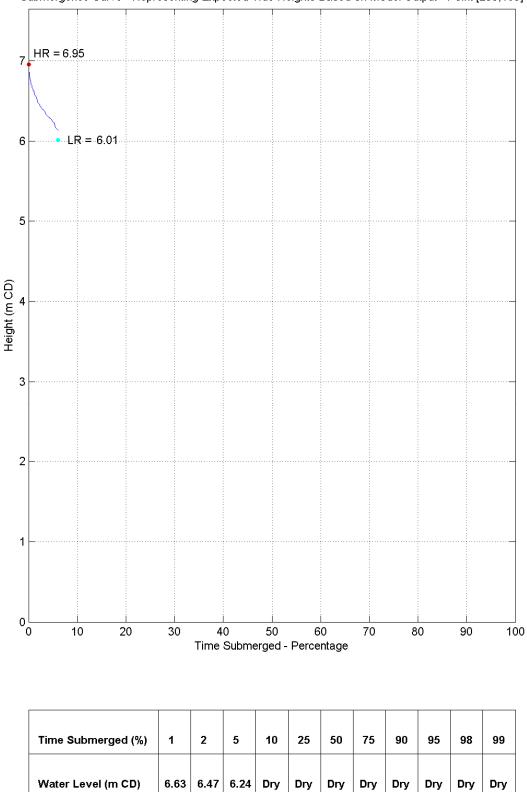
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [234,189]

Figure B-28: Submergence curve for existing simulation – Point 28



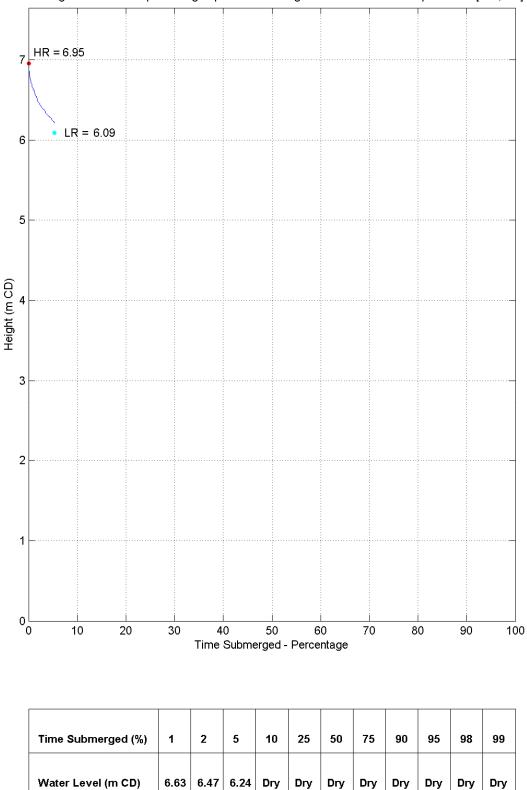
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [236,189]

Figure B-29: Submergence curve for existing simulation – Point 29



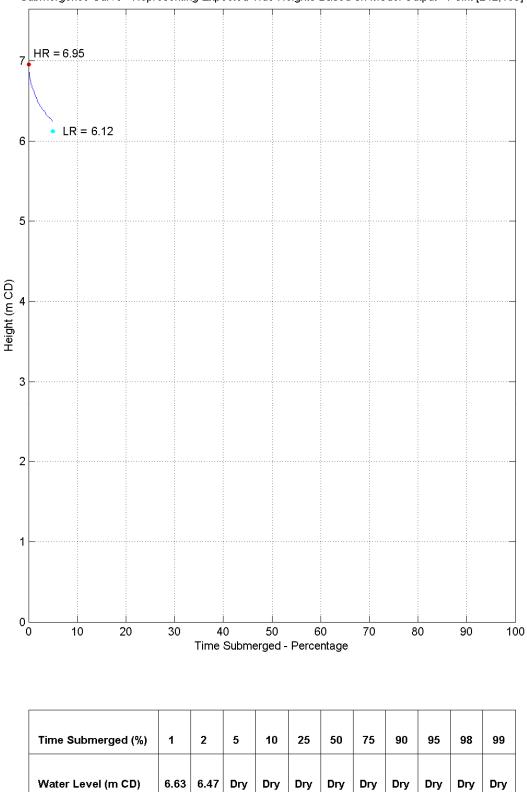
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [239,189]

Figure B-30: Submergence curve for existing simulation – Point 30

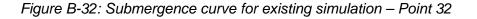


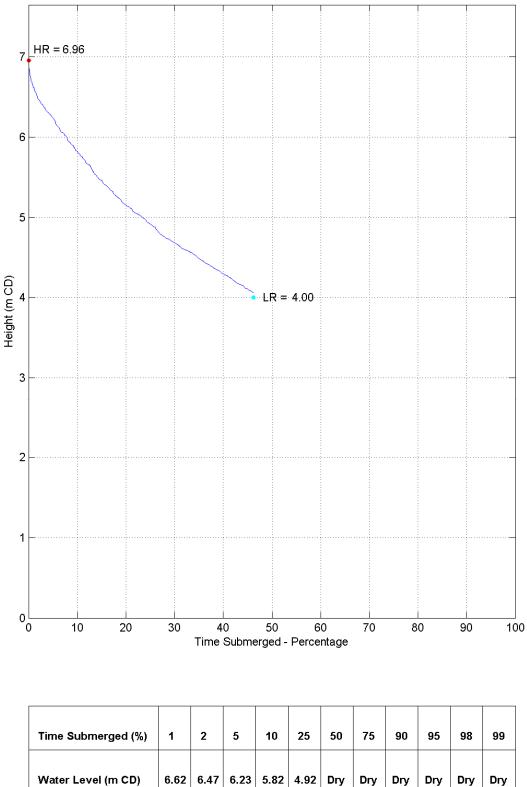
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [240,189]

Figure B-31: Submergence curve for existing simulation – Point 31

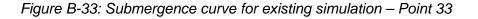


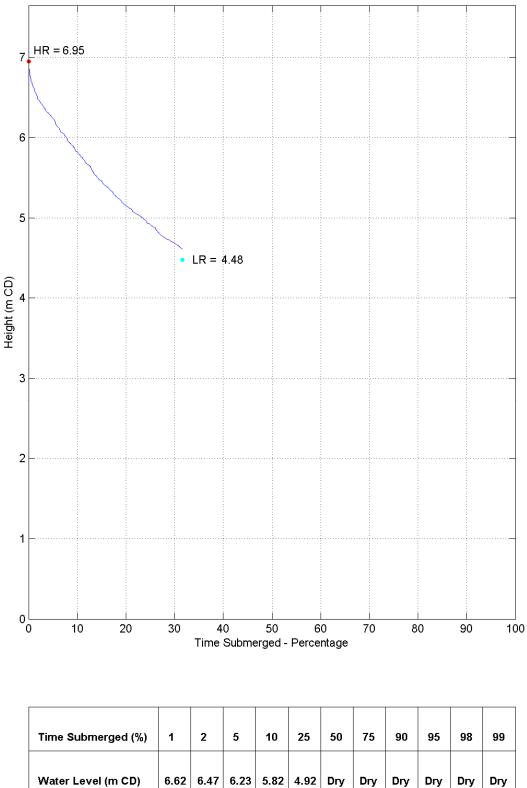
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [242,189]





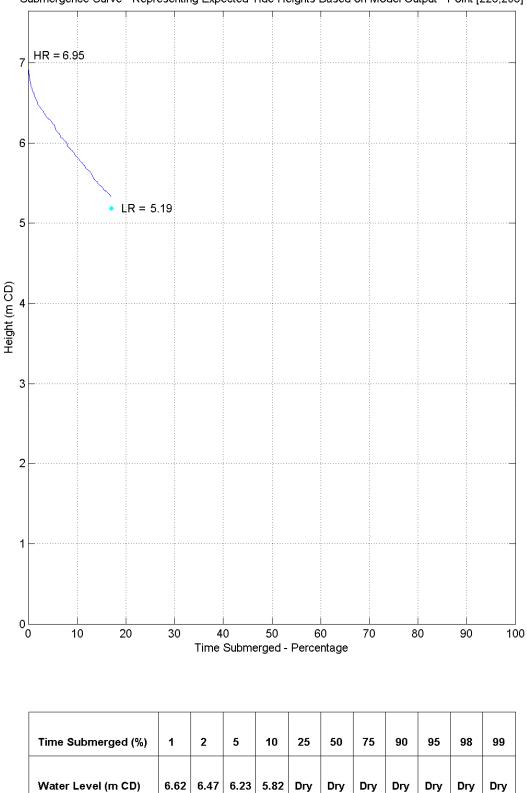
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [225,206]





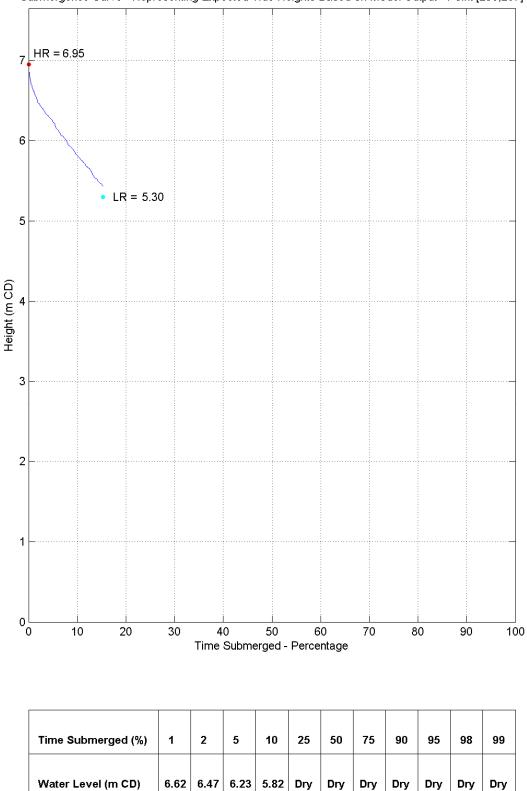
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [226,207]

Figure B-34: Submergence curve for existing simulation – Point 34



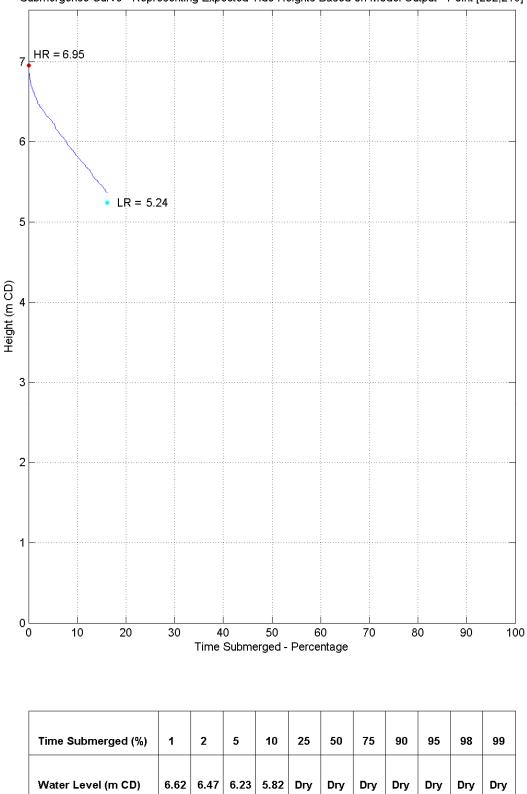
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [228,206]

Figure B-35: Submergence curve for existing simulation – Point 35



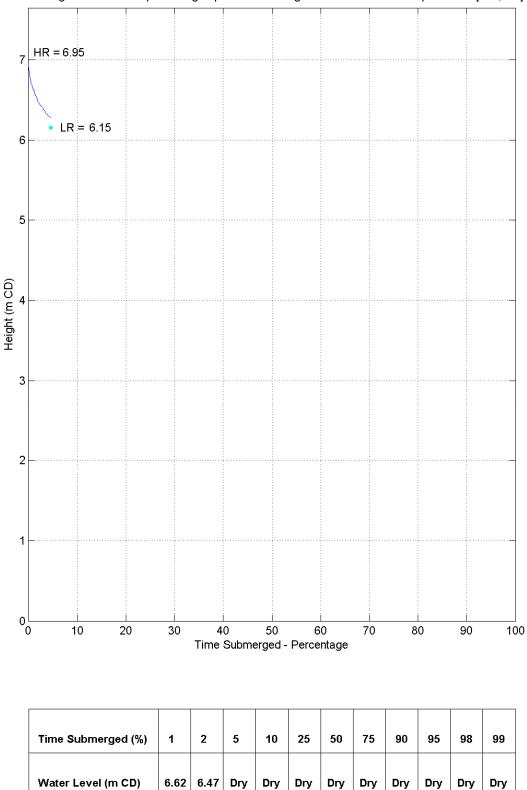
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [230,207]

Figure B-36: Submergence curve for existing simulation – Point 36



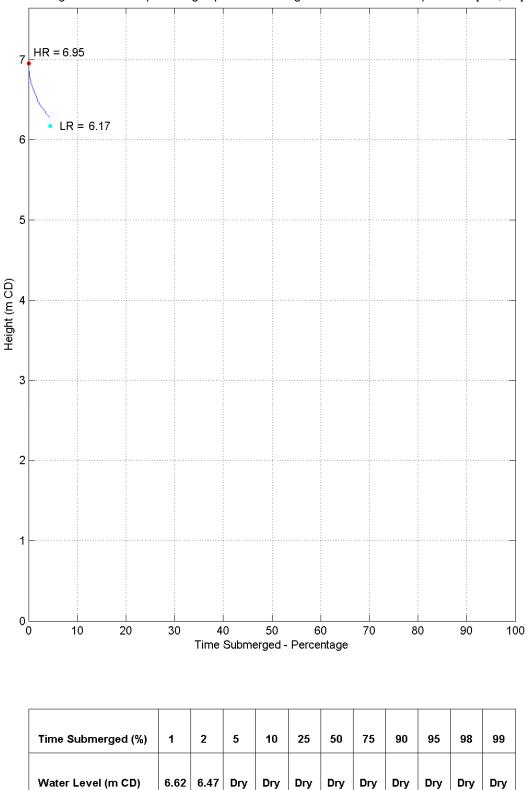
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [232,210]

Figure B-37: Submergence curve for existing simulation – Point 37



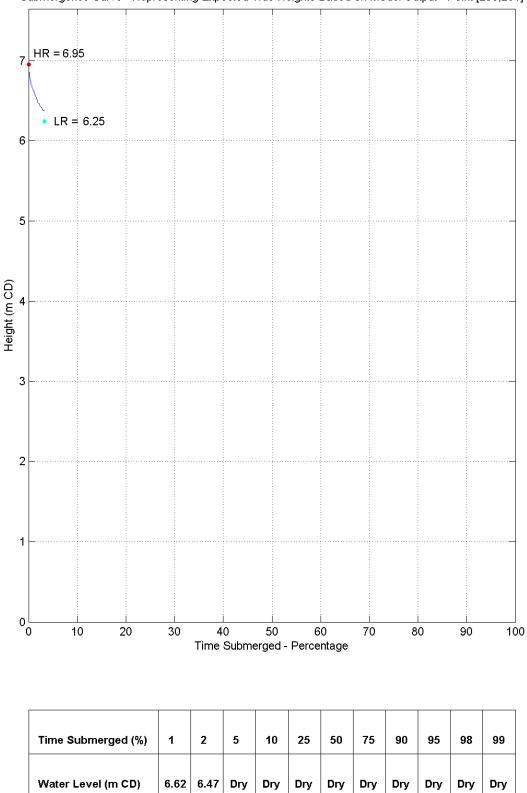
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [233,196]

Figure B-38: Submergence curve for existing simulation – Point 38



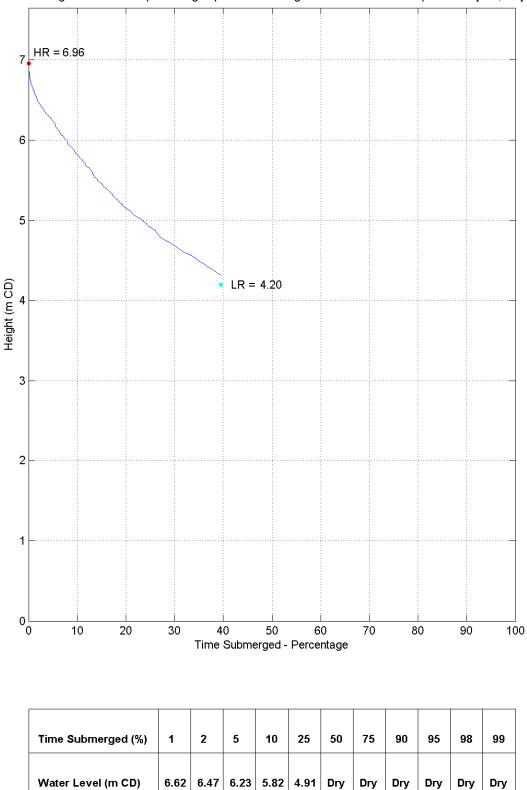
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [234,198]

Figure B-39: Submergence curve for existing simulation – Point 39

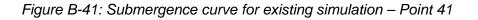


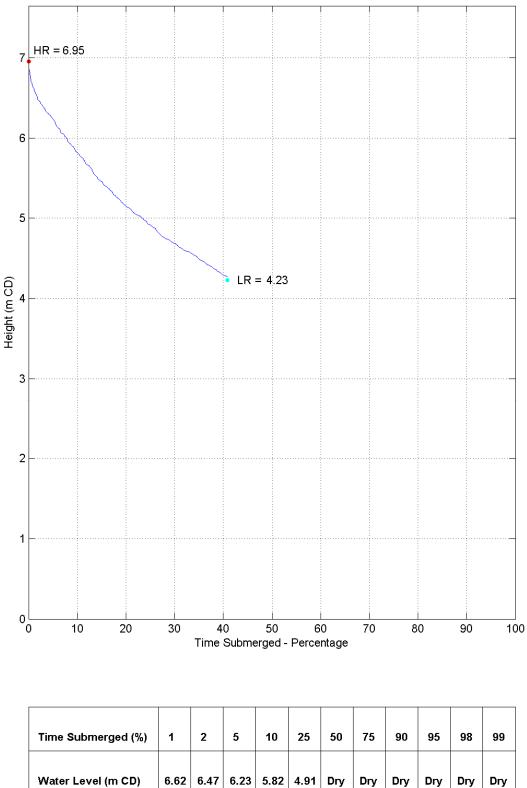
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [236,201]

Figure B-40: Submergence curve for existing simulation – Point 40

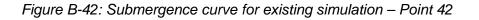


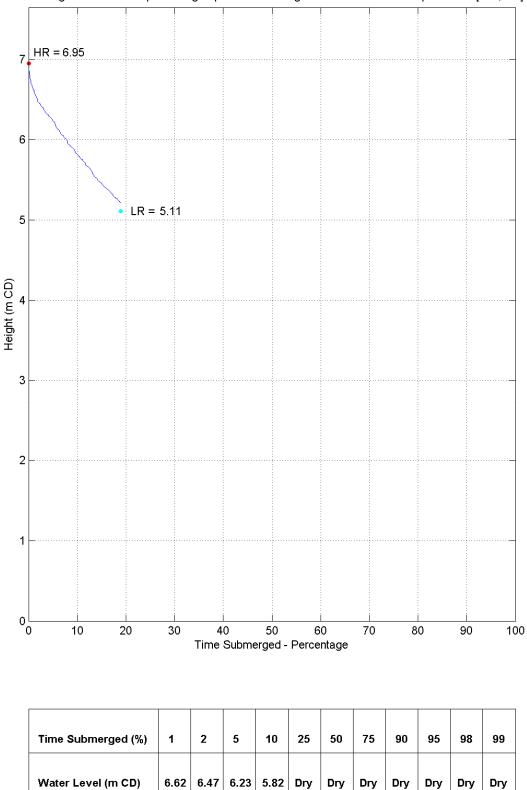
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [221,215]





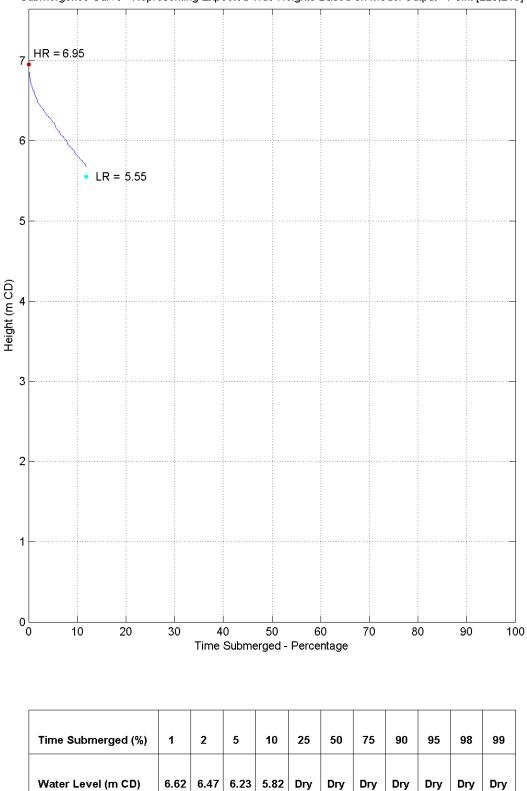
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [222,216]





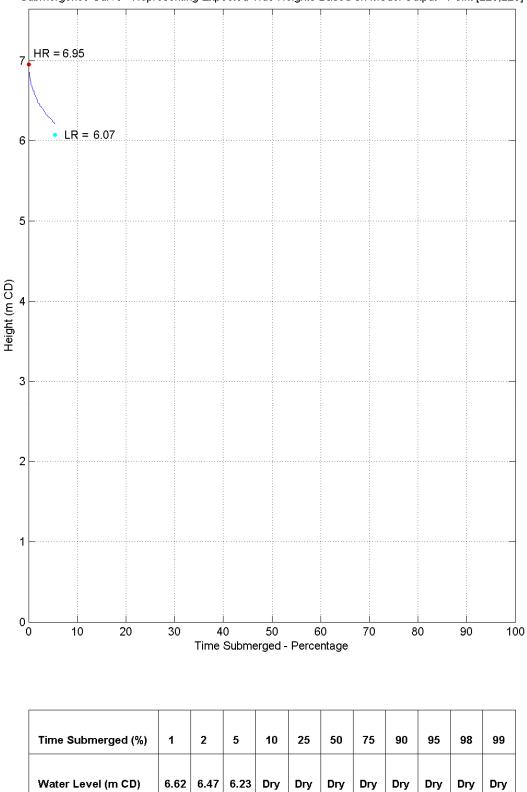
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [223,218]

Figure B-43: Submergence curve for existing simulation – Point 43



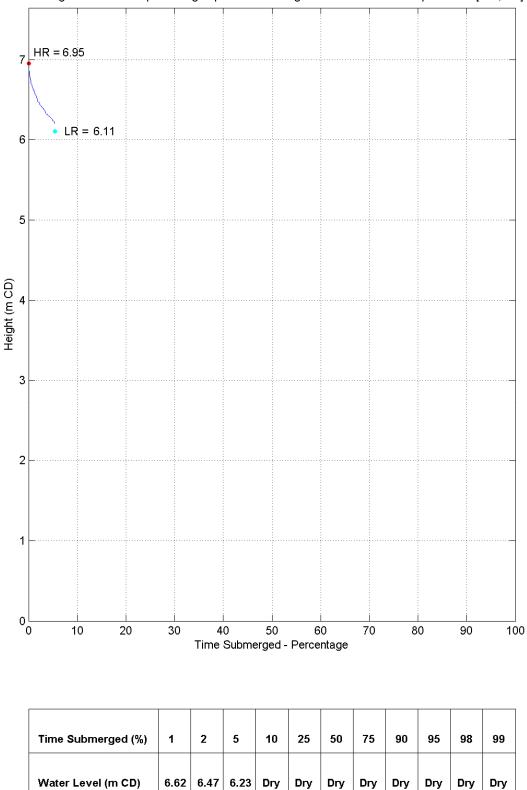
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [225,219]

Figure B-44: Submergence curve for existing simulation – Point 44



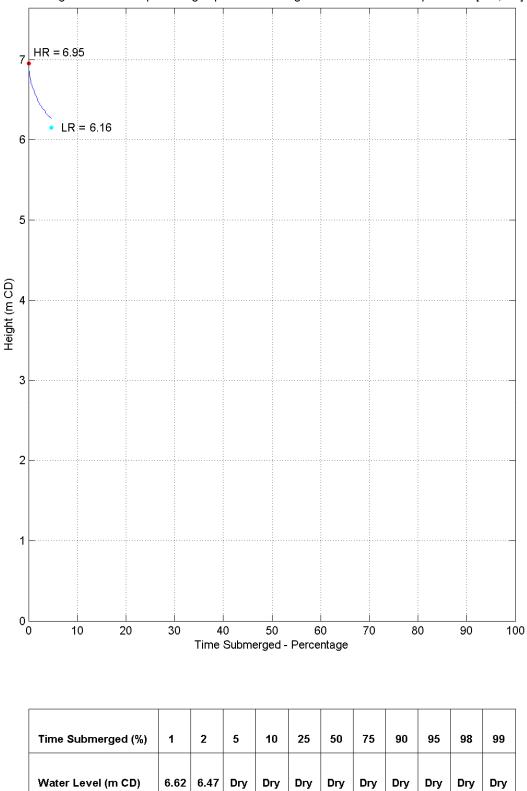
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [228,220]

Figure B-45: Submergence curve for existing simulation – Point 45



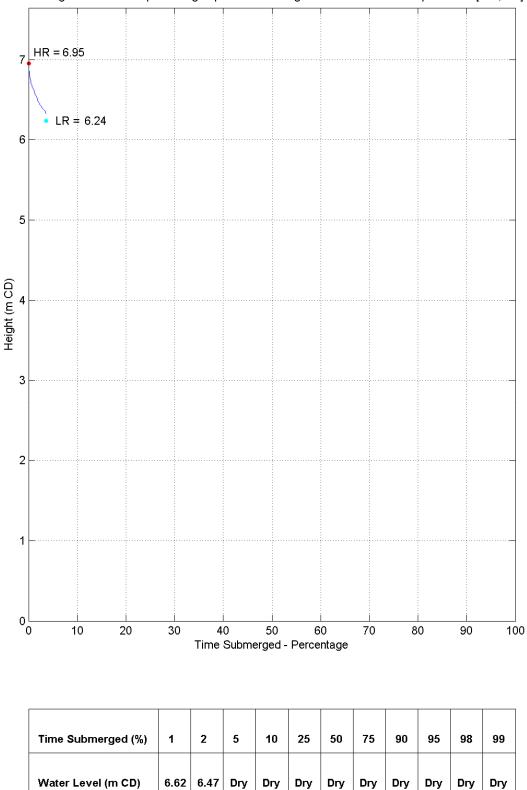
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [230,220]

Figure B-46: Submergence curve for existing simulation – Point 46



Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [232,220]

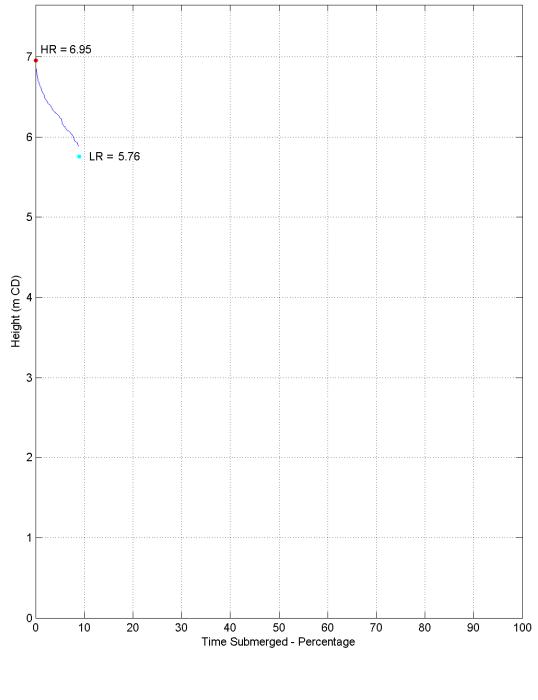
Figure B-47: Submergence curve for existing simulation – Point 47



Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [234,221]

Figure B-48: Submergence curve for existing simulation – Point 48

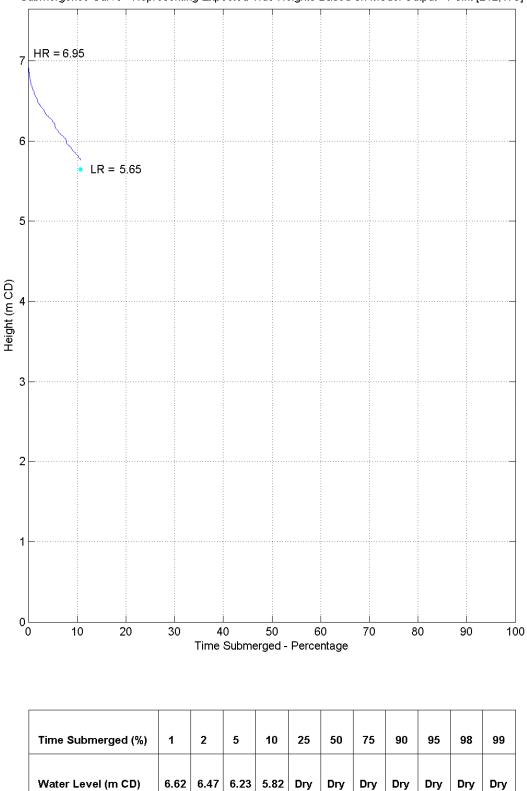
C. APPENDIX – SUBMERGENCE CURVES FOR 5 CULVERT SIMULATION

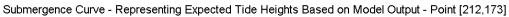


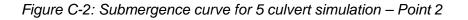
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [210,173]

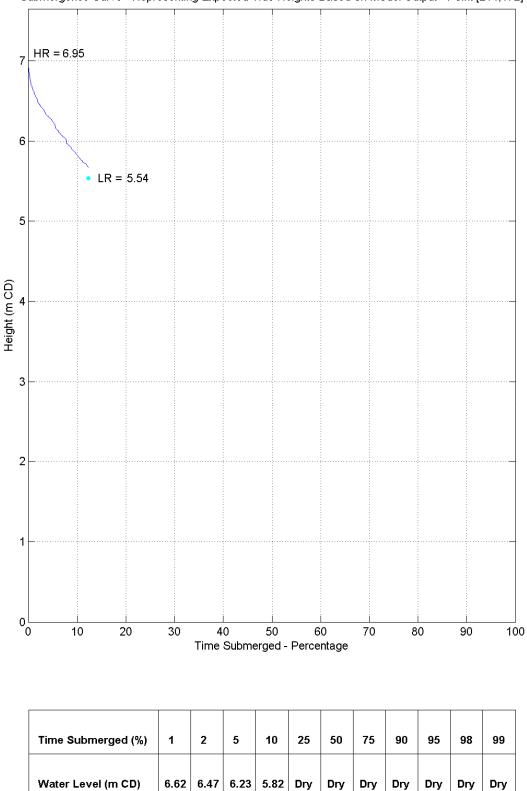
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Water Level (m CD)	6.62	6.47	6.23	Dry							

Figure C-1: Submergence curve for 5 culvert simulation – Point 1

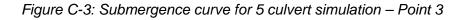


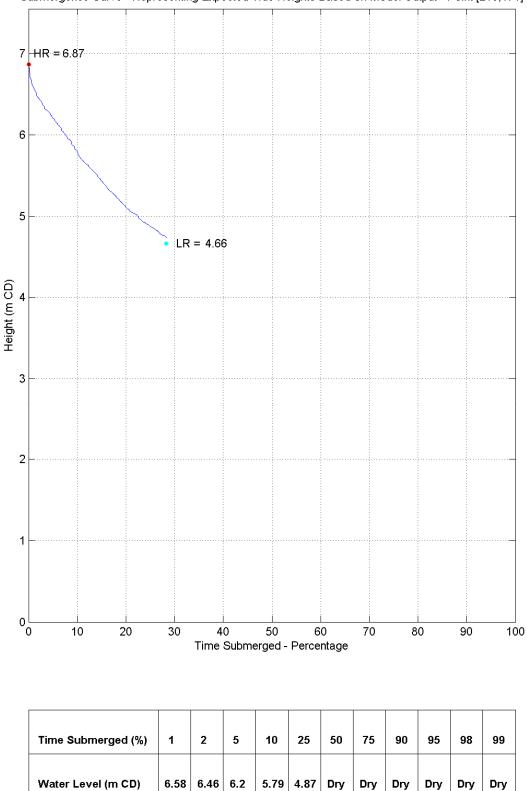




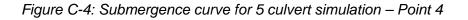


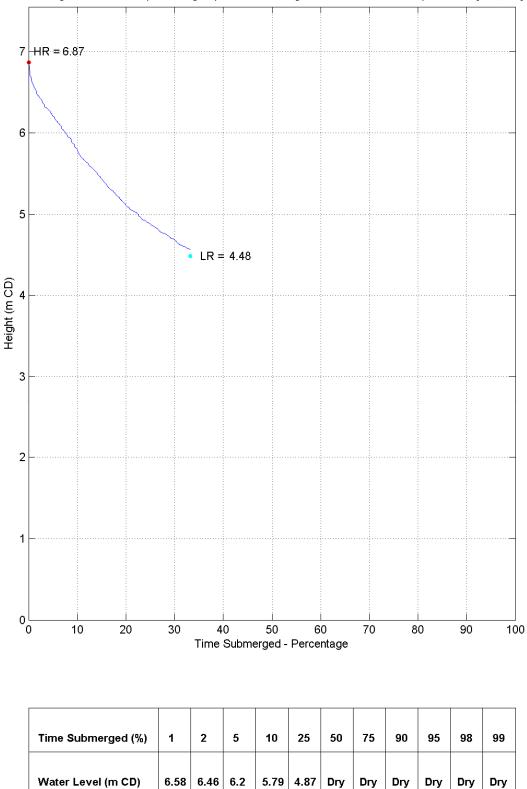
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [214,172]

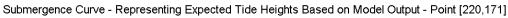


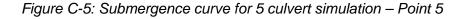


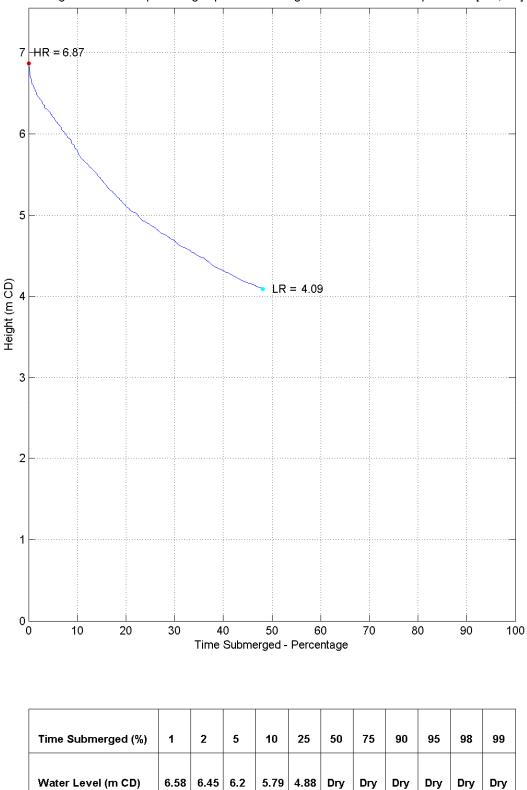
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [218,171]



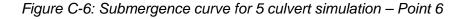


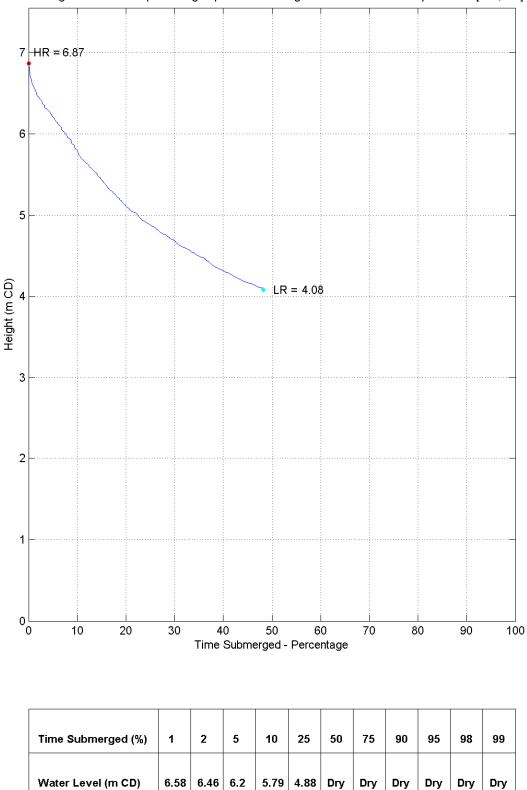




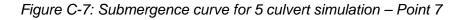


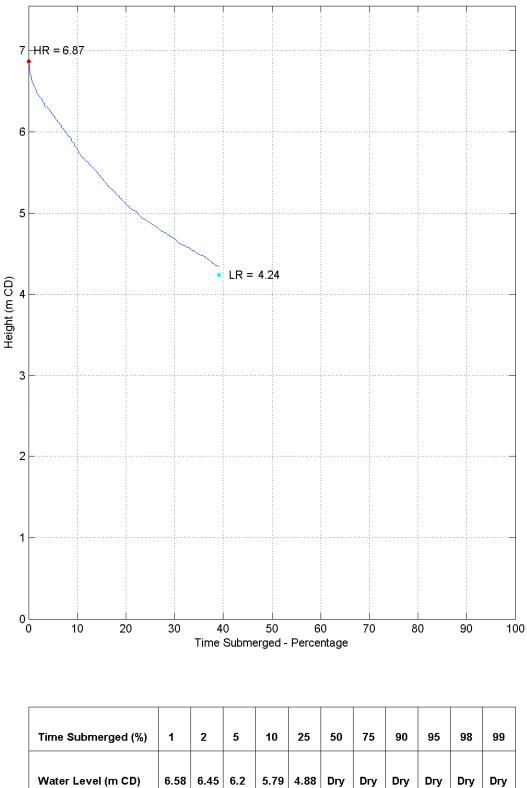
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [222,171]



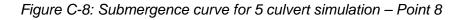


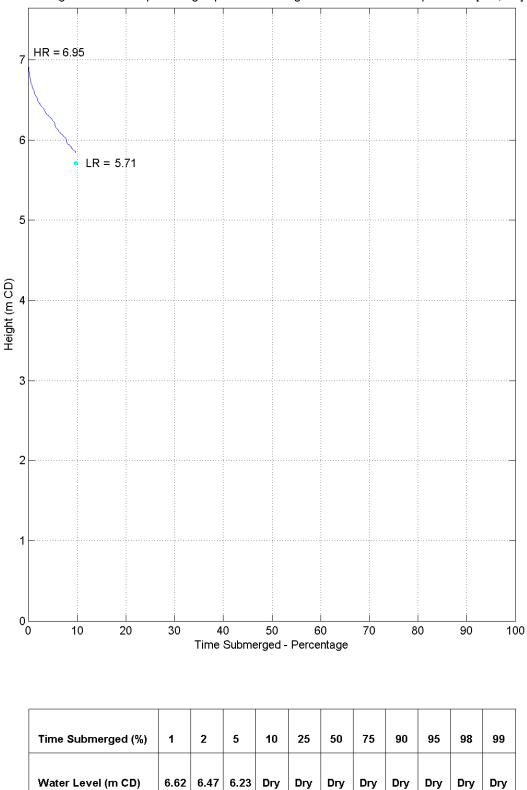
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [223,171]





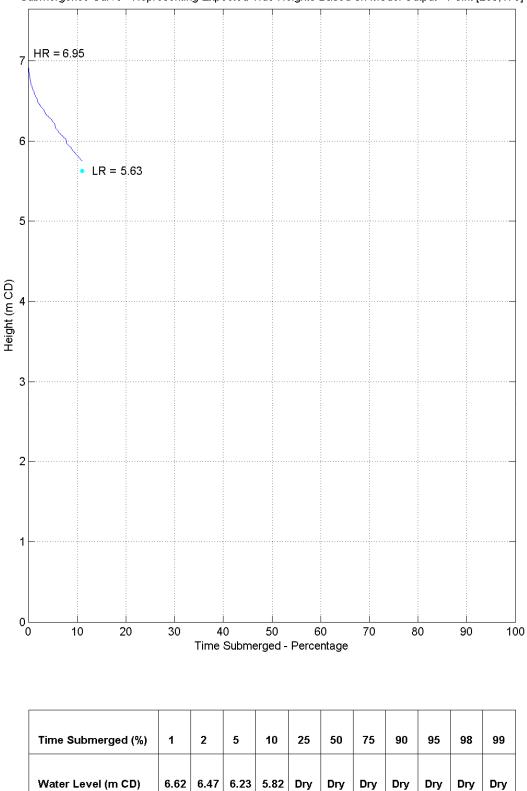
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [223,173]





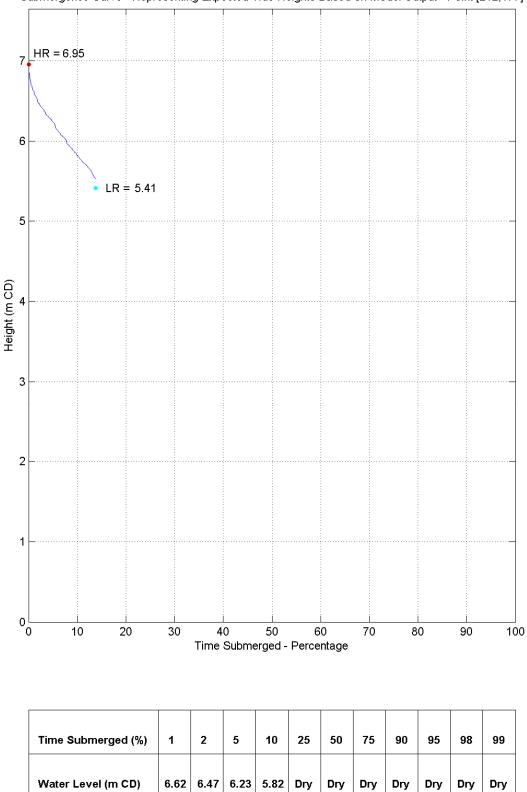
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [207,177]

Figure C-9: Submergence curve for 5 culvert simulation – Point 9



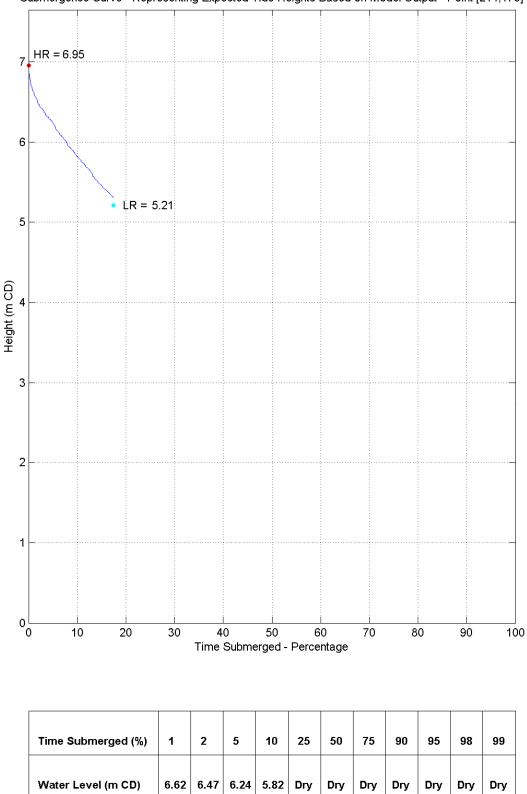
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [209,176]

Figure C-10: Submergence curve for 5 culvert simulation – Point 10



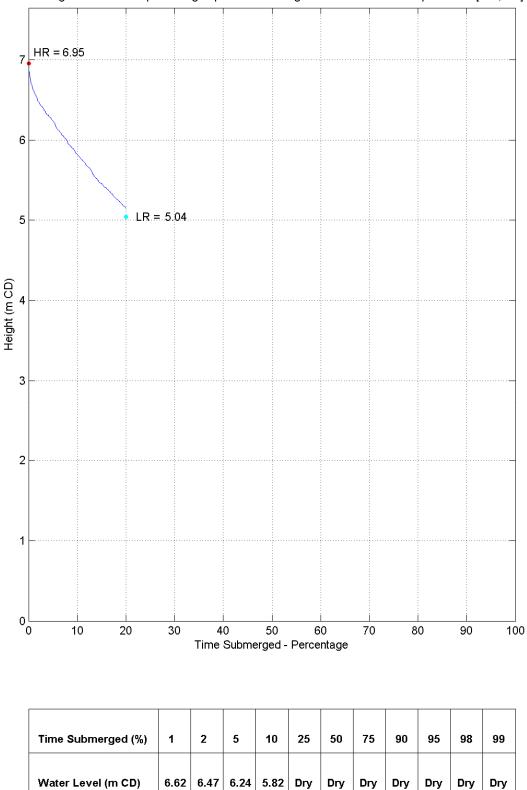
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [212,177]

Figure C-11: Submergence curve for 5 culvert simulation – Point 11



Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [214,178]

Figure C-12: Submergence curve for 5 culvert simulation – Point 12



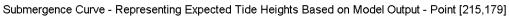
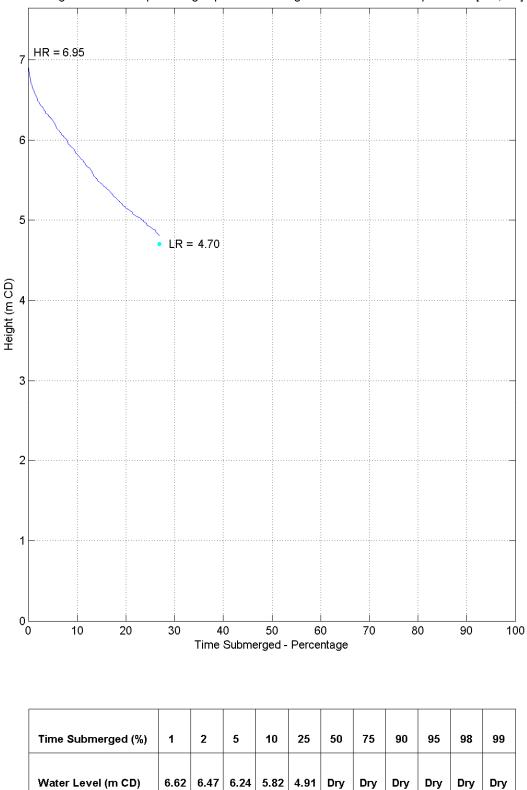
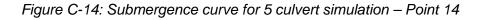
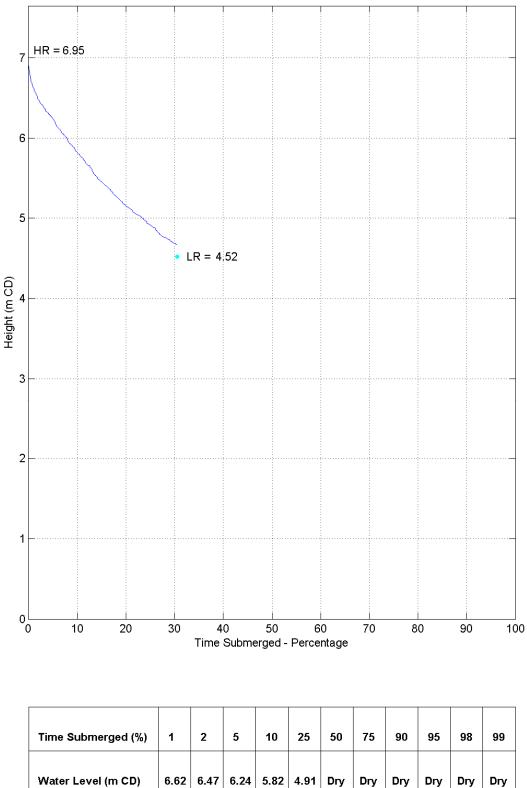


Figure C-13: Submergence curve for 5 culvert simulation – Point 13

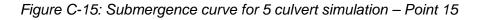


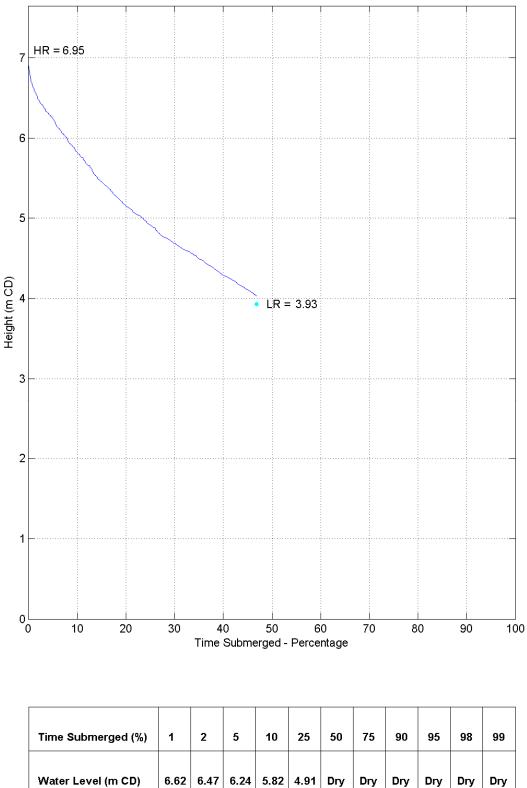
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [217,180]





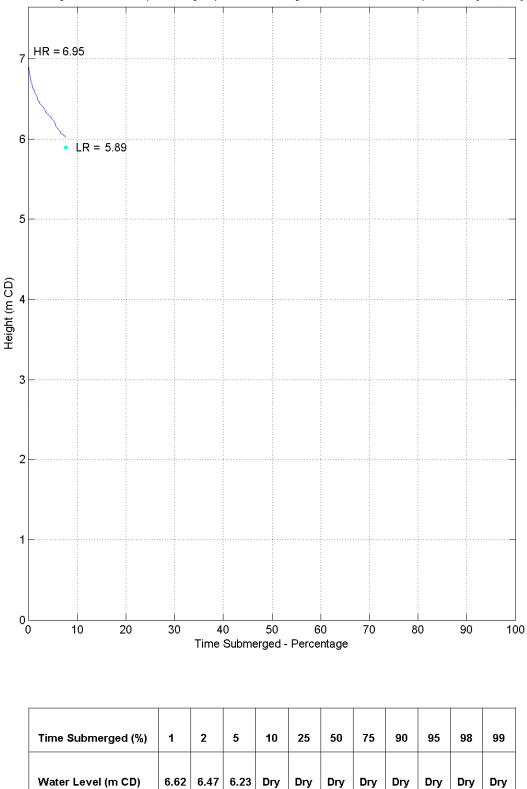
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [218,180]





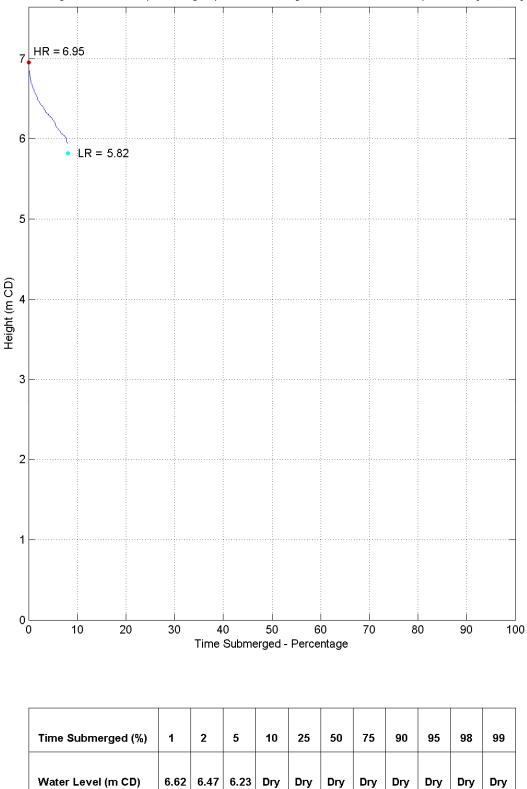
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [220,180]

Figure C-16: Submergence curve for 5 culvert simulation – Point 16



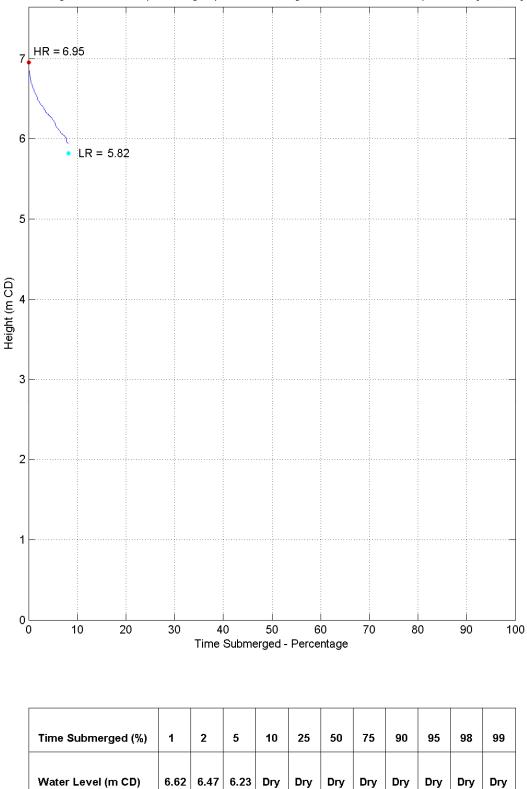
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [202,190]

Figure C-17: Submergence curve for 5 culvert simulation – Point 17



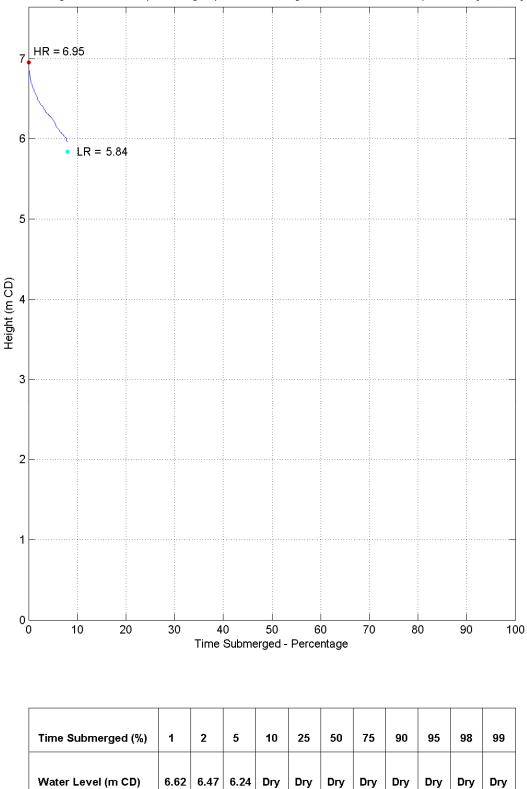
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [204,191]

Figure C-18: Submergence curve for 5 culvert simulation – Point 18



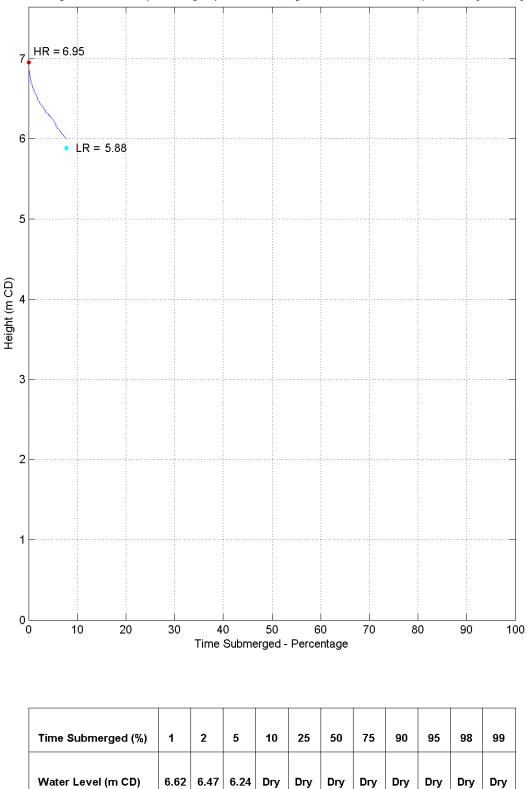
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [205,192]

Figure C-19: Submergence curve for 5 culvert simulation – Point 19



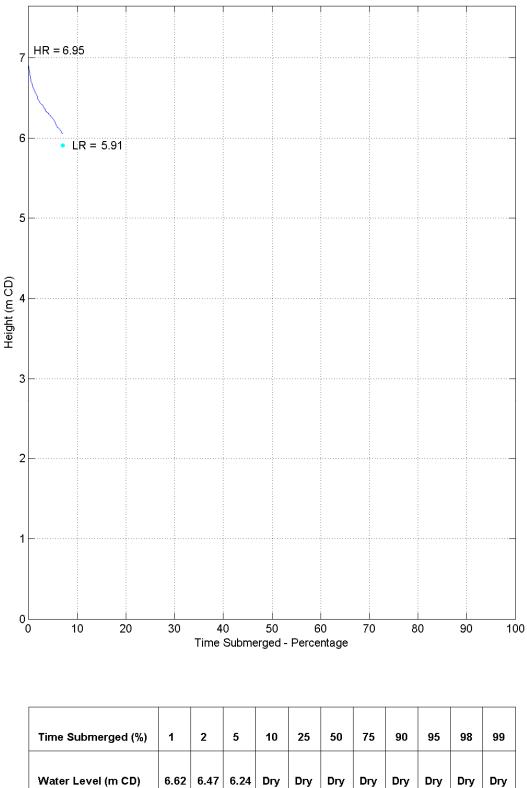
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [208,193]

Figure C-20: Submergence curve for 5 culvert simulation – Point 20

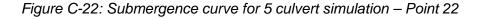


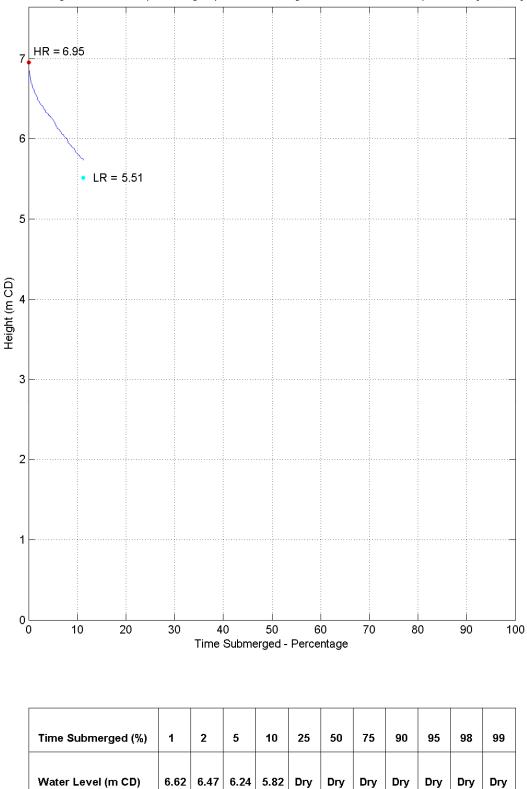
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [209,194]

Figure C-21: Submergence curve for 5 culvert simulation – Point 21

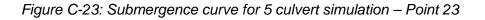


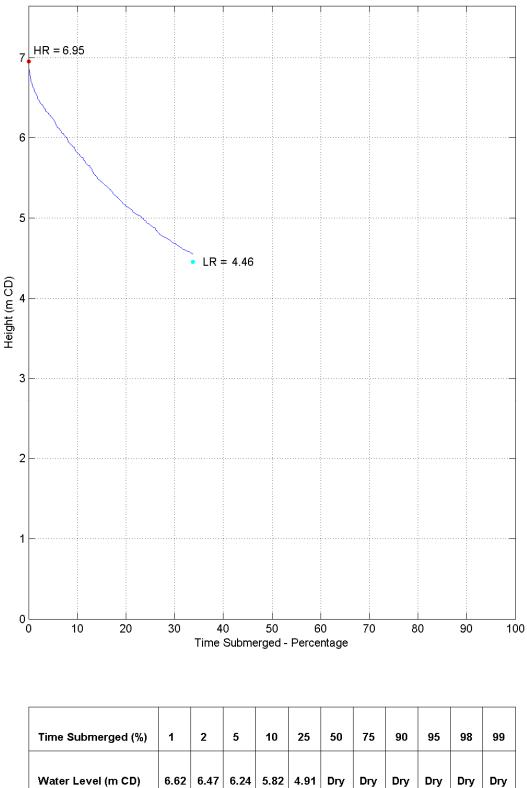
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [210,193]



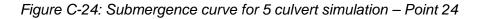


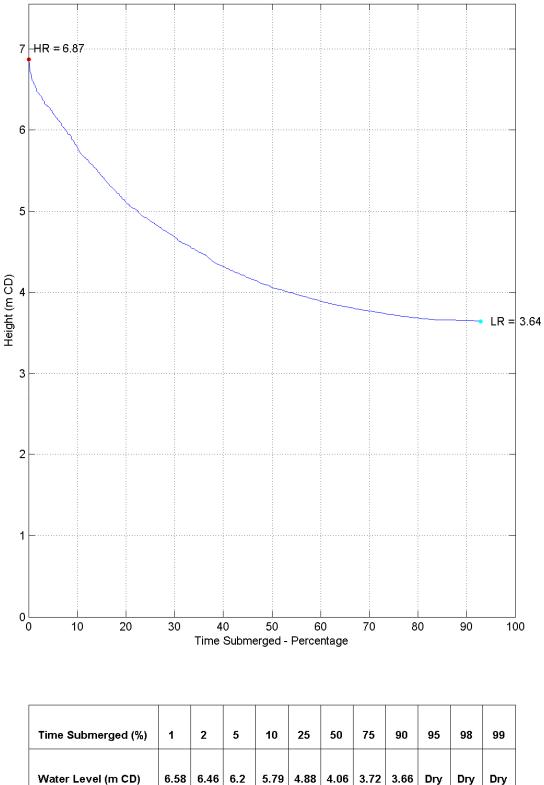
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [211,193]



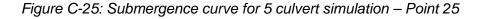


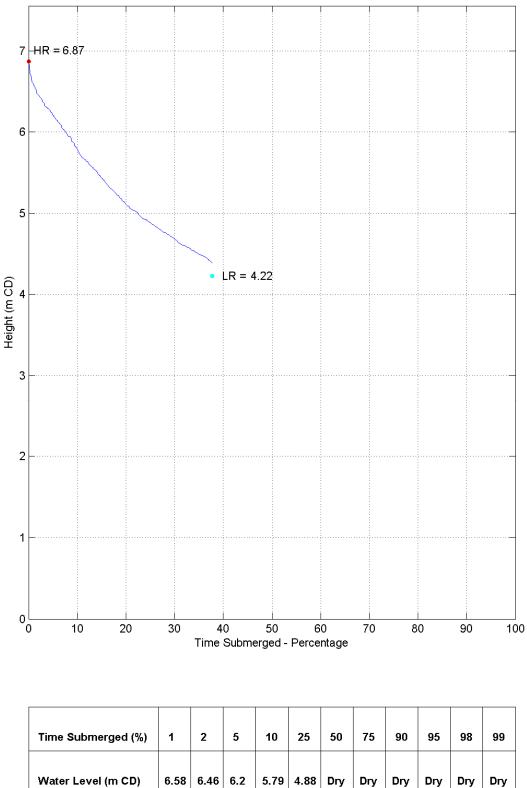
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [212,193]



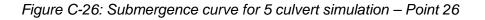


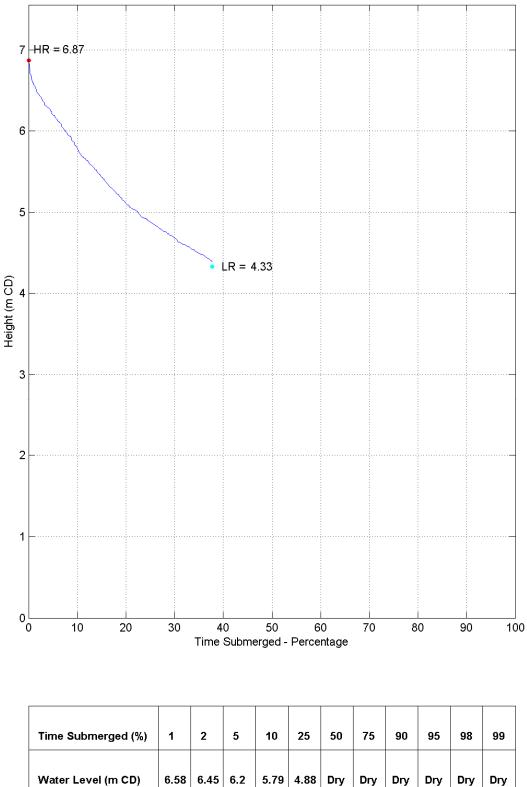
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [231,188]



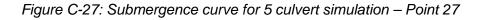


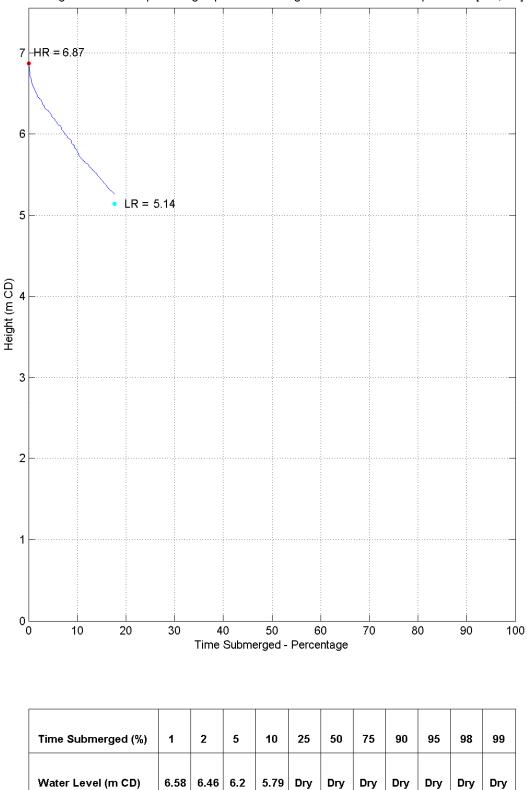
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [232,187]





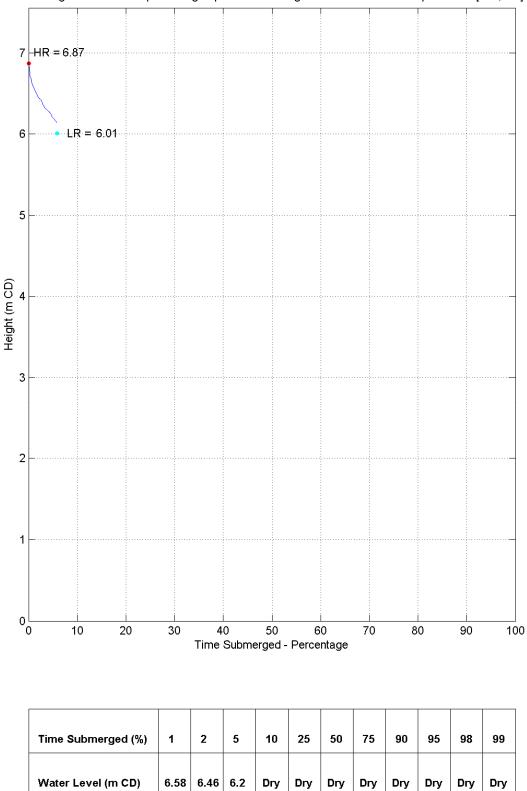
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [233,188]





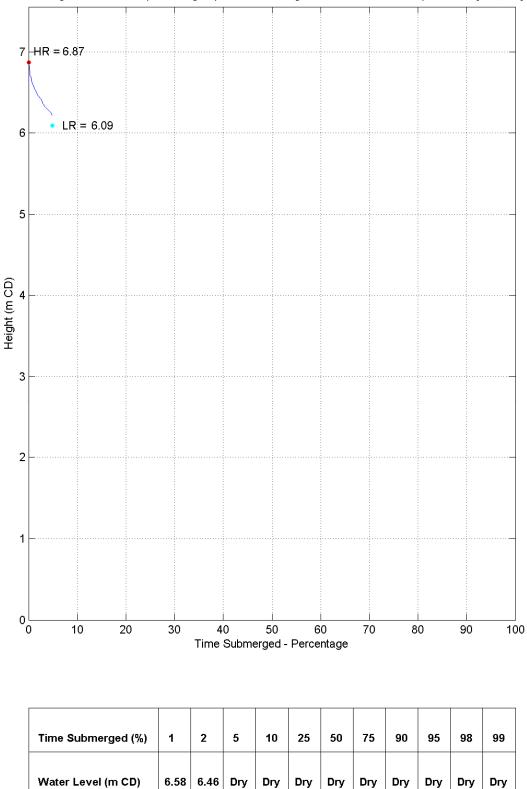
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [236,189]

Figure C-28: Submergence curve for 5 culvert simulation – Point 28



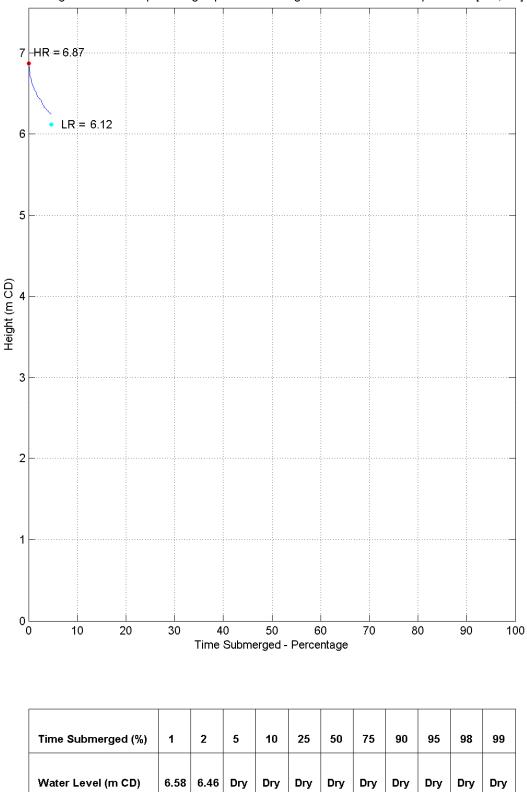
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [239,189]

Figure C-29: Submergence curve for 5 culvert simulation – Point 29

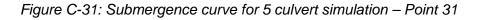


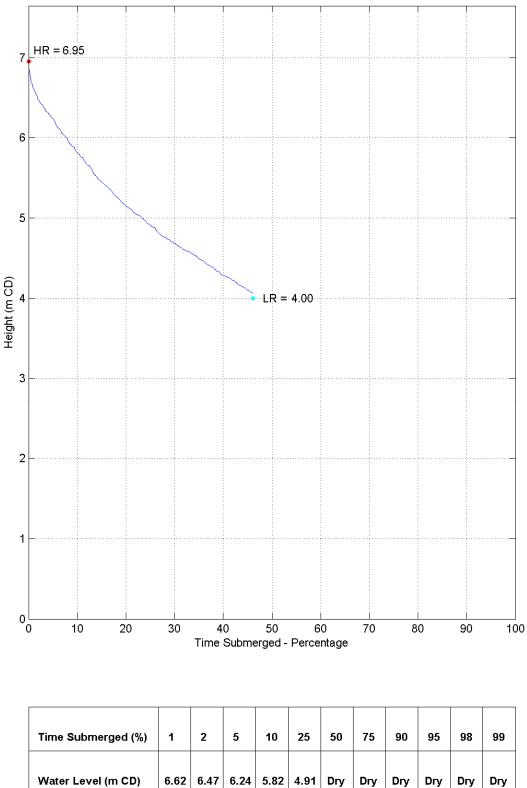
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [240,189]

Figure C-30: Submergence curve for 5 culvert simulation – Point 30

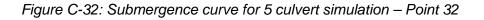


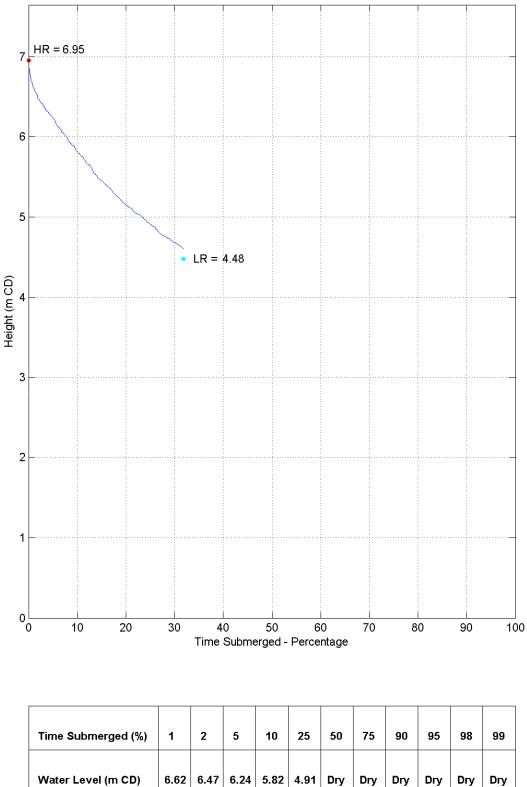
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [242,189]





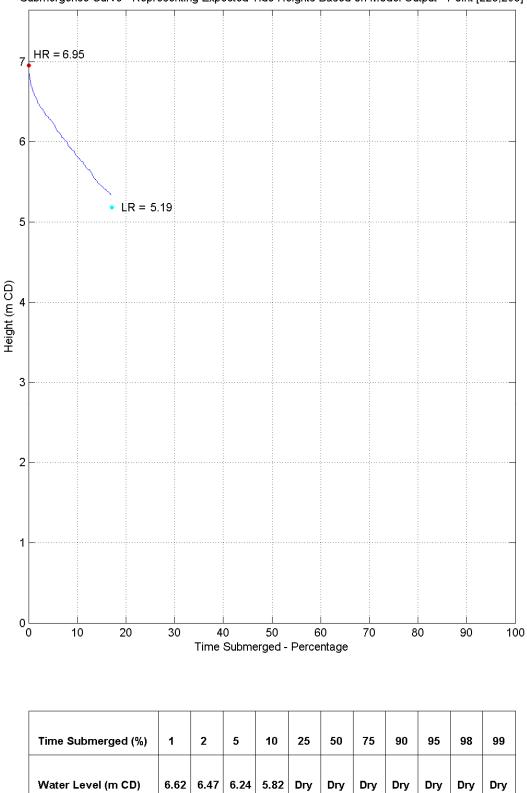
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [225,206]



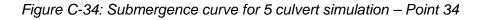


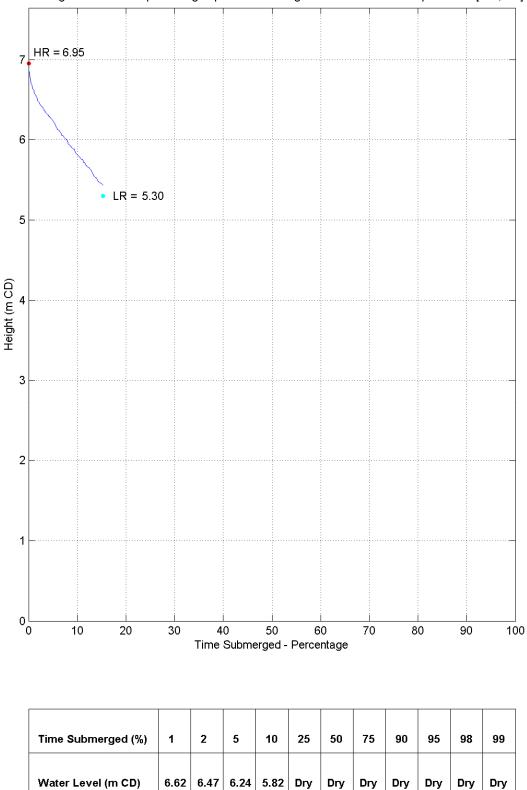
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [226,207]

Figure C-33: Submergence curve for 5 culvert simulation – Point 33

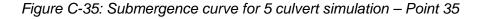


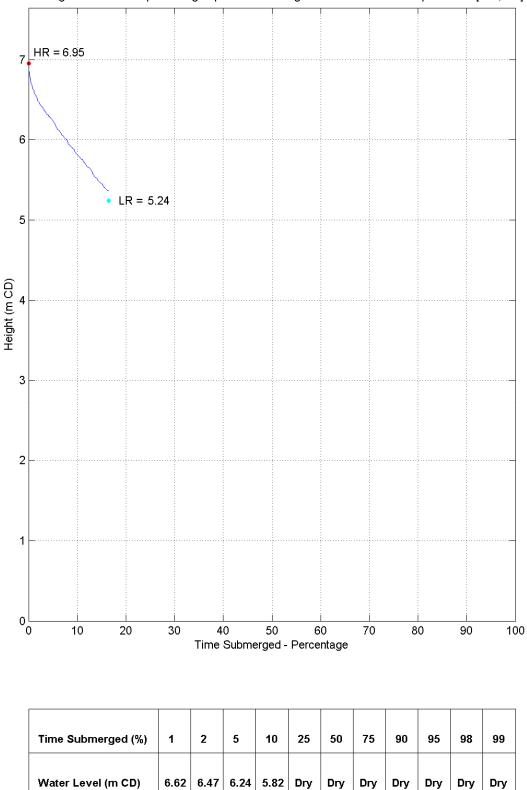
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [228,206]





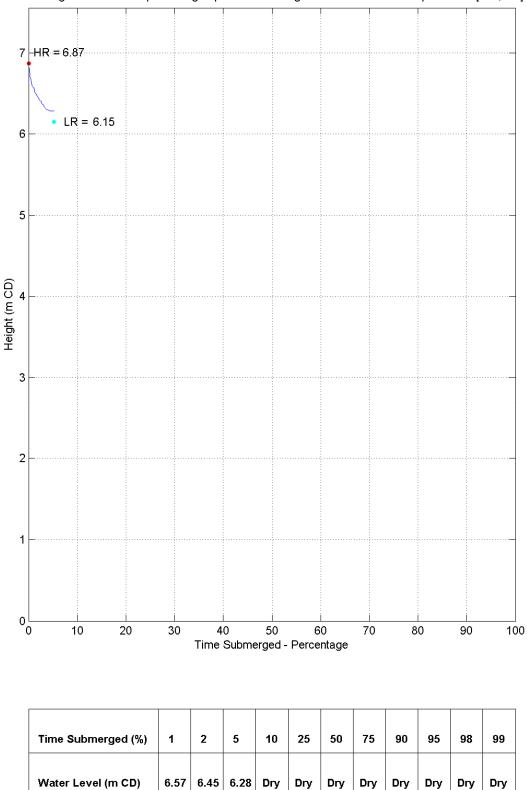
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [230,207]





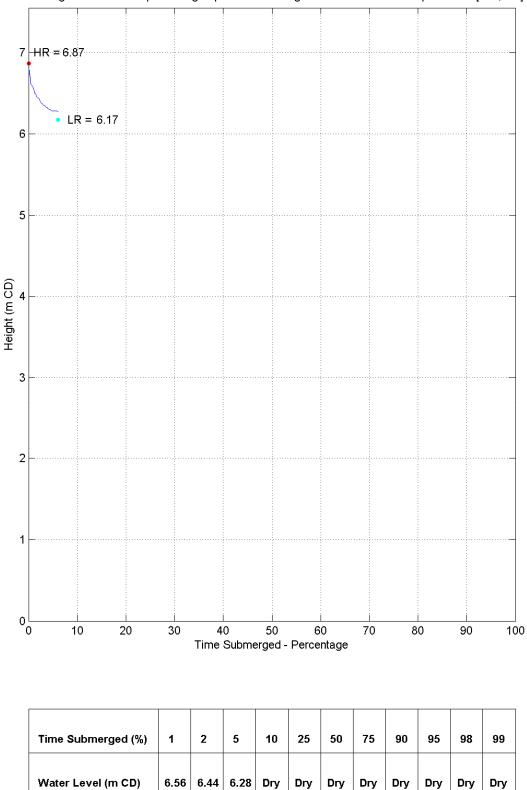
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [232,210]

Figure C-36: Submergence curve for 5 culvert simulation – Point 36



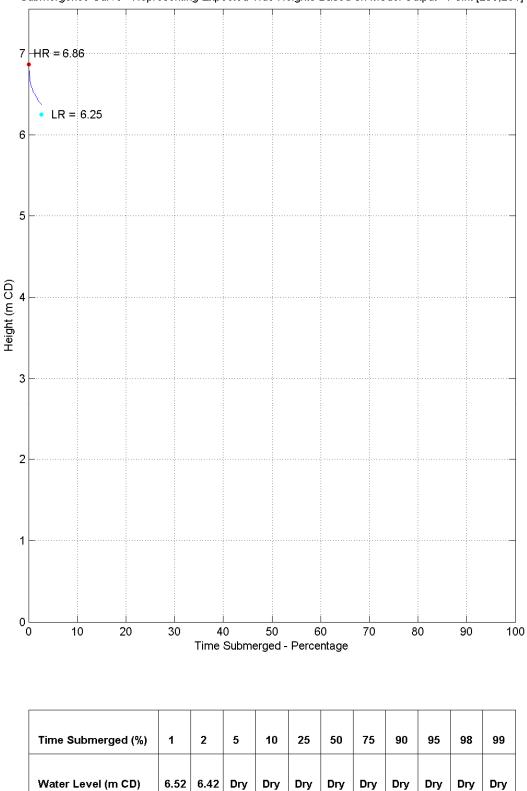
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [233,196]

Figure C-37: Submergence curve for 5 culvert simulation – Point 37

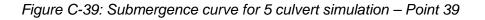


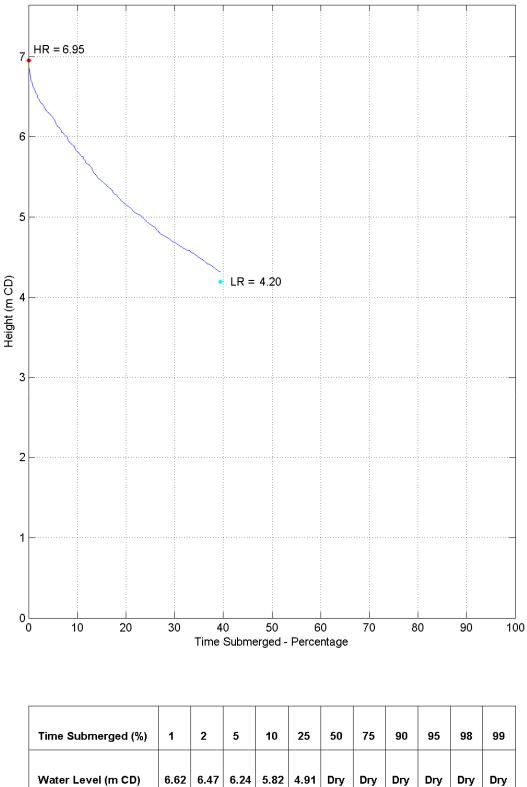
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [234,198]

Figure C-38: Submergence curve for 5 culvert simulation – Point 38



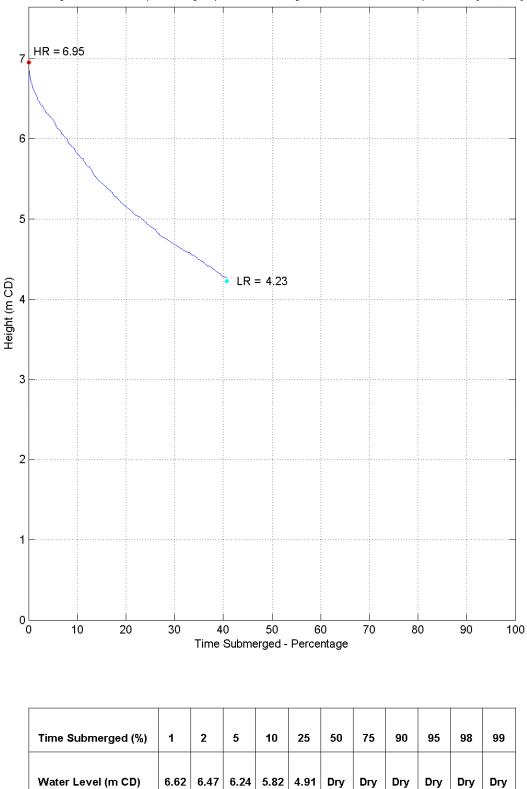
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [236,201]



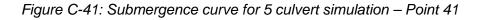


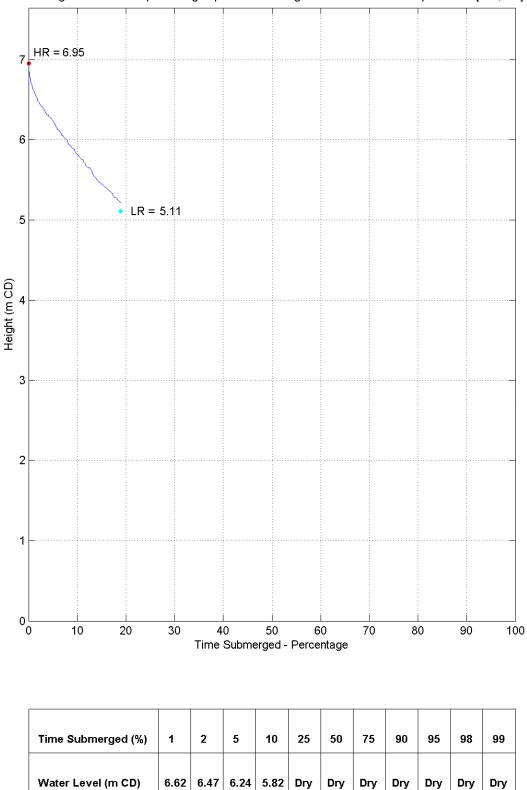
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [221,215]

Figure C-40: Submergence curve for 5 culvert simulation – Point 40



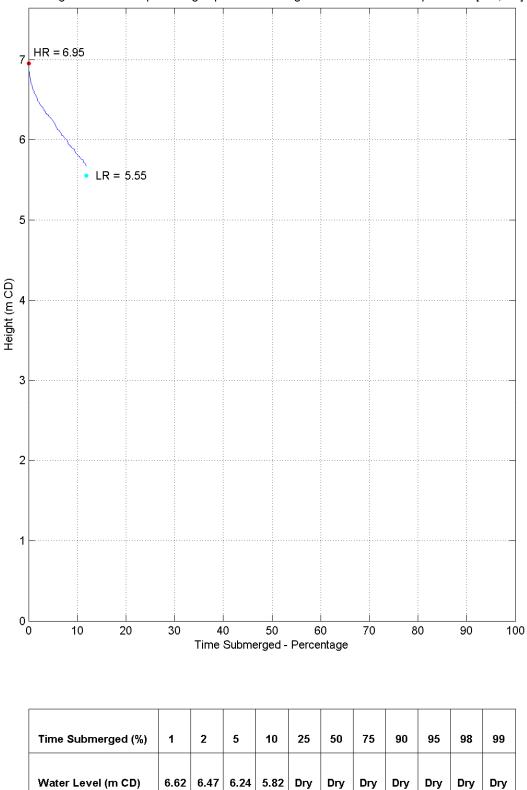
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [222,216]





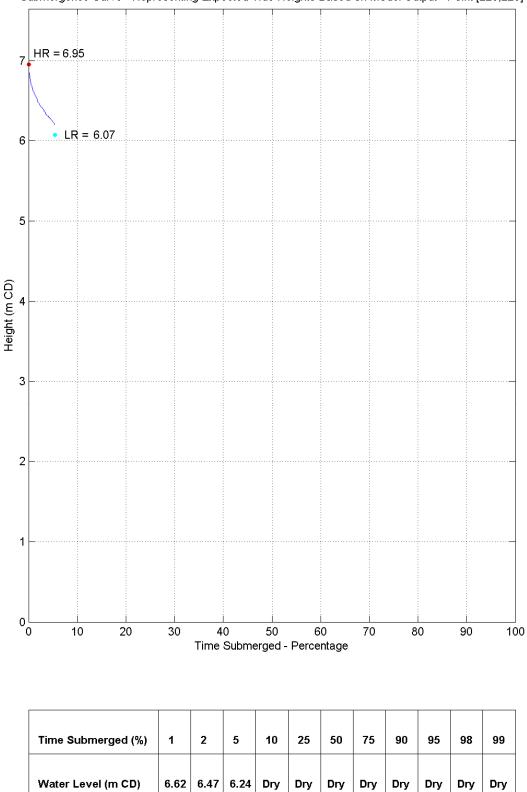
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [223,218]

Figure C-42: Submergence curve for 5 culvert simulation – Point 42



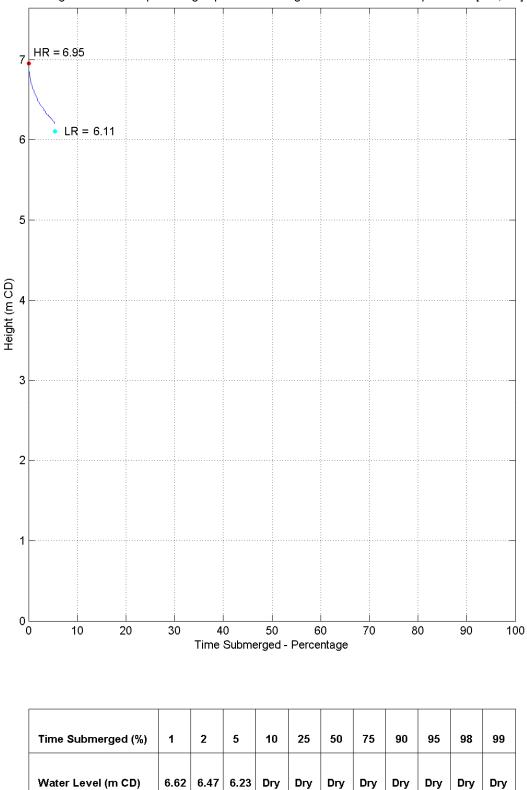
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [225,219]

Figure C-43: Submergence curve for 5 culvert simulation – Point 43



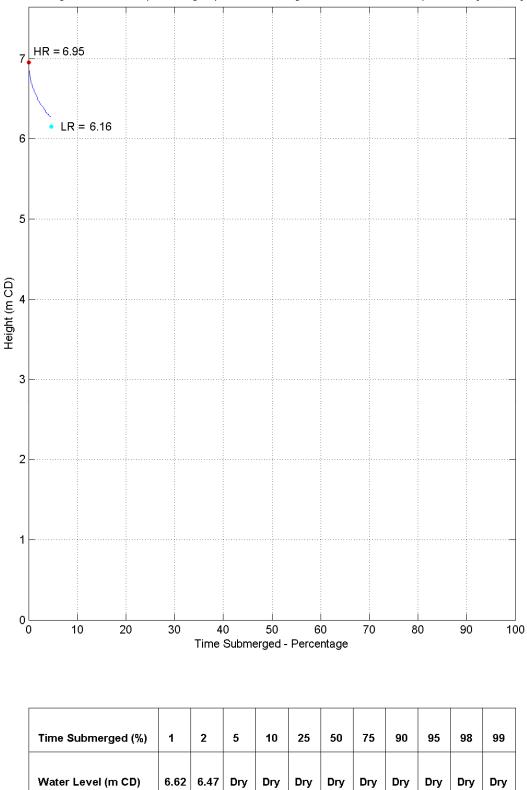
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [228,220]

Figure C-44: Submergence curve for 5 culvert simulation – Point 44



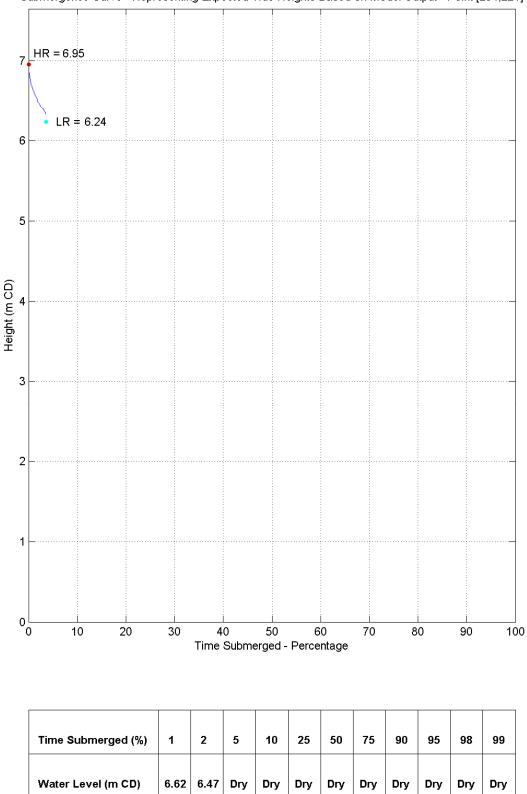
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [230,220]

Figure C-45: Submergence curve for 5 culvert simulation – Point 45

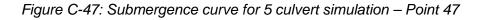


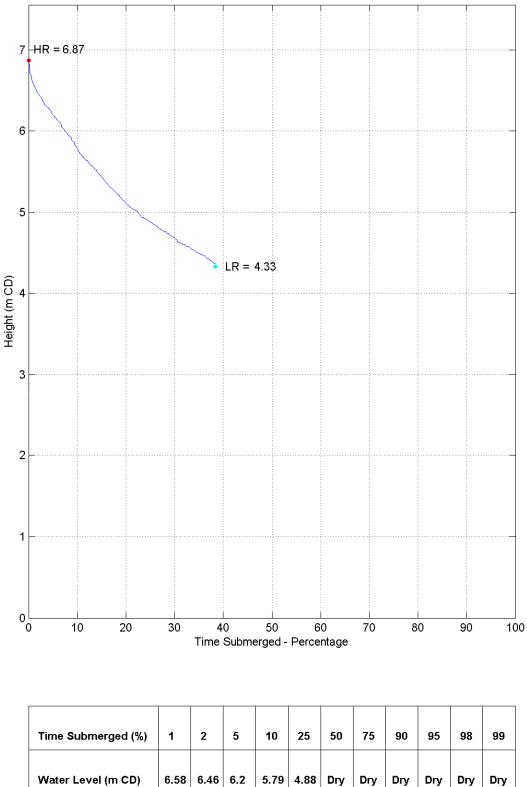
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [232,220]

Figure C-46: Submergence curve for 5 culvert simulation – Point 46



Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [234,221]

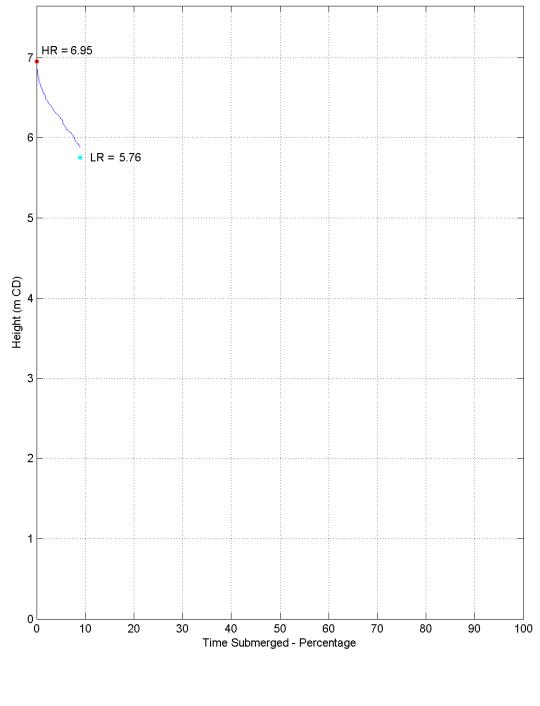




Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [234,189]

Figure C-48: Submergence curve for 5 culvert simulation – Point 48

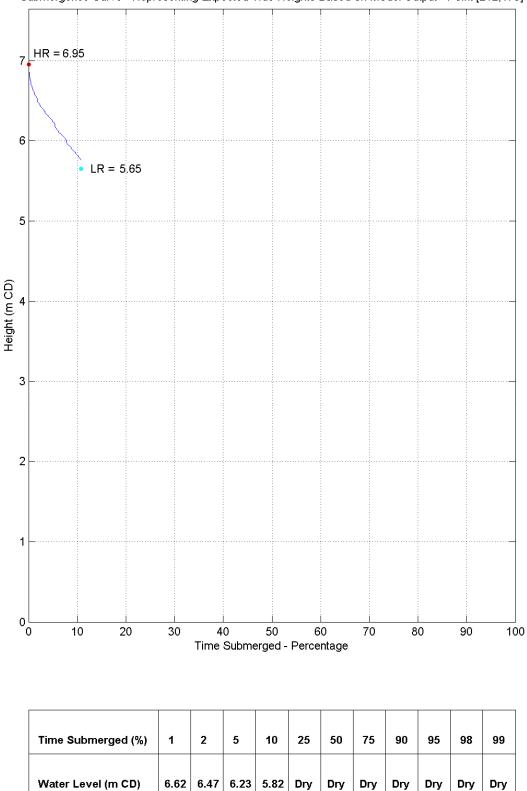
D. APPENDIX - SUBMERGENCE CURVES FOR 10 CULVERT SIMULATION



Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [210,173]

Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Water Level (m CD)	6.62	6.47	6.23	Dry							

Figure D-1: Submergence curve for 10 culvert simulation – Point 1



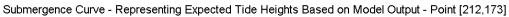
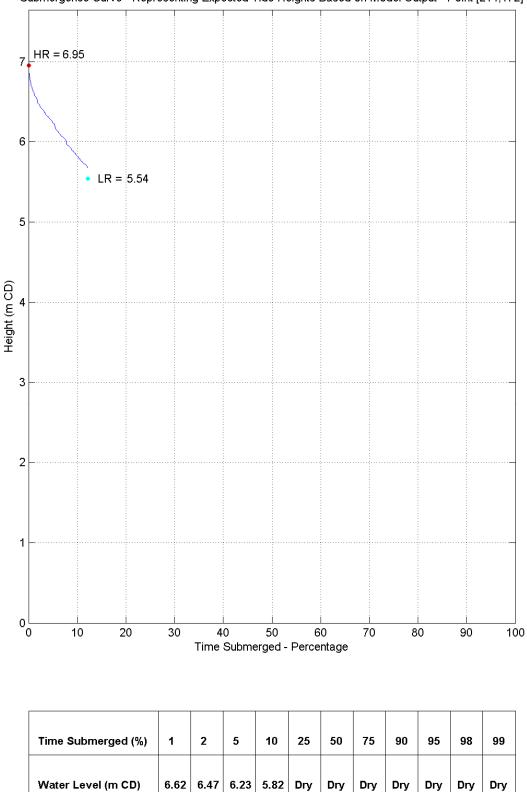
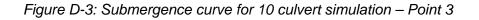
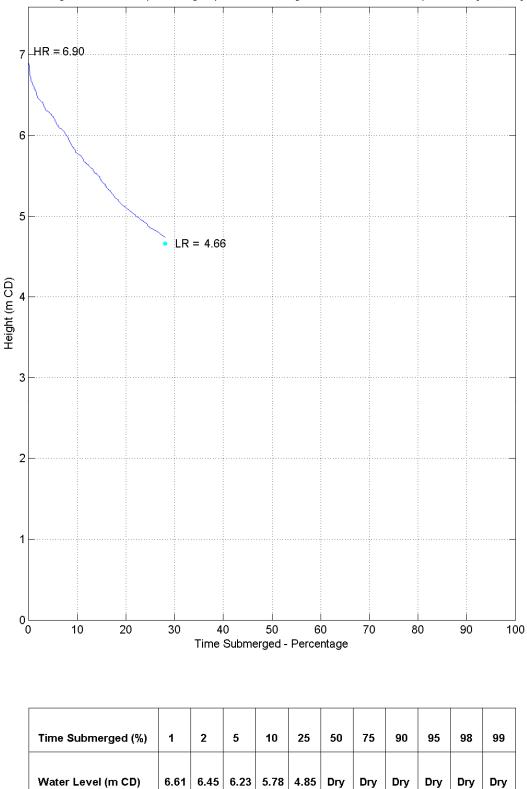


Figure D-2: Submergence curve for 10 culvert simulation – Point 2

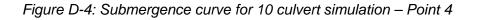


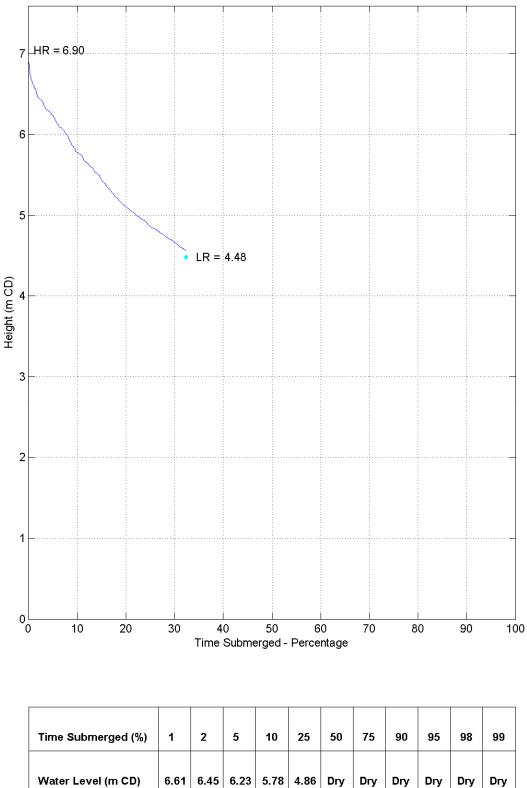
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [214,172]

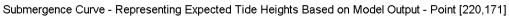


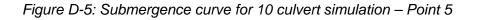


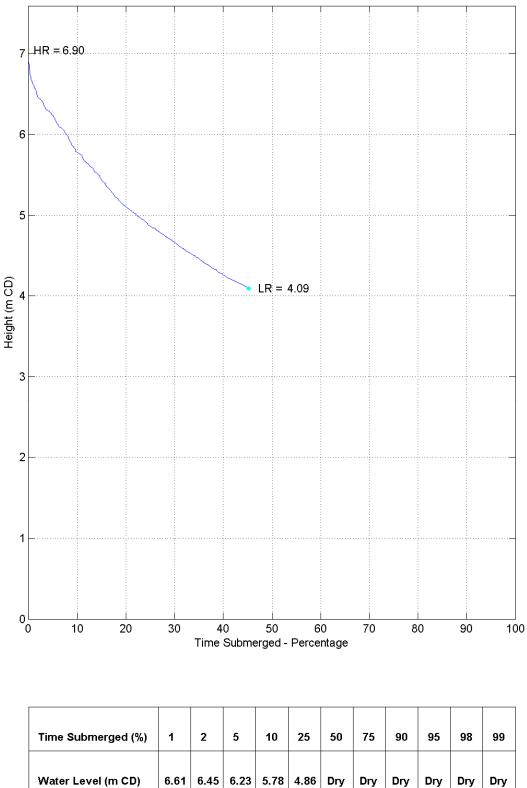
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [218,171]



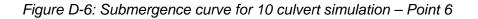


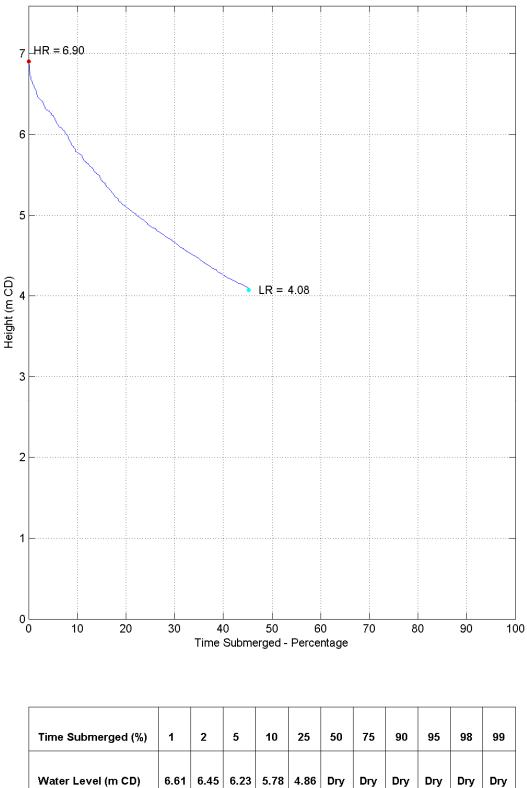




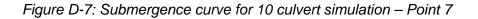


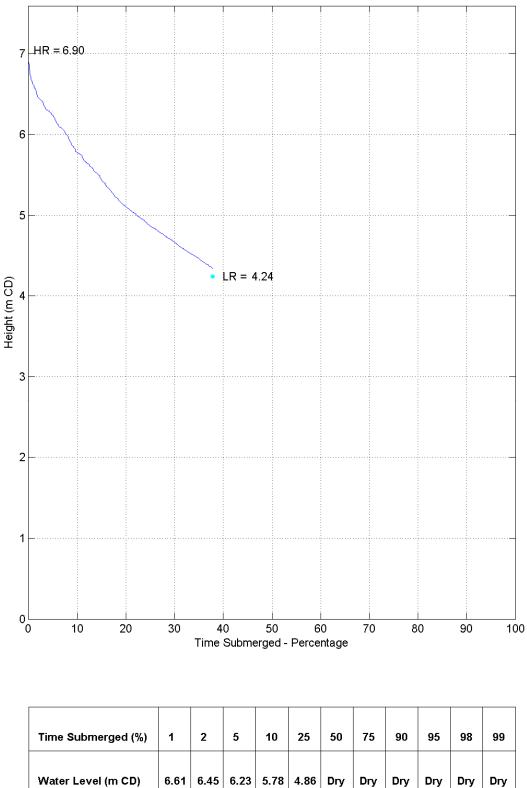
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [222,171]





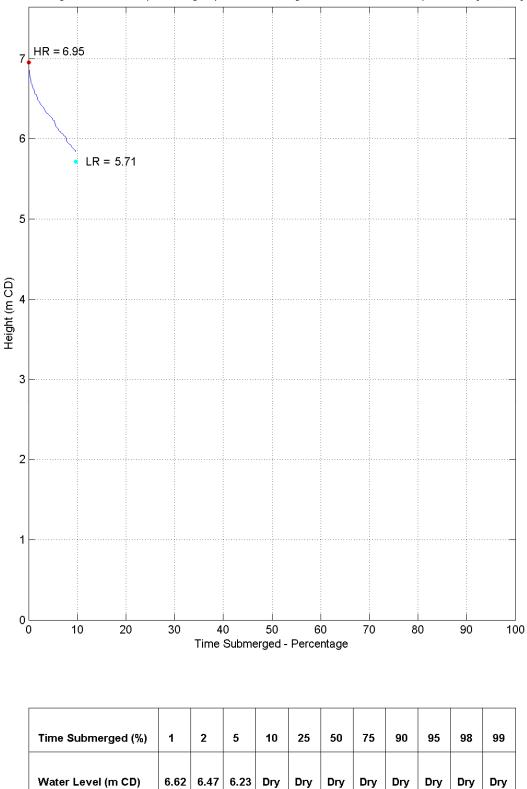
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [223,171]





Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [223,173]

Figure D-8: Submergence curve for 10 culvert simulation – Point 8



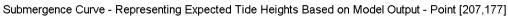
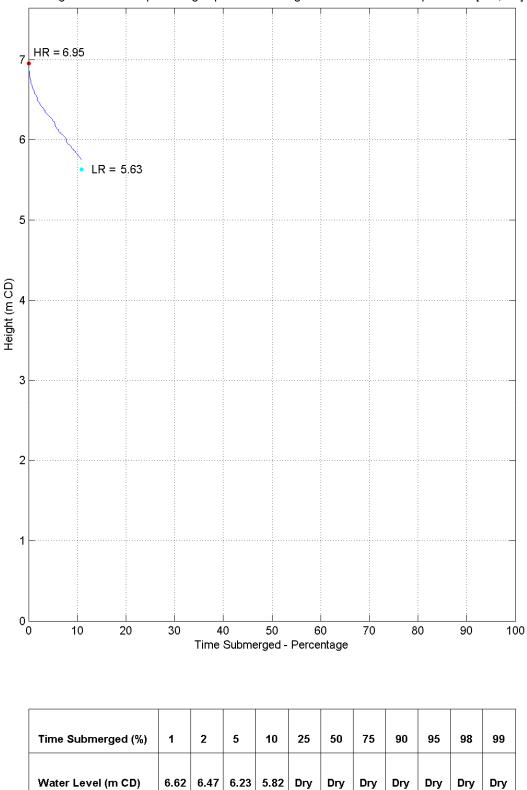
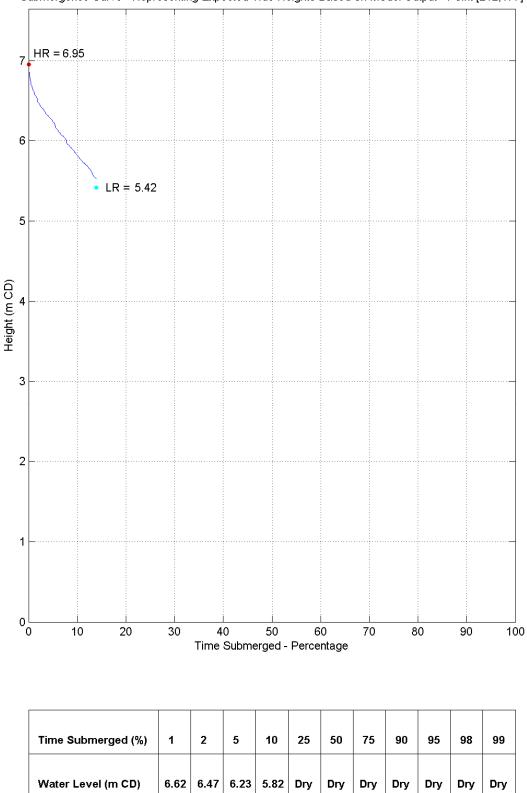


Figure D-9: Submergence curve for 10 culvert simulation – Point 9



Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [209,176]

Figure D-10: Submergence curve for 10 culvert simulation – Point 10



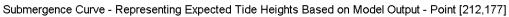
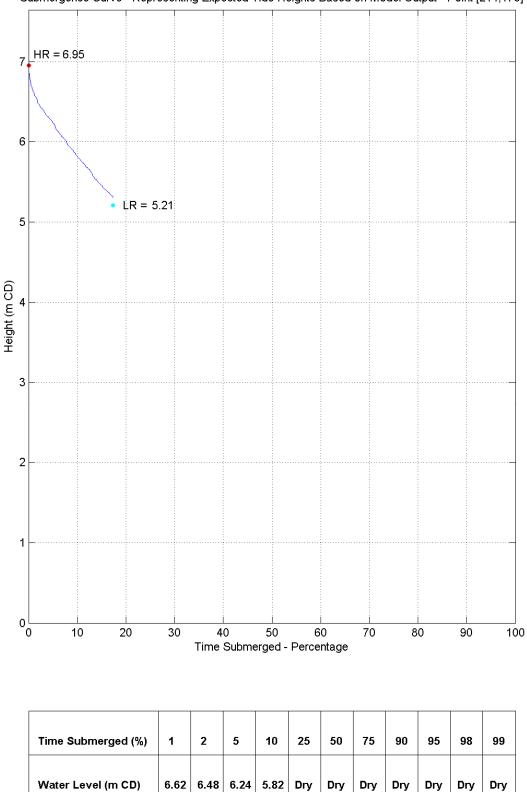


Figure D-11: Submergence curve for 10 culvert simulation – Point 11



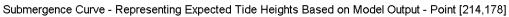
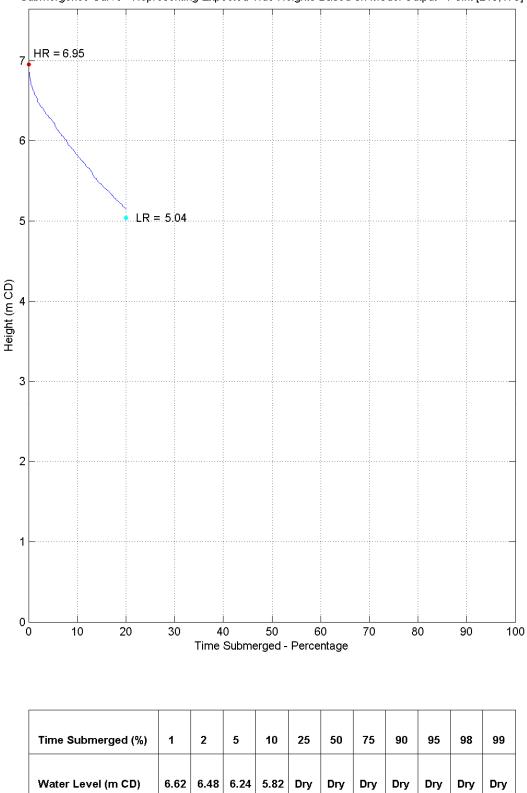


Figure D-12: Submergence curve for 10 culvert simulation – Point 12



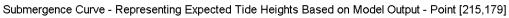
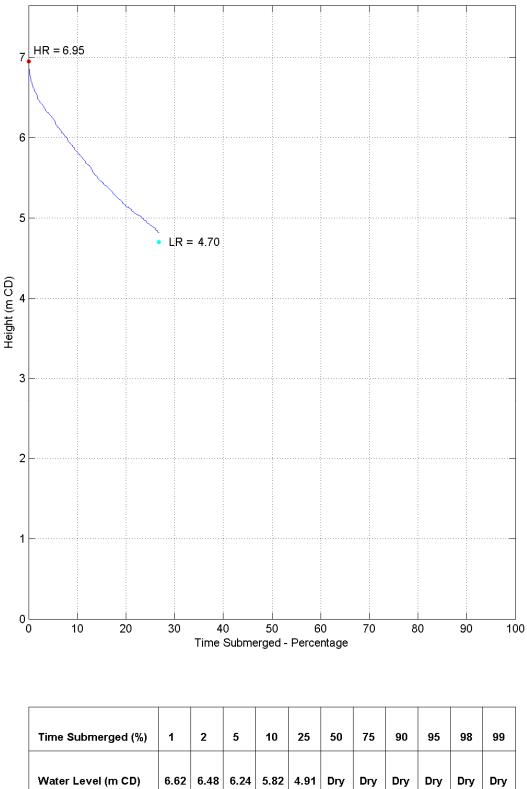
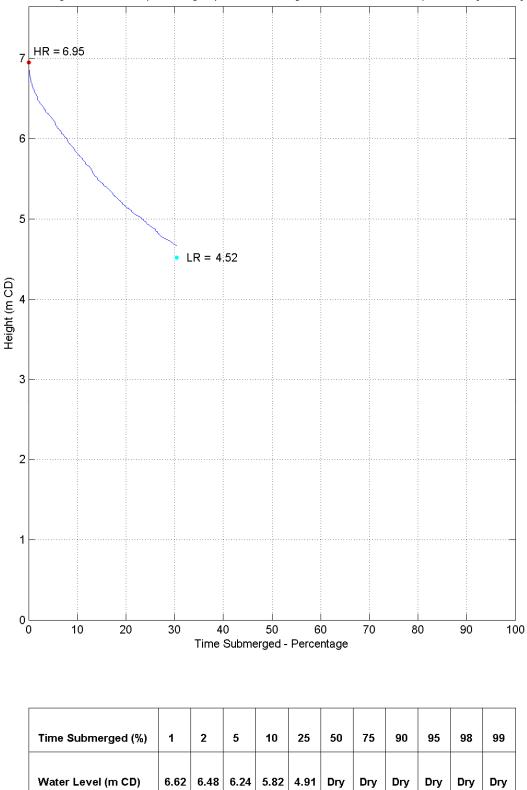


Figure D-13: Submergence curve for 10 culvert simulation – Point 13

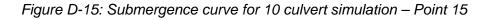


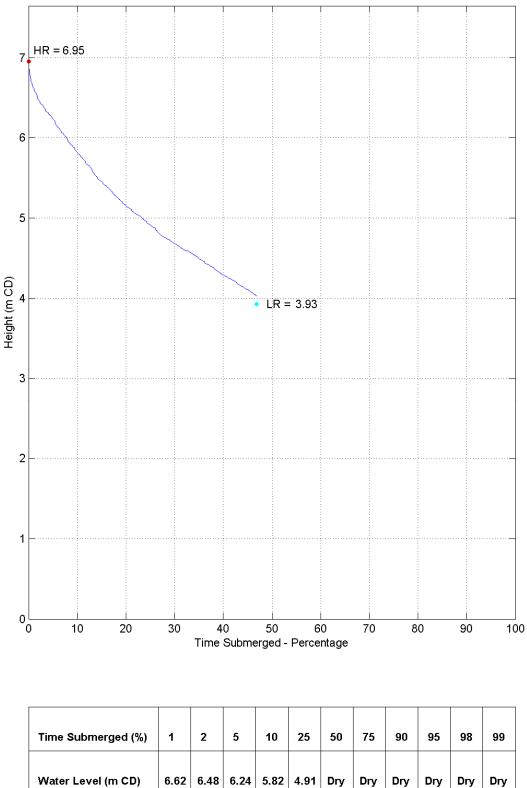
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [217,180]

Figure D-14: Submergence curve for 10 culvert simulation – Point 14



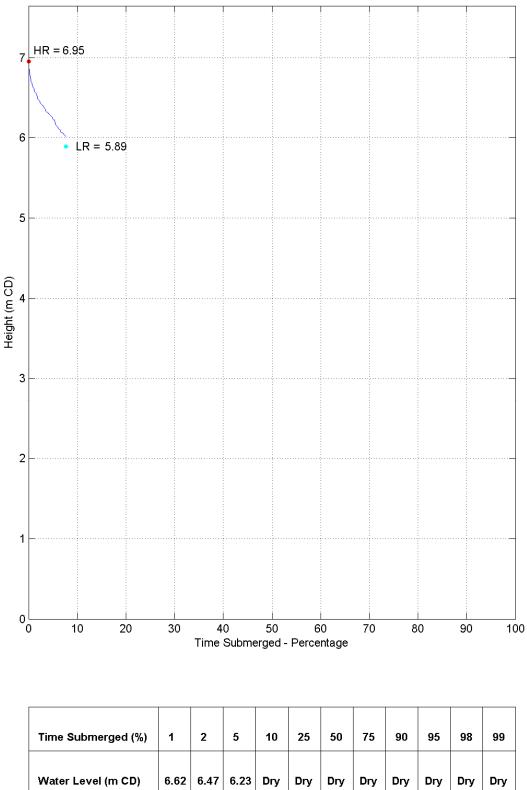
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [218,180]





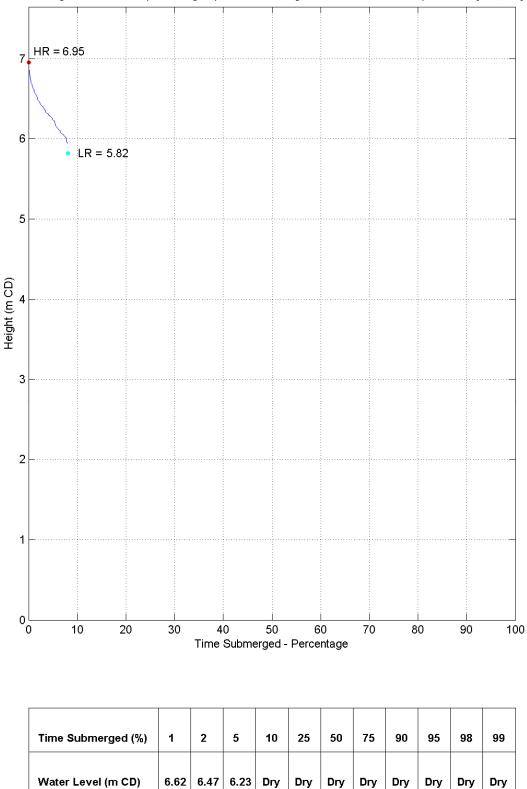
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [220,180]

Figure D-16: Submergence curve for 10 culvert simulation – Point 16



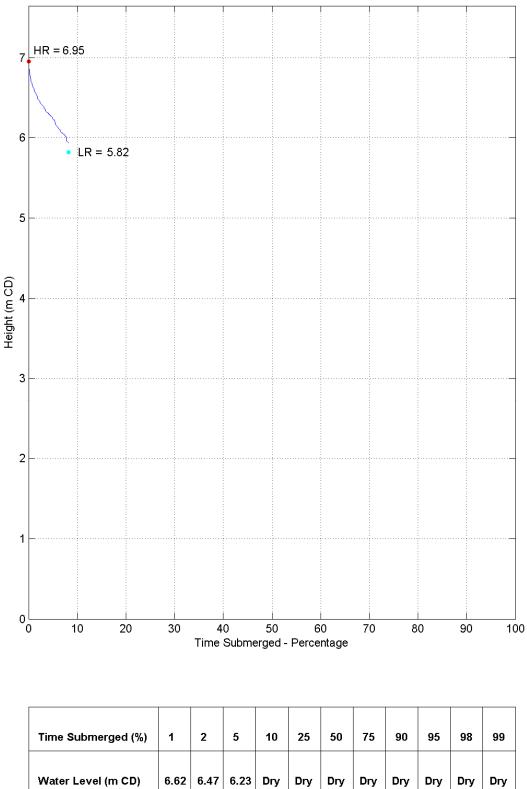
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [202,190]

Figure D-17: Submergence curve for 10 culvert simulation – Point 17



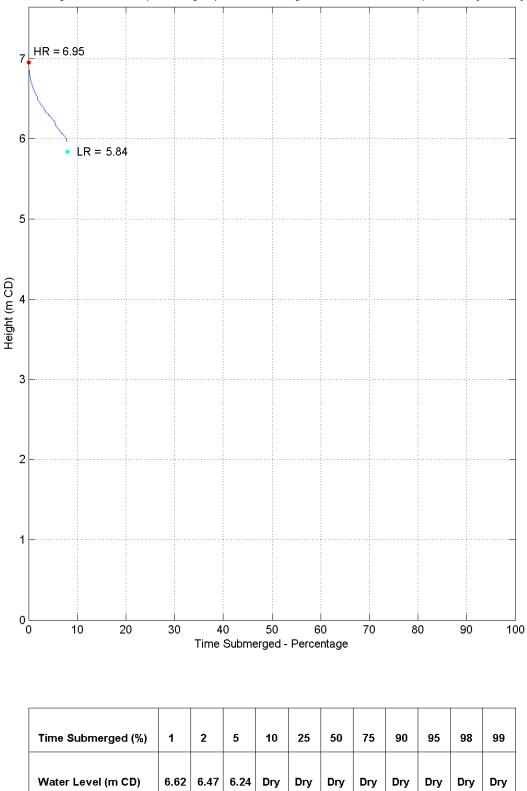
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [204,191]

Figure D-18: Submergence curve for 10 culvert simulation – Point 18



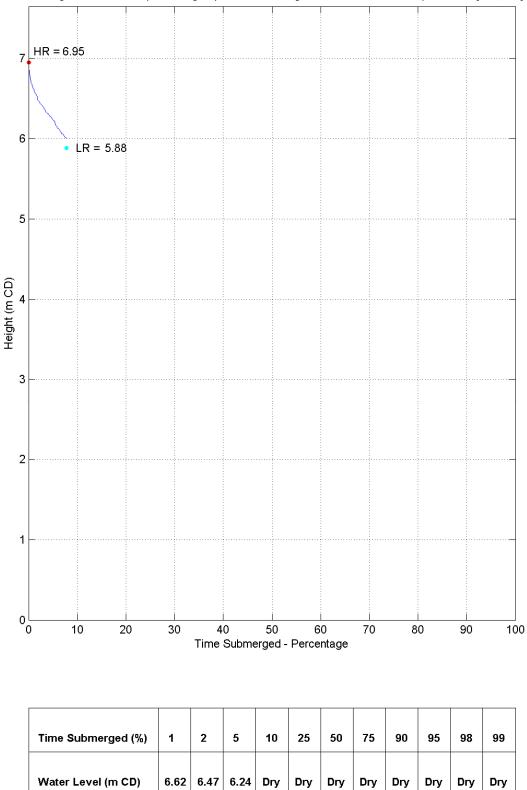
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [205,192]

Figure D-19: Submergence curve for 10 culvert simulation – Point 19



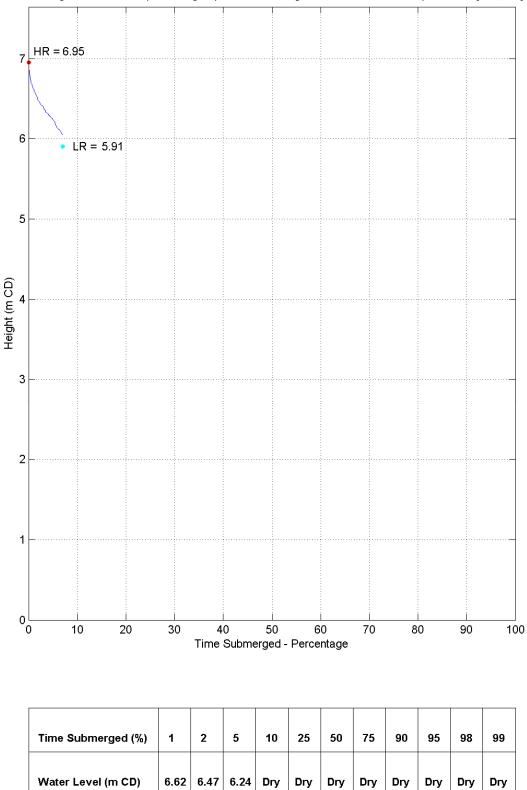
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [208,193]

Figure D-20: Submergence curve for 10 culvert simulation – Point 20



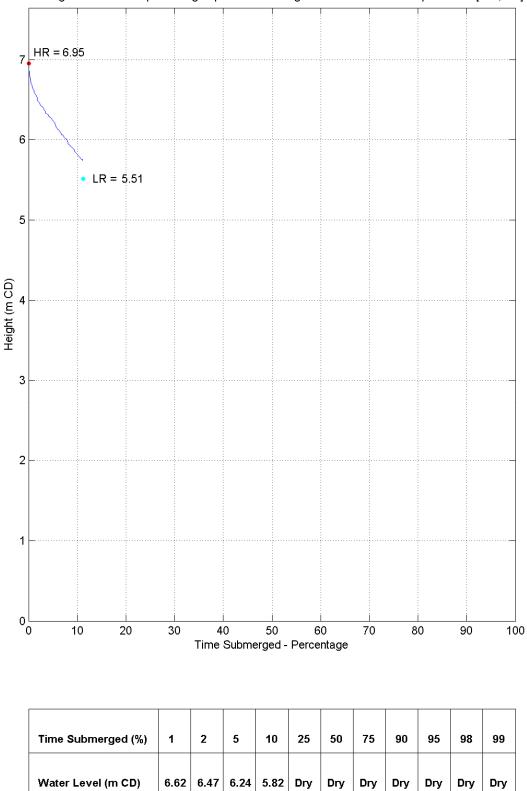
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [209,194]

Figure D-21: Submergence curve for 10 culvert simulation – Point 21



Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [210,193]

Figure D-22: Submergence curve for 10 culvert simulation – Point 22



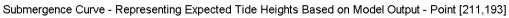
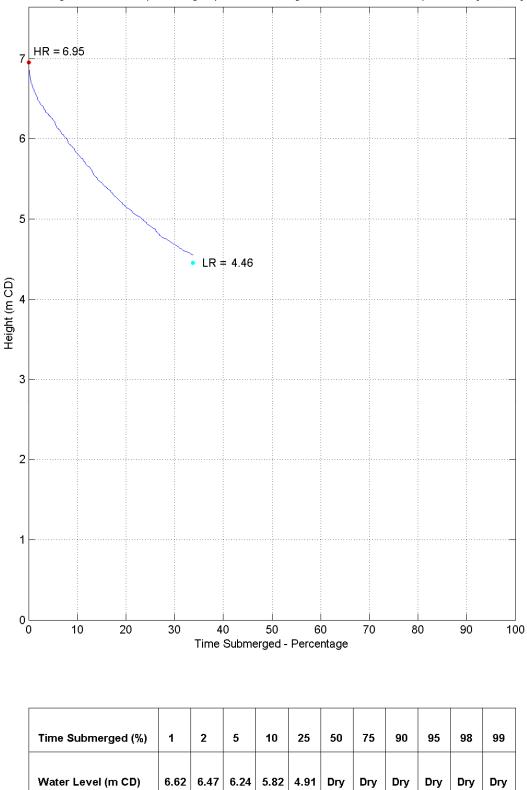
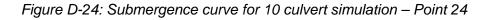
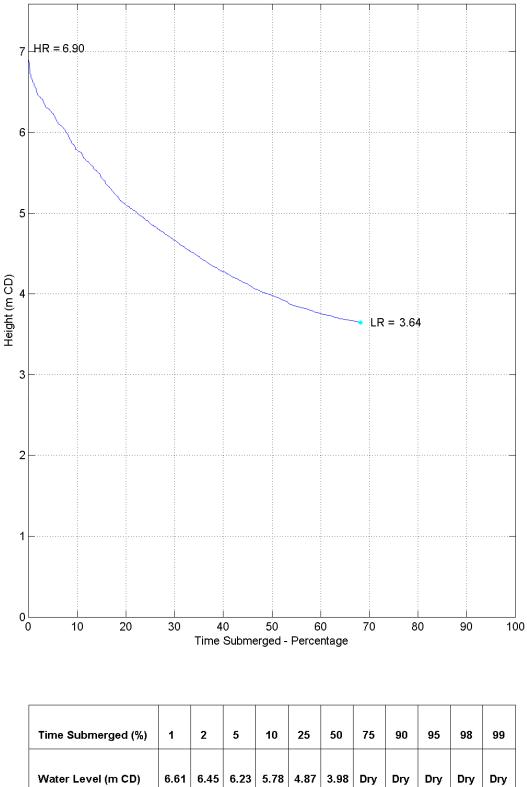


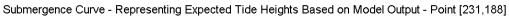
Figure D-23: Submergence curve for 10 culvert simulation – Point 23

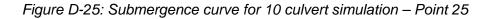


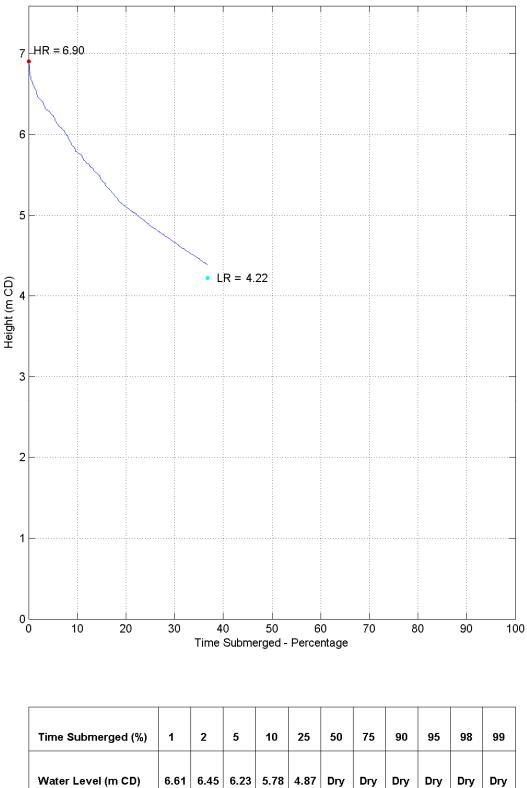
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [212,193]





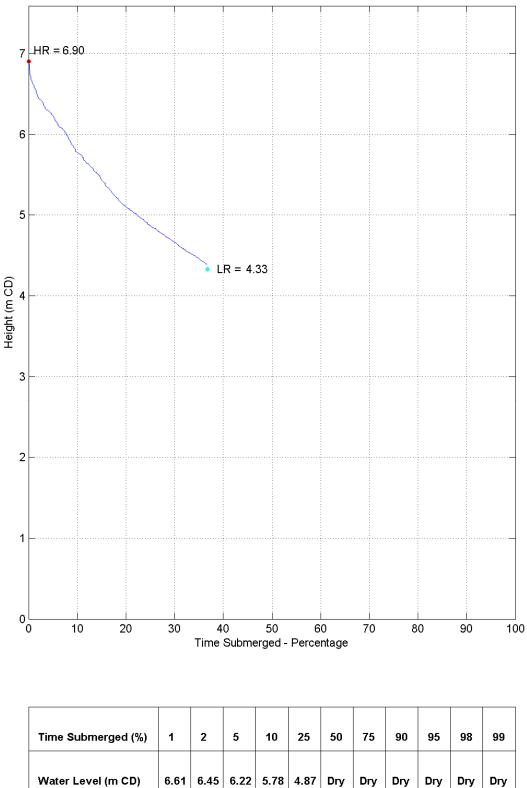






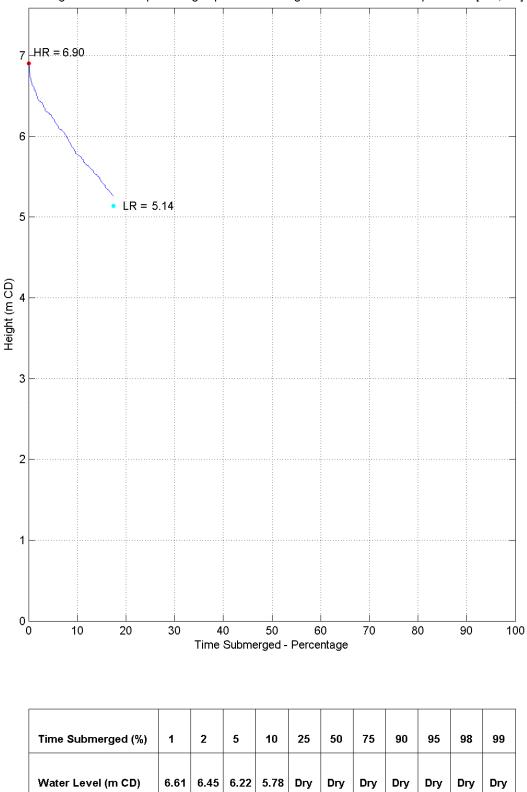
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [232,187]

Figure D-26: Submergence curve for 10 culvert simulation – Point 26



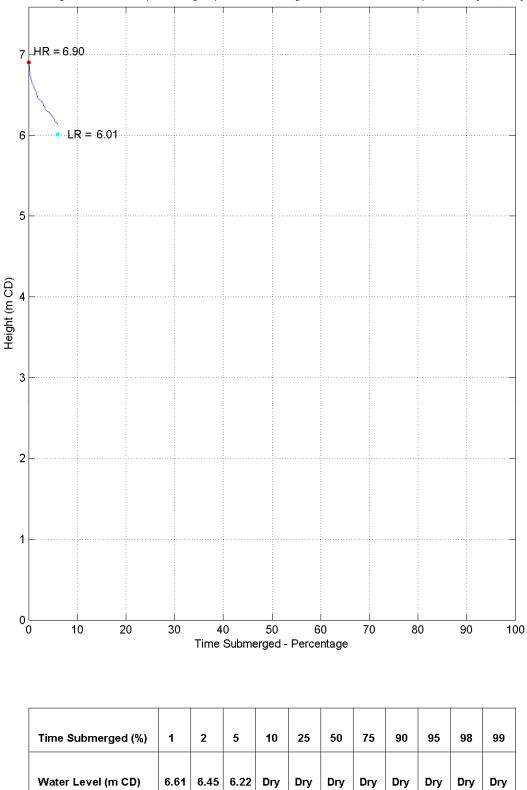
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [233,188]

Figure D-27: Submergence curve for 10 culvert simulation – Point 27



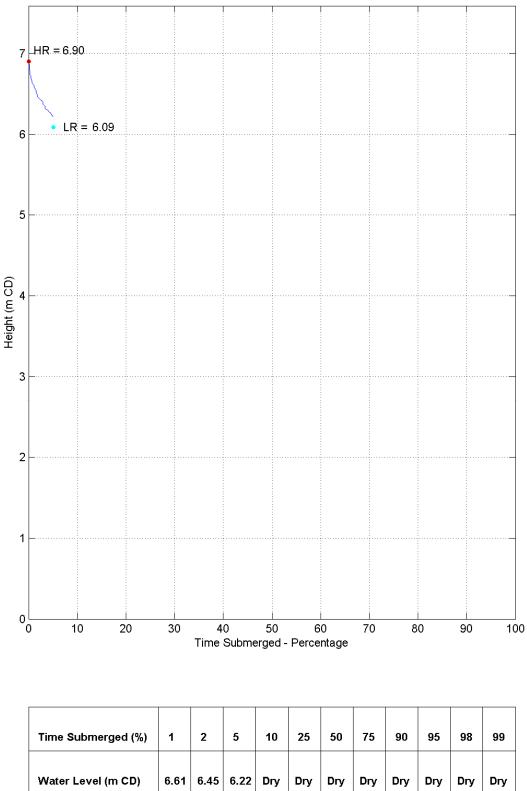
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [236,189]

Figure D-28: Submergence curve for 10 culvert simulation – Point 28



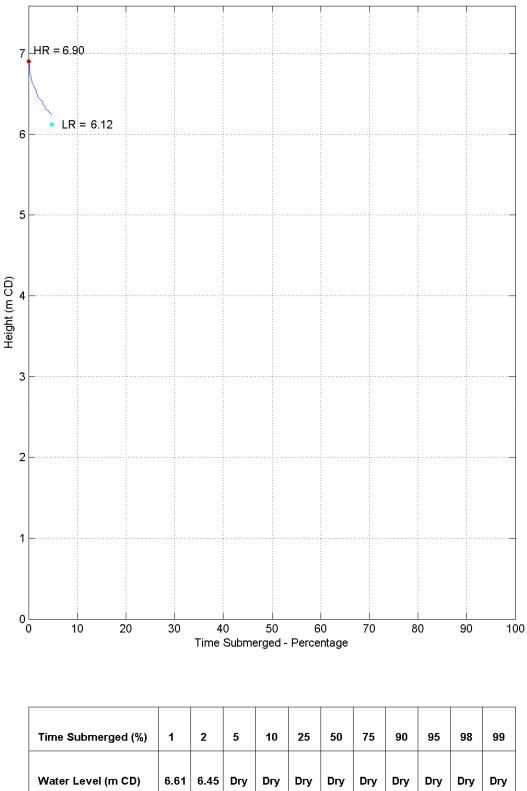
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [239,189]

Figure D-29: Submergence curve for 10 culvert simulation – Point 29

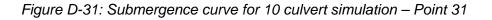


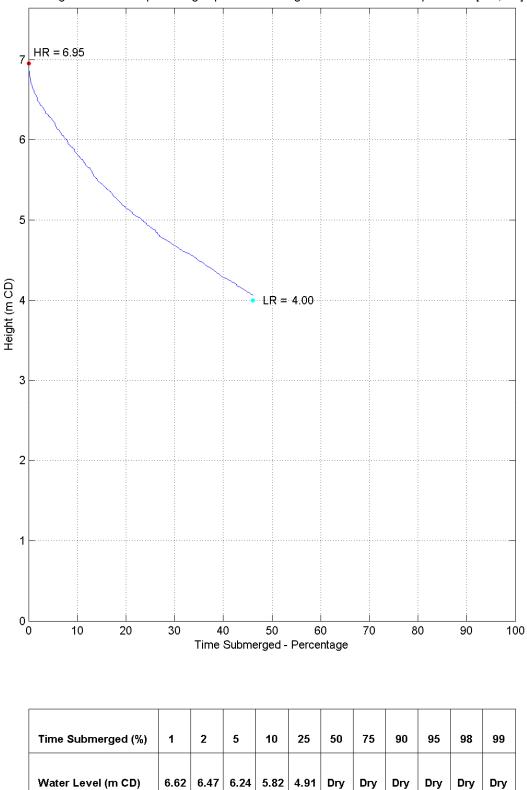
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [240,189]

Figure D-30: Submergence curve for 10 culvert simulation – Point 30

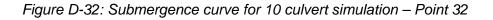


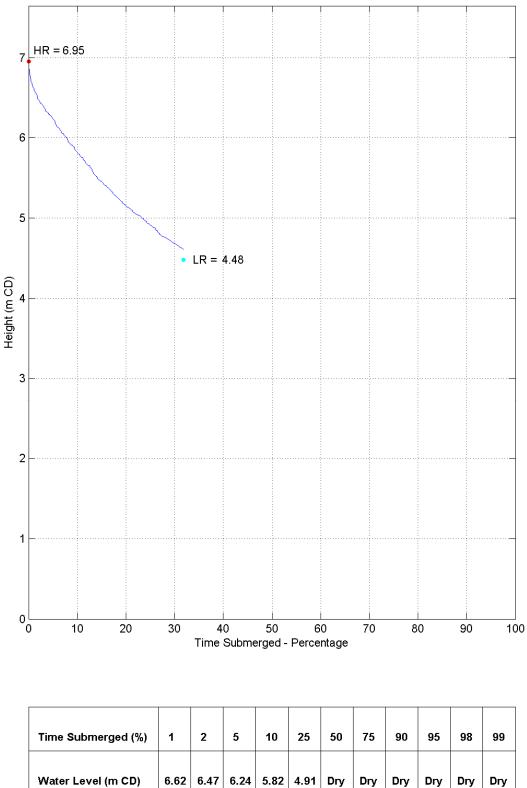
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [242,189]





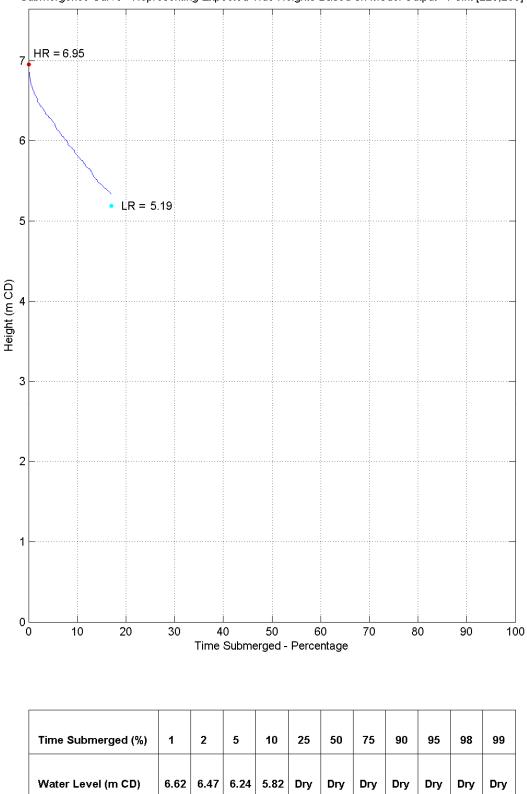
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [225,206]





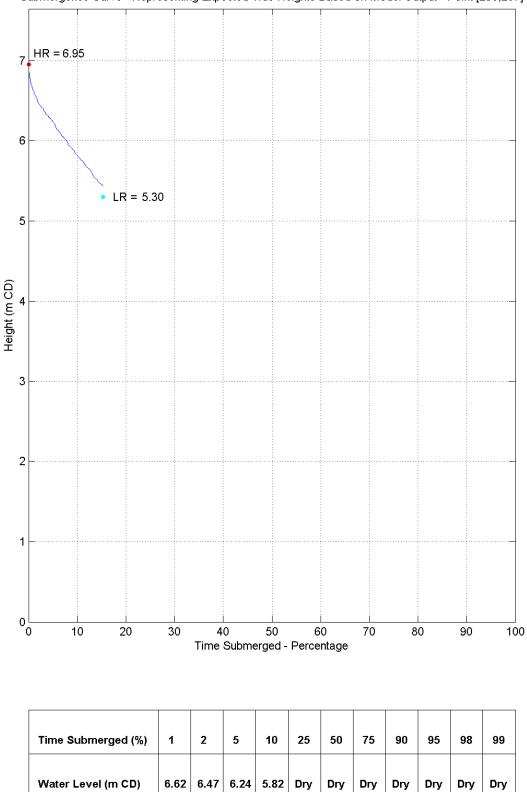
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [226,207]

Figure D-33: Submergence curve for 10 culvert simulation – Point 33



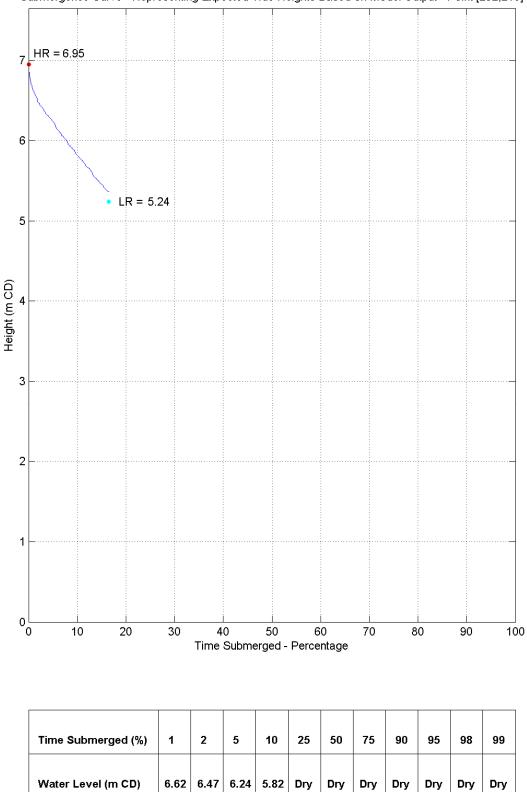
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [228,206]

Figure D-34: Submergence curve for 10 culvert simulation – Point 34



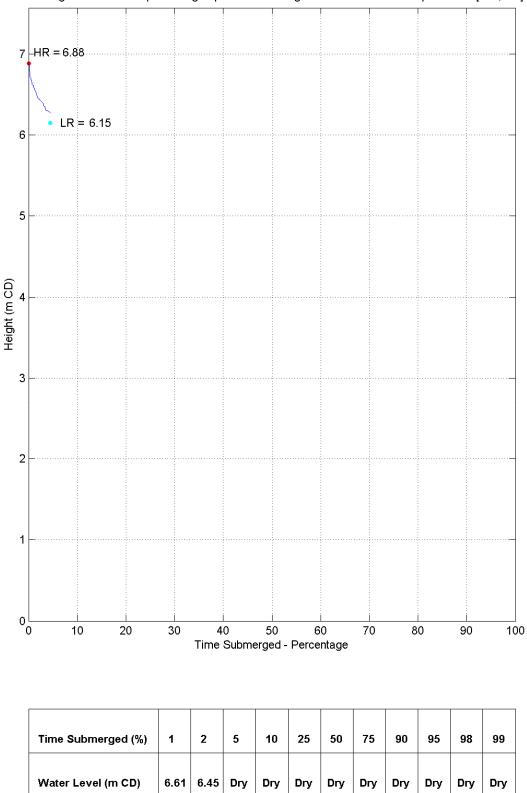
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [230,207]

Figure D-35: Submergence curve for 10 culvert simulation – Point 35



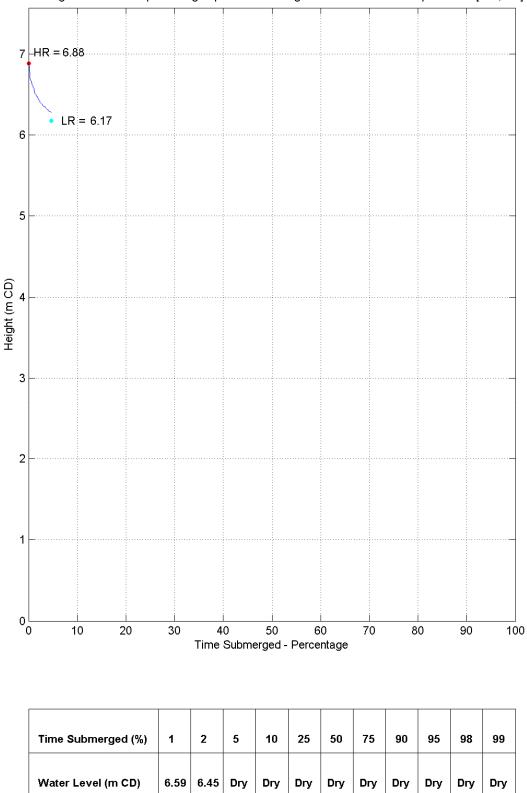
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [232,210]

Figure D-36: Submergence curve for 10 culvert simulation – Point 36



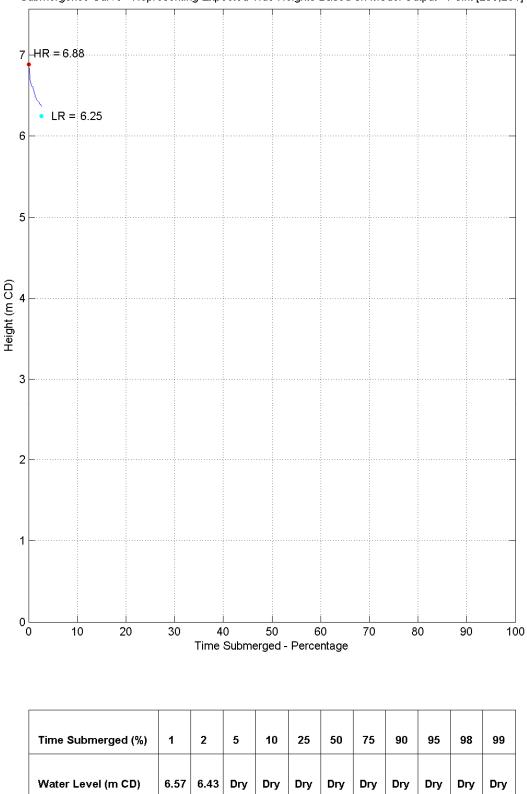
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [233,196]

Figure D-37: Submergence curve for 10 culvert simulation – Point 37



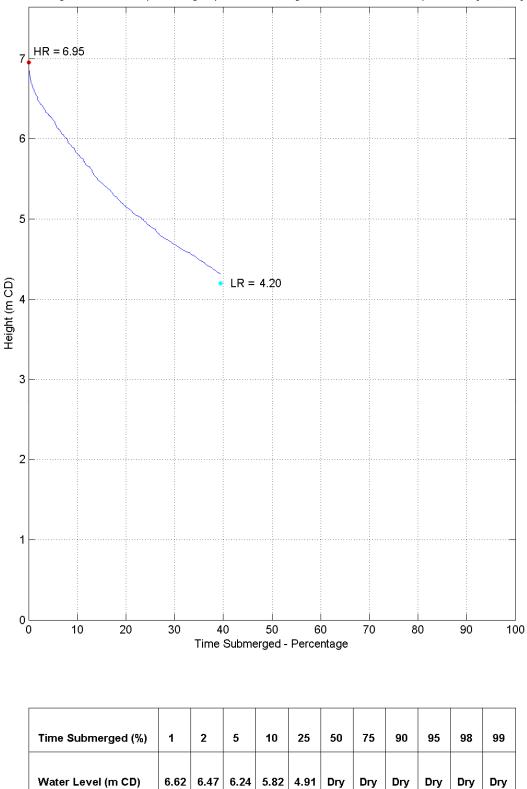
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [234,198]

Figure D-38: Submergence curve for 10 culvert simulation – Point 38



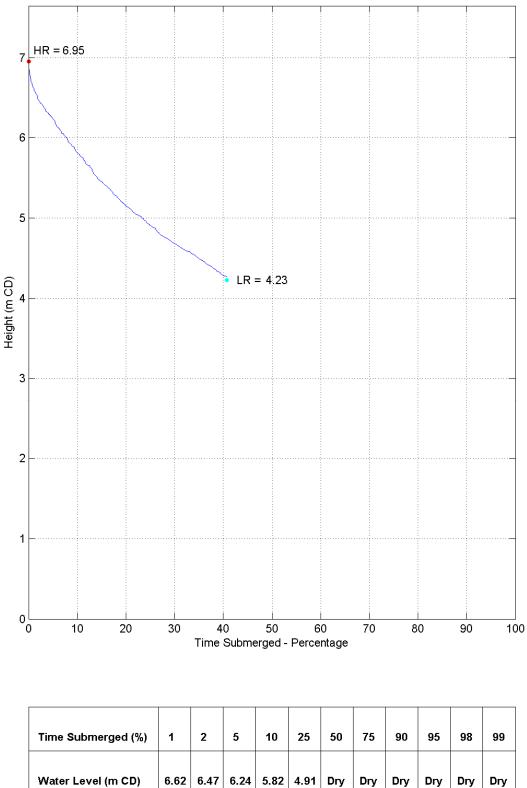
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [236,201]

Figure D-39: Submergence curve for 10 culvert simulation – Point 39

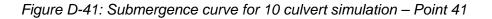


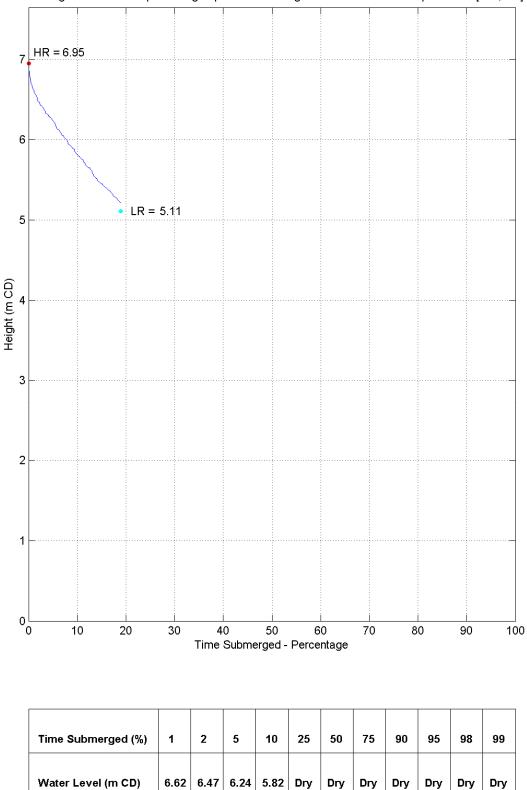
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [221,215]

Figure D-40: Submergence curve for 10 culvert simulation – Point 40



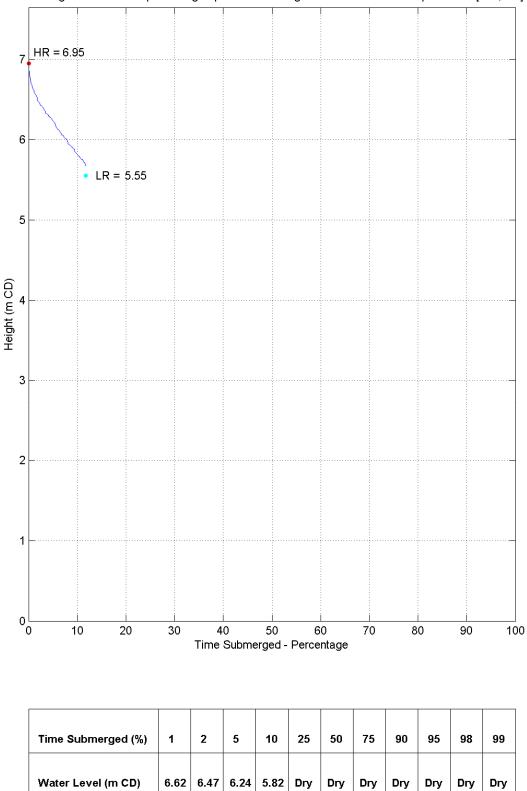
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [222,216]





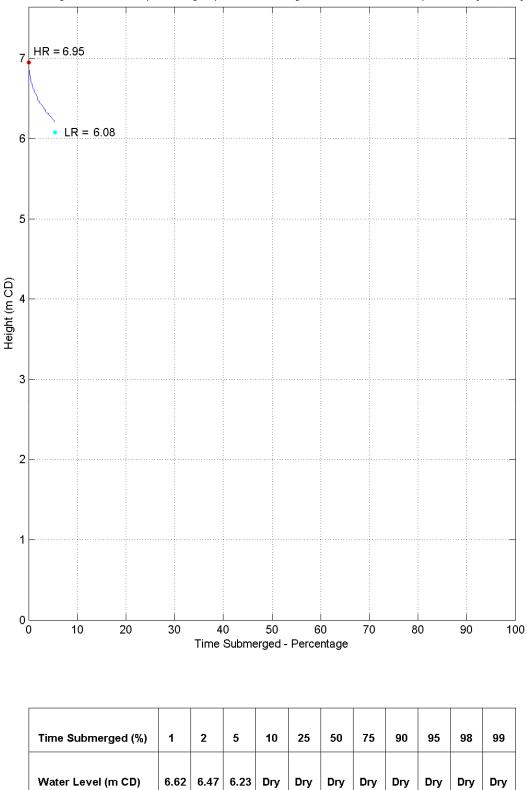
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [223,218]

Figure D-42: Submergence curve for 10 culvert simulation – Point 42



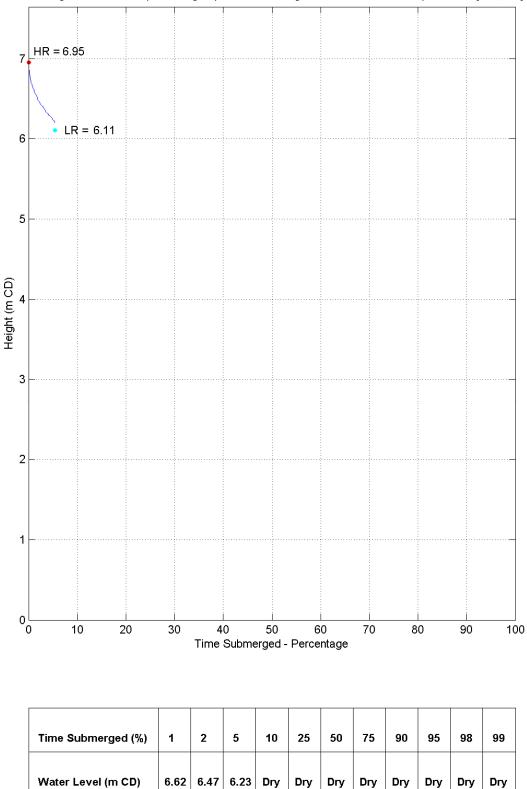
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [225,219]

Figure D-43: Submergence curve for 10 culvert simulation – Point 43



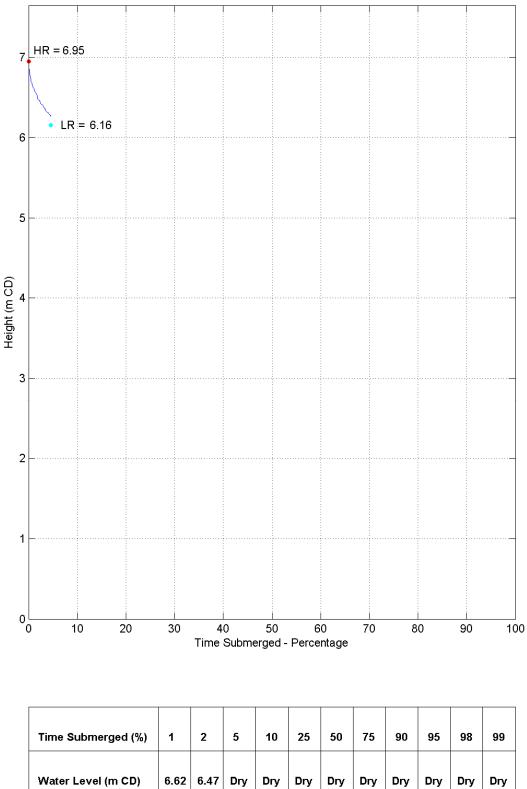
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [228,220]

Figure D-44: Submergence curve for 10 culvert simulation – Point 44



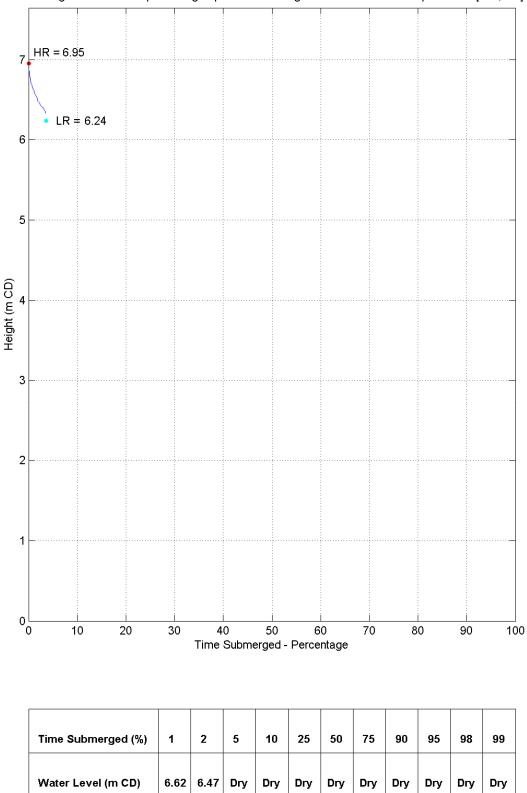
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [230,220]

Figure D-45: Submergence curve for 10 culvert simulation – Point 45

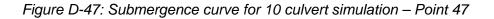


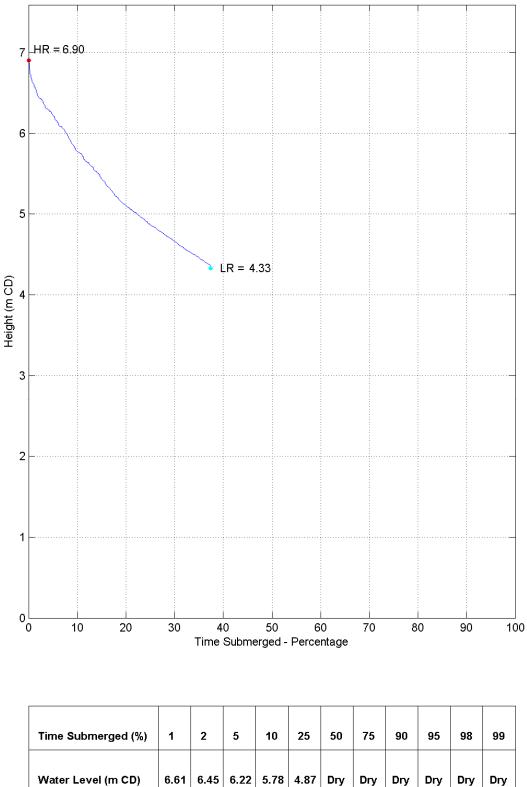
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [232,220]

Figure D-46: Submergence curve for 10 culvert simulation – Point 46



Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [234,221]

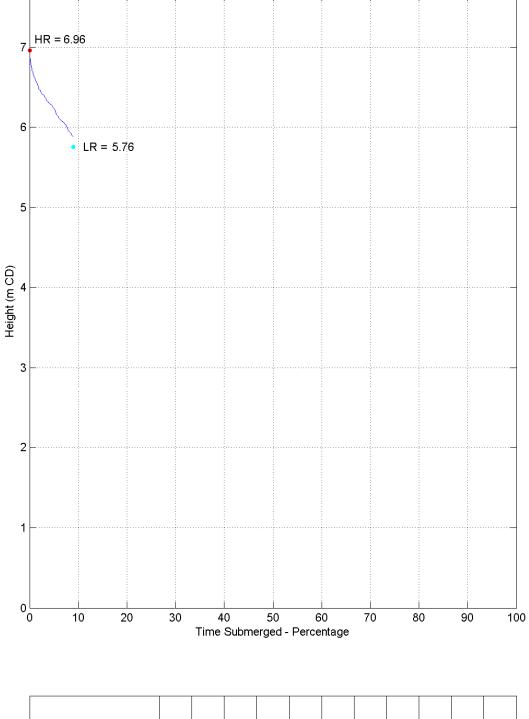




Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [234,189]

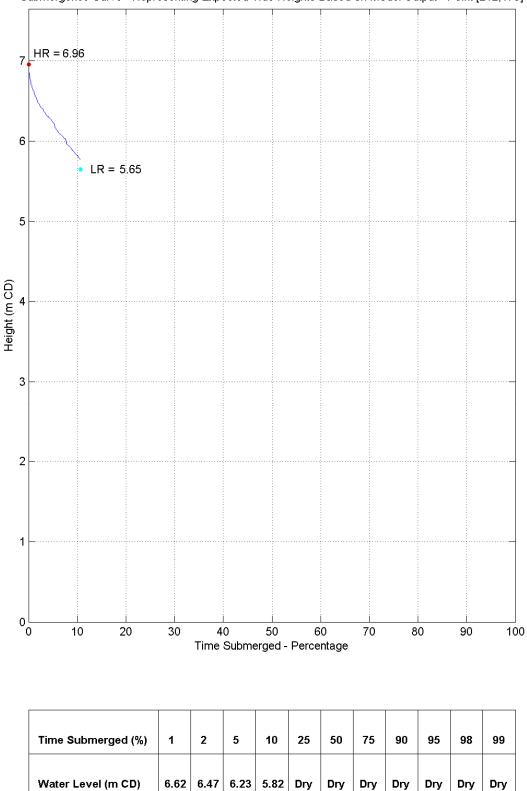
Figure D-48: Submergence curve for 10 culvert simulation – Point 48

E. APPENDIX - SUBMERGENCE CURVES FOR 15 CULVERT SIMULATION



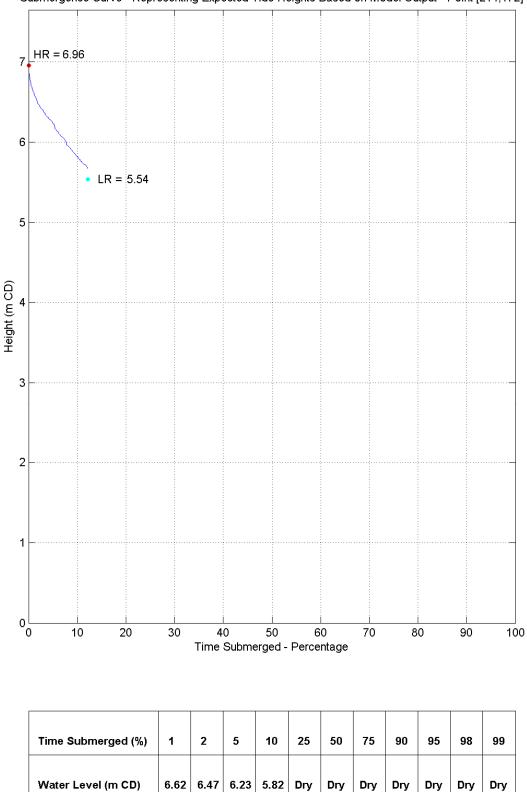
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Water Level (m CD)	6.62	6.47	6.23	Dry							

Figure E-1: Submergence curve for 15 culvert simulation – Point 1

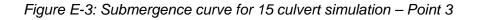


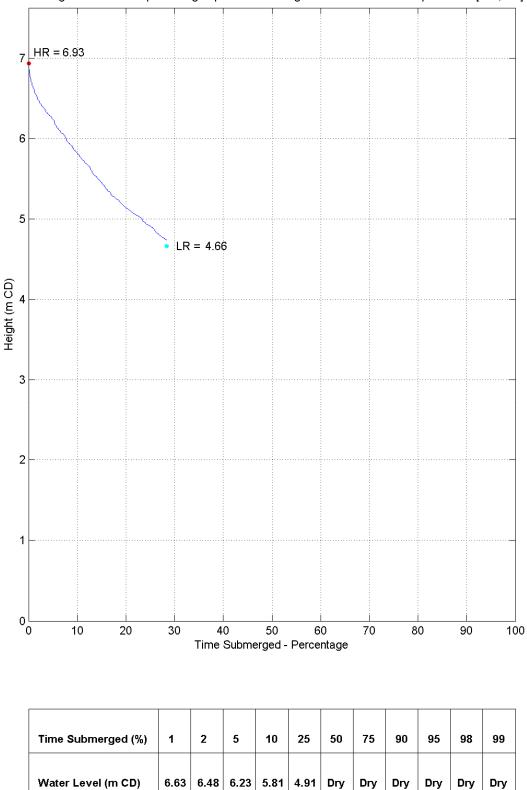
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [212,173]

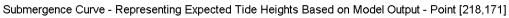
Figure E-2: Submergence curve for 15 culvert simulation – Point 2

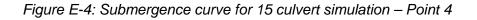


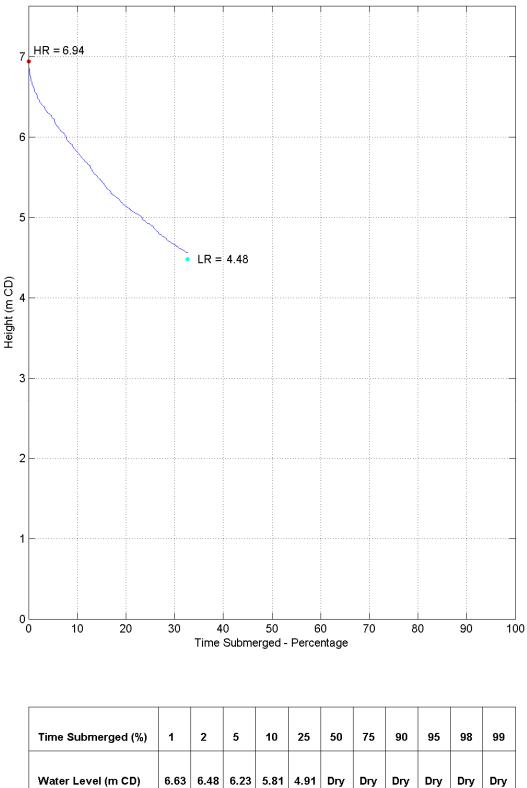
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [214,172]



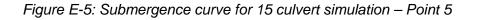


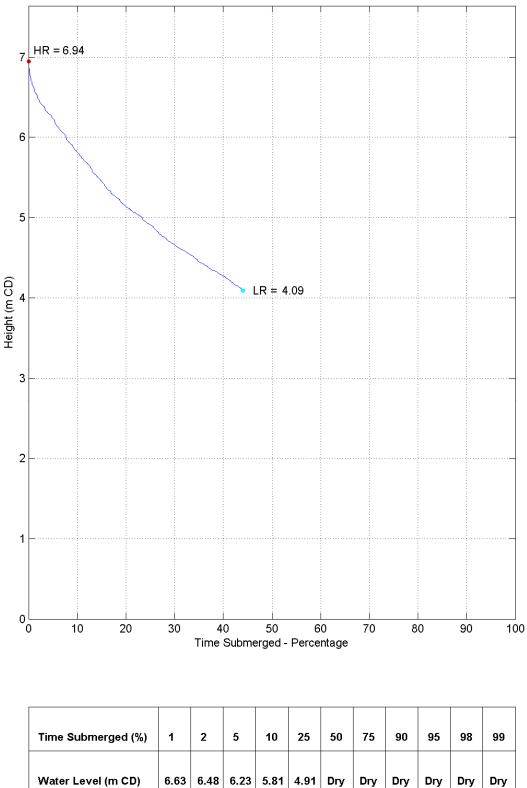




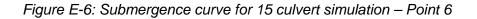


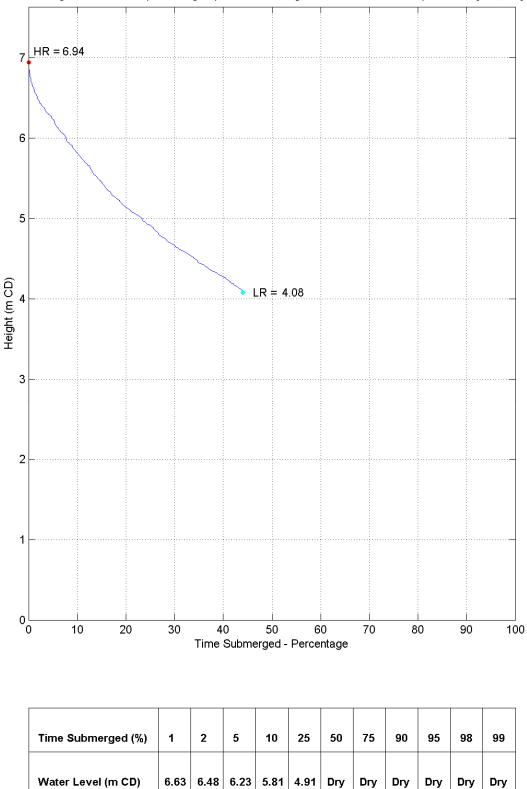
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [220,171]

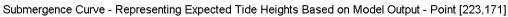


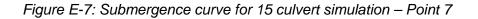


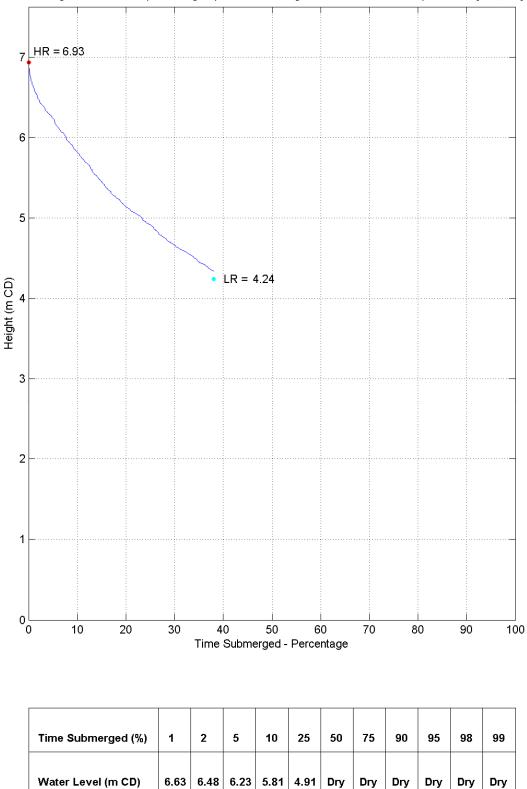
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [222,171]











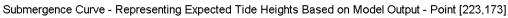
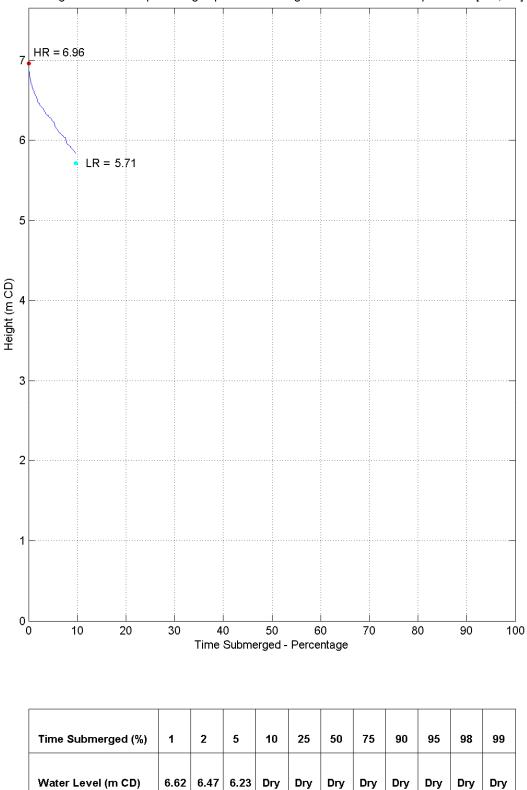
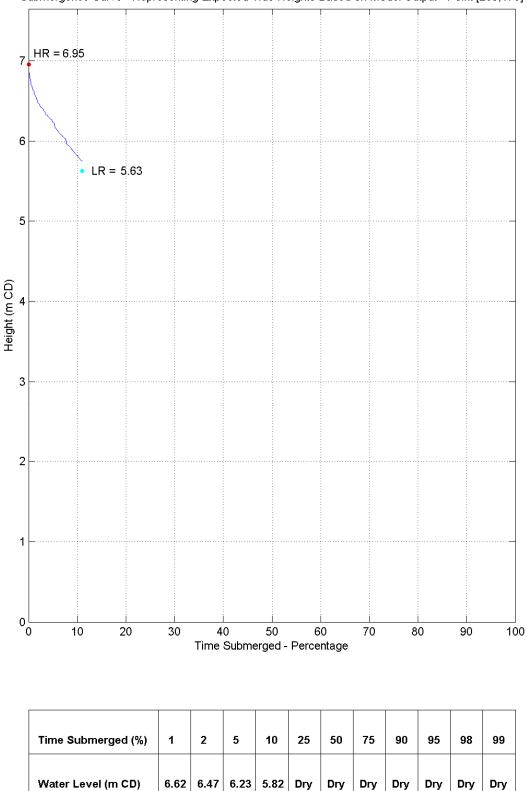


Figure E-8: Submergence curve for 15 culvert simulation – Point 8



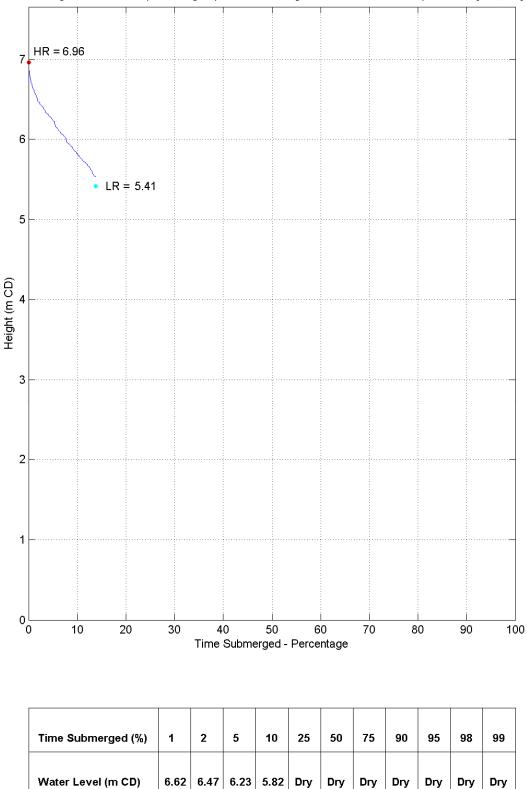
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [207,177]

Figure E-9: Submergence curve for 15 culvert simulation – Point 9



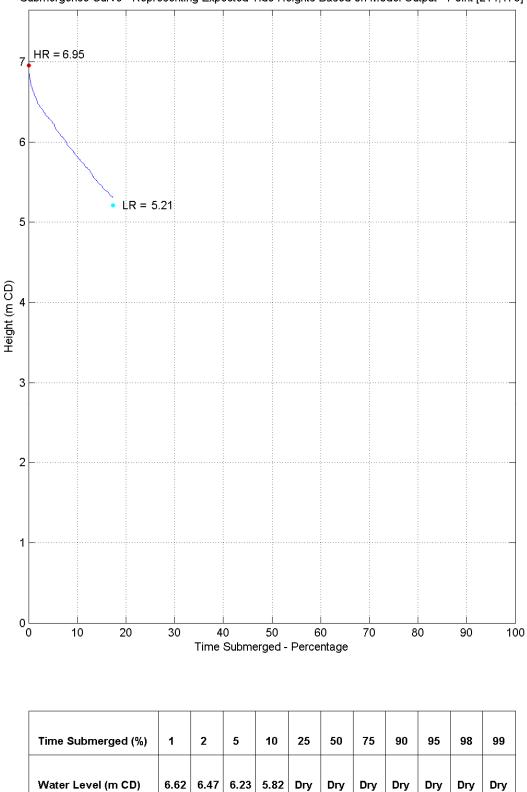
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [209,176]

Figure E-10: Submergence curve for 15 culvert simulation – Point 10



Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [212,177]

Figure E-11: Submergence curve for 15 culvert simulation – Point 11



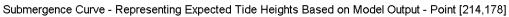
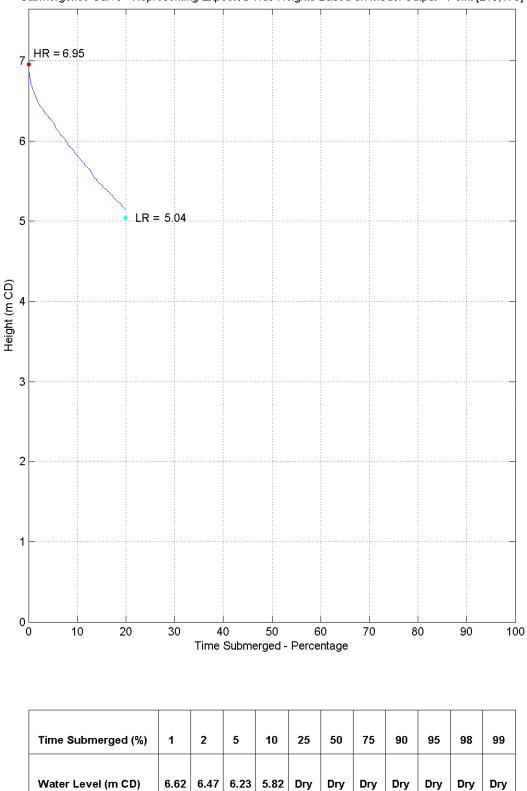
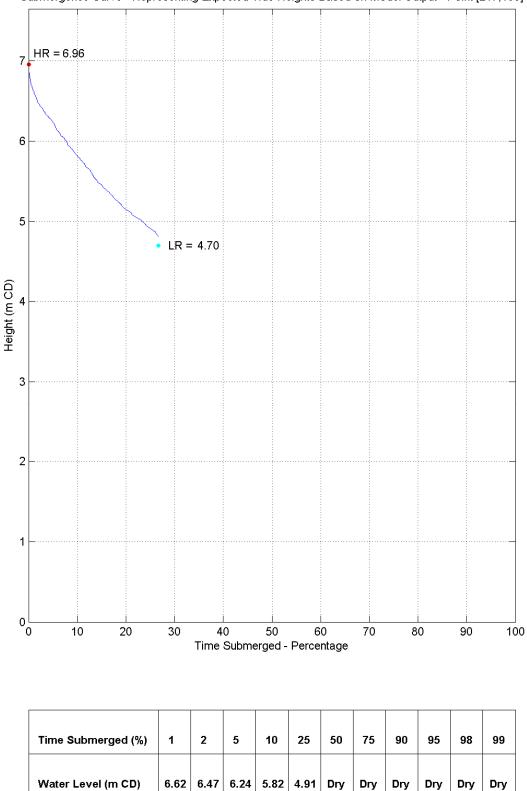


Figure E-12: Submergence curve for 15 culvert simulation – Point 12



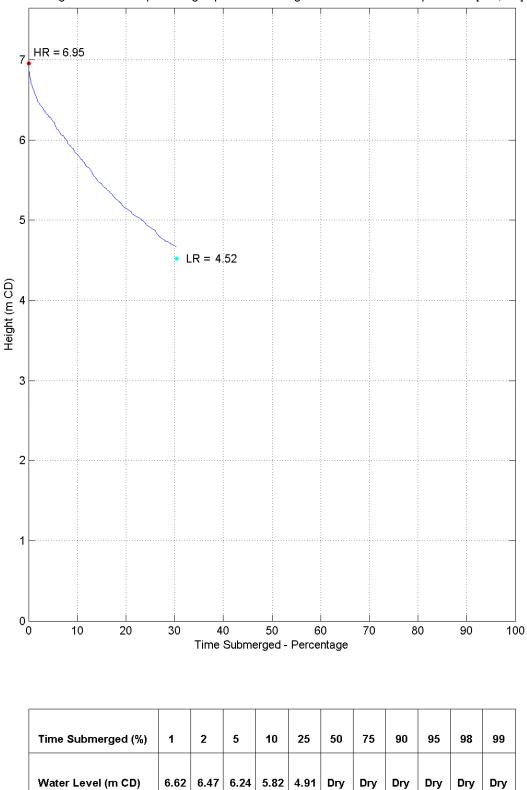
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [215,179]

Figure E-13: Submergence curve for 15 culvert simulation – Point 13



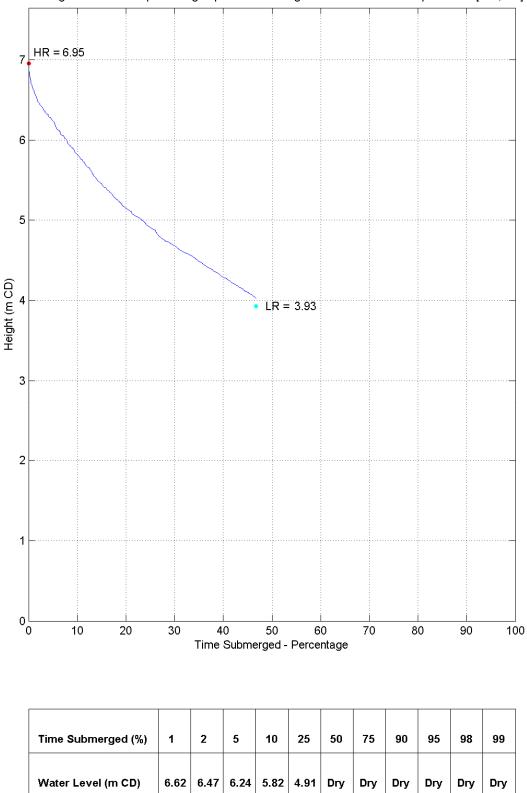
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [217,180]

Figure E-14: Submergence curve for 15 culvert simulation – Point 14



Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [218,180]

Figure E-15: Submergence curve for 15 culvert simulation – Point 15



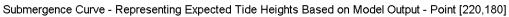
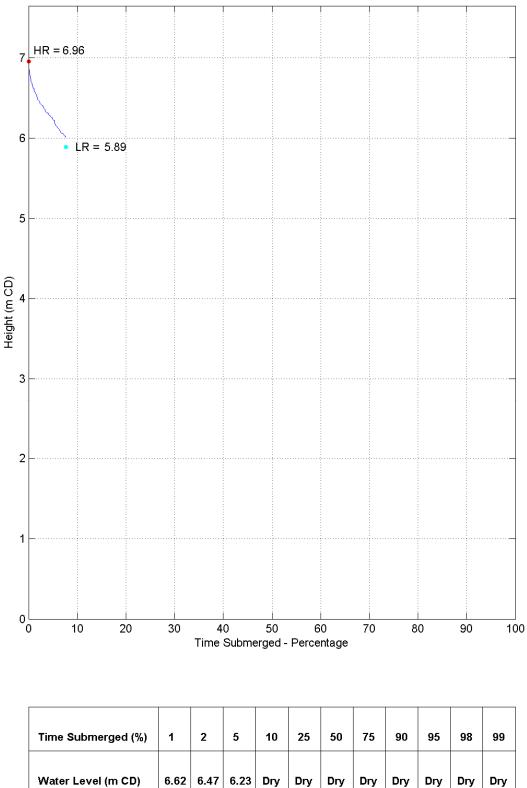
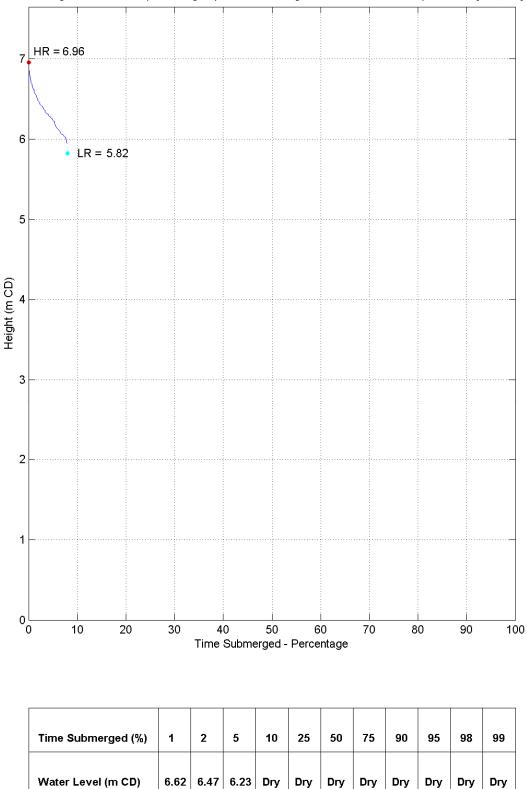


Figure E-16: Submergence curve for 15 culvert simulation – Point 16



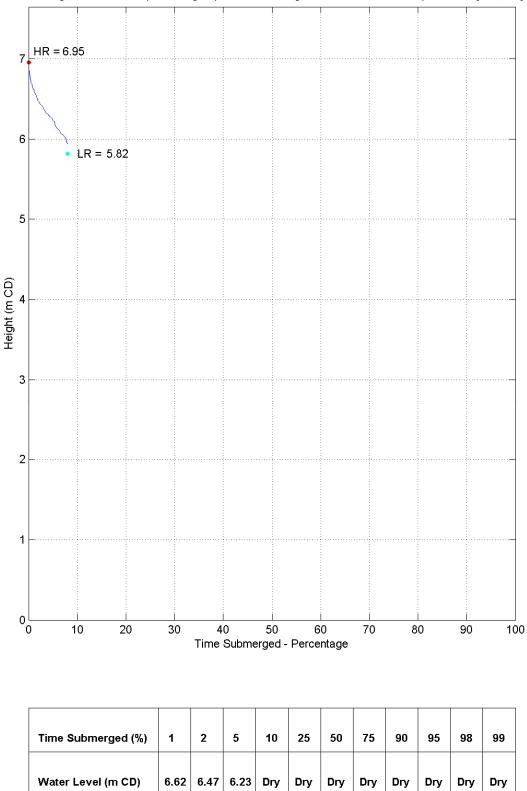
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [202,190]

Figure E-17: Submergence curve for 15 culvert simulation – Point 17



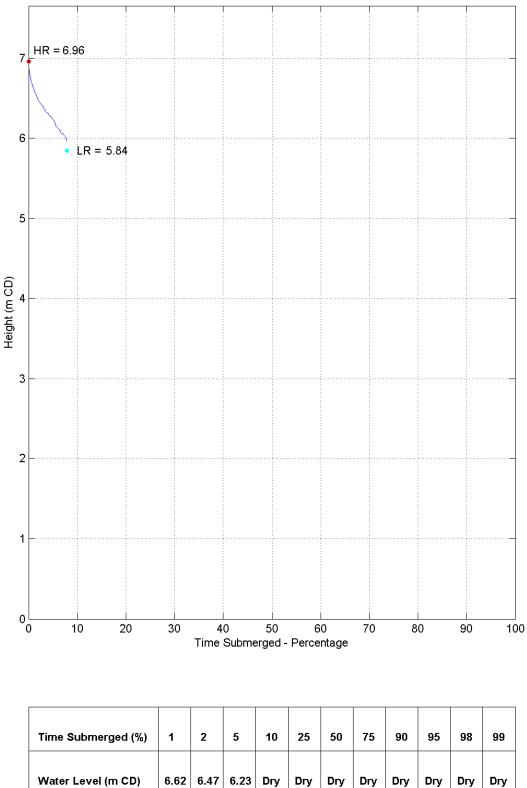
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [204,191]

Figure E-18: Submergence curve for 15 culvert simulation – Point 18



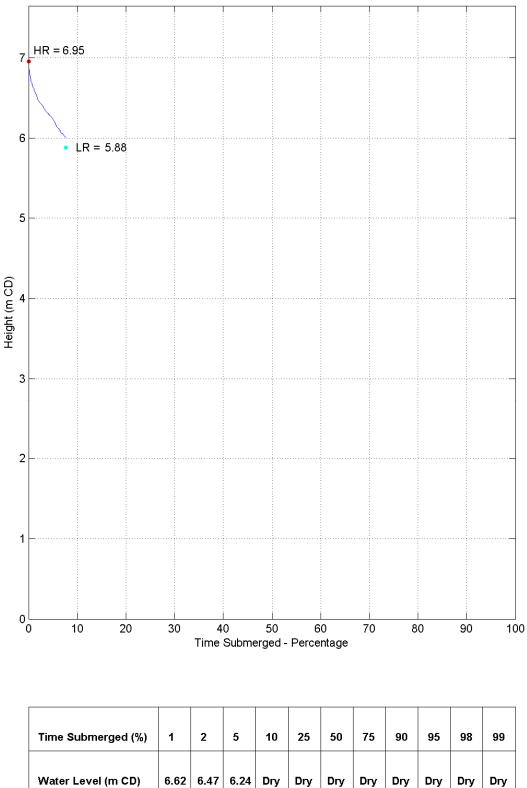
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [205,192]

Figure E-19: Submergence curve for 15 culvert simulation – Point 19



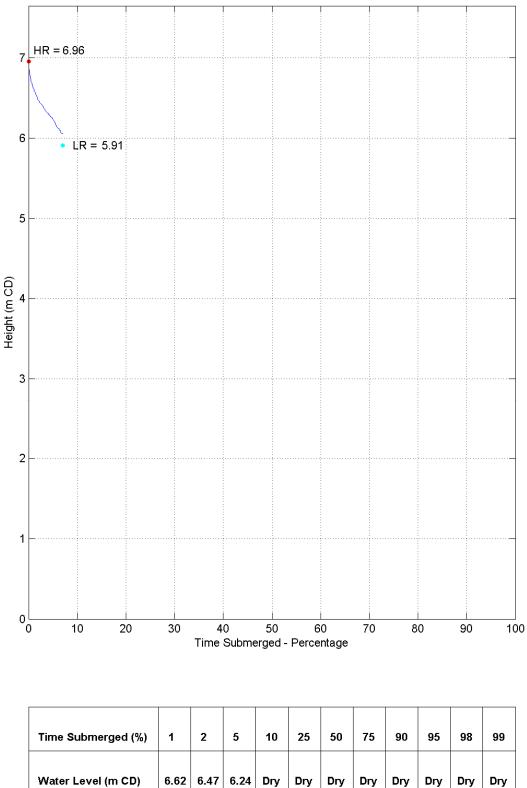
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [208,193]

Figure E-20: Submergence curve for 15 culvert simulation – Point 20



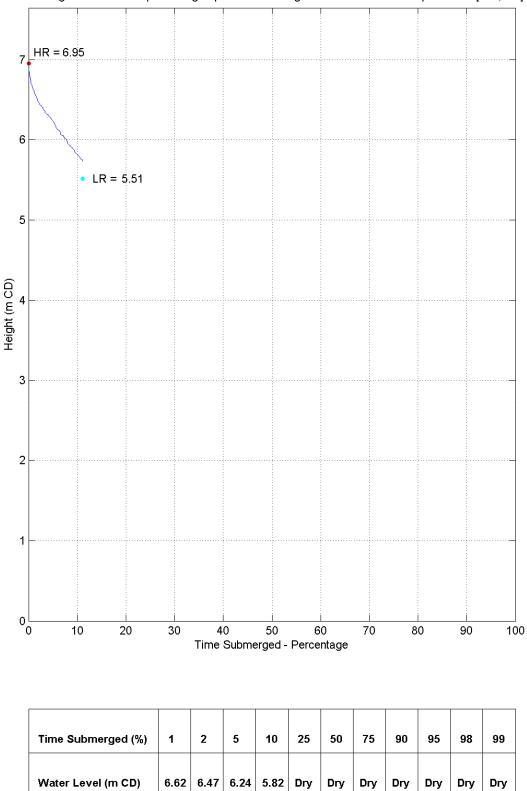
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [209,194]

Figure E-21: Submergence curve for 15 culvert simulation – Point 21



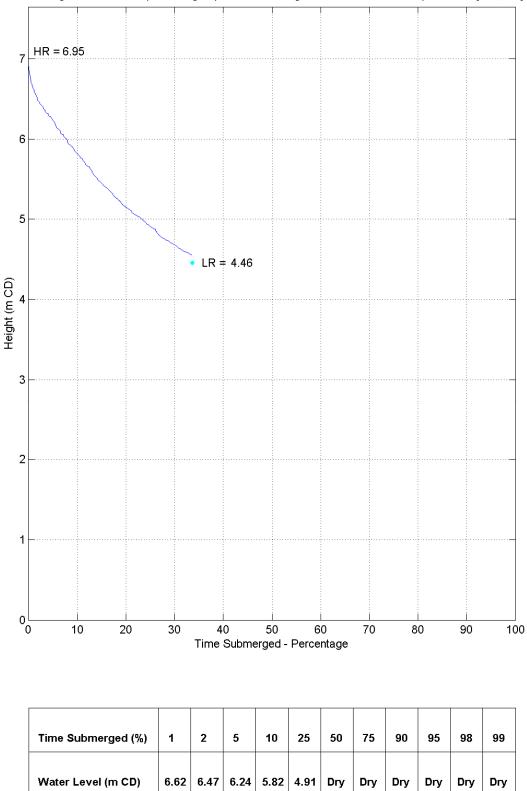
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [210,193]

Figure E-22: Submergence curve for 15 culvert simulation – Point 22

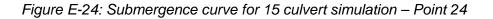


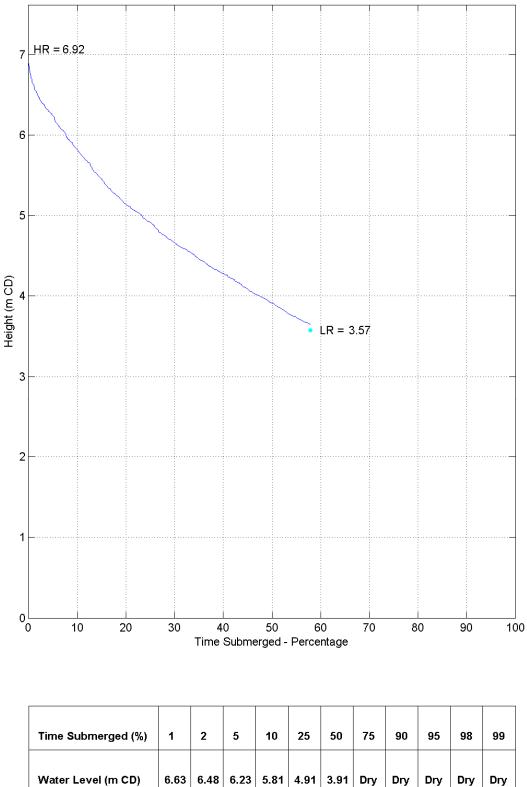
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [211,193]

Figure E-23: Submergence curve for 15 culvert simulation – Point 23



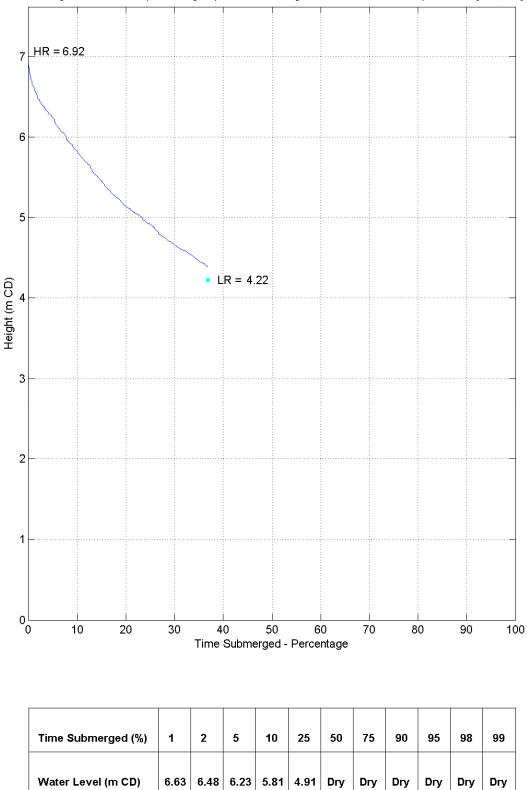
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [212,193]





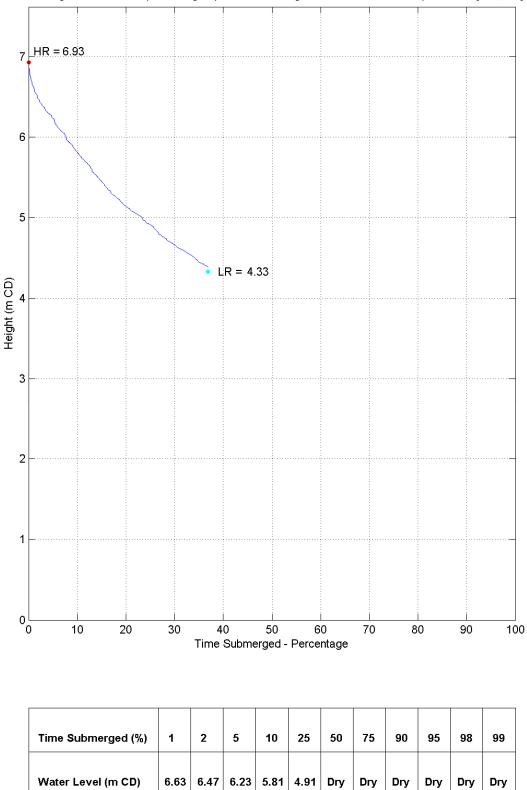
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [231,188]

Figure E-25: Submergence curve for 15 culvert simulation – Point 25



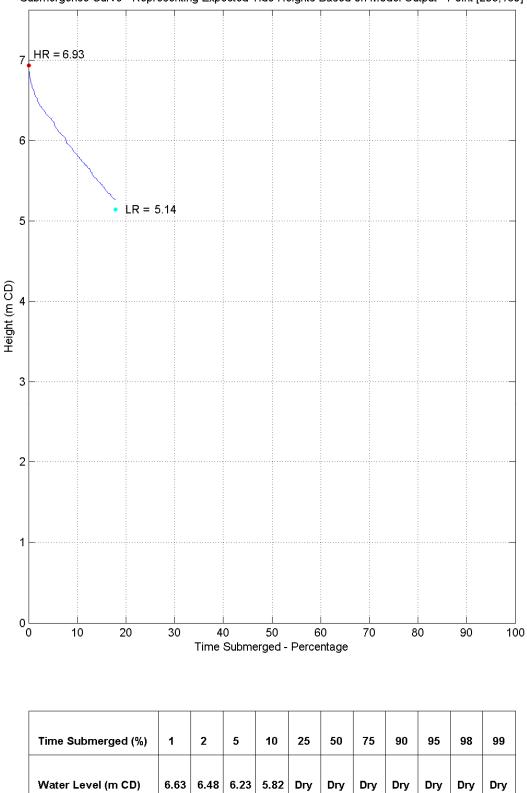
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [232,187]

Figure E-26: Submergence curve for 15 culvert simulation – Point 26



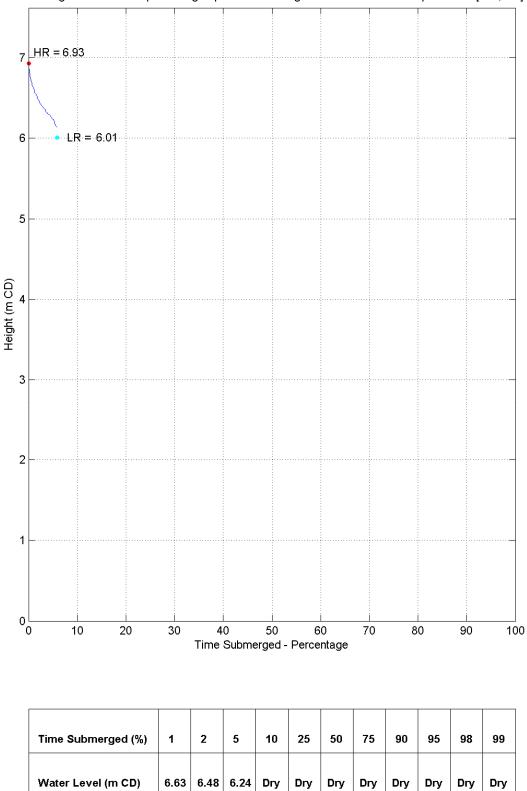
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [233,188]

Figure E-27: Submergence curve for 15 culvert simulation – Point 27



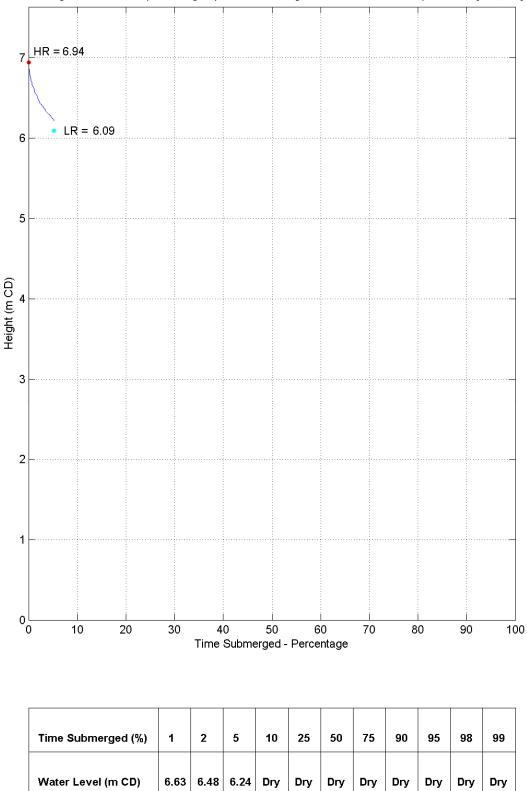
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [236,189]

Figure E-28: Submergence curve for 15 culvert simulation – Point 28



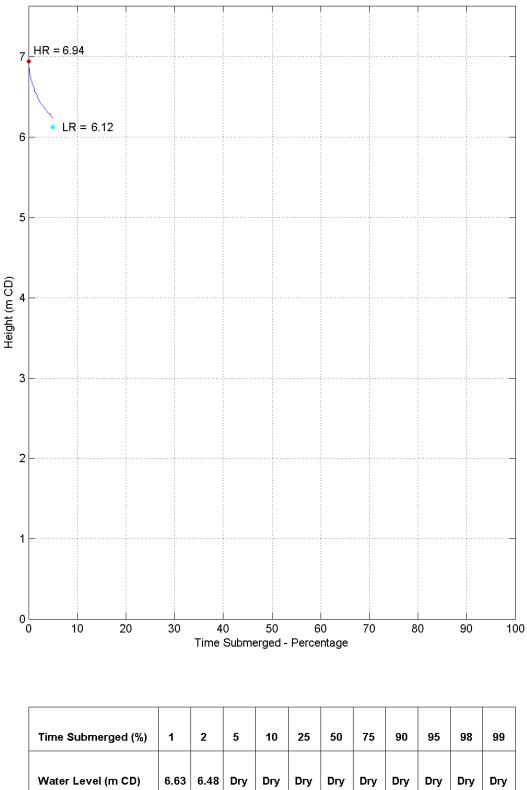
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [239,189]

Figure E-29: Submergence curve for 15 culvert simulation – Point 29

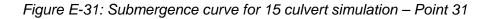


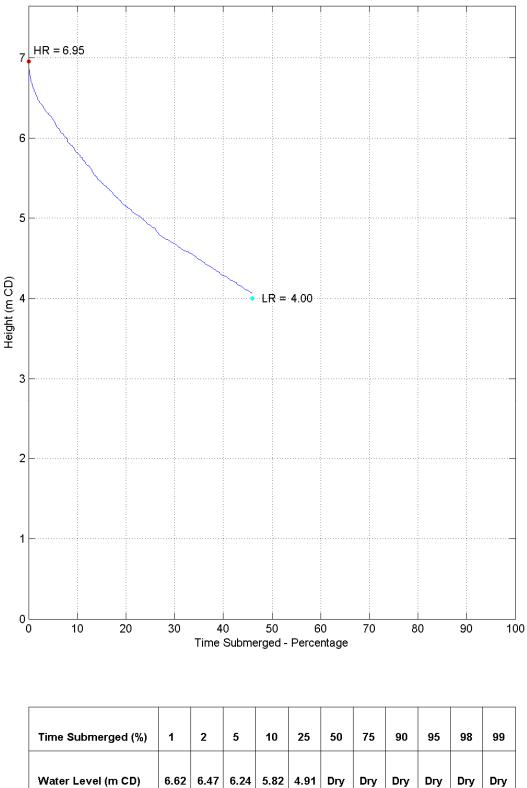
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [240,189]

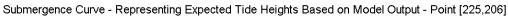
Figure E-30: Submergence curve for 15 culvert simulation – Point 30

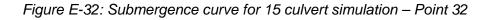


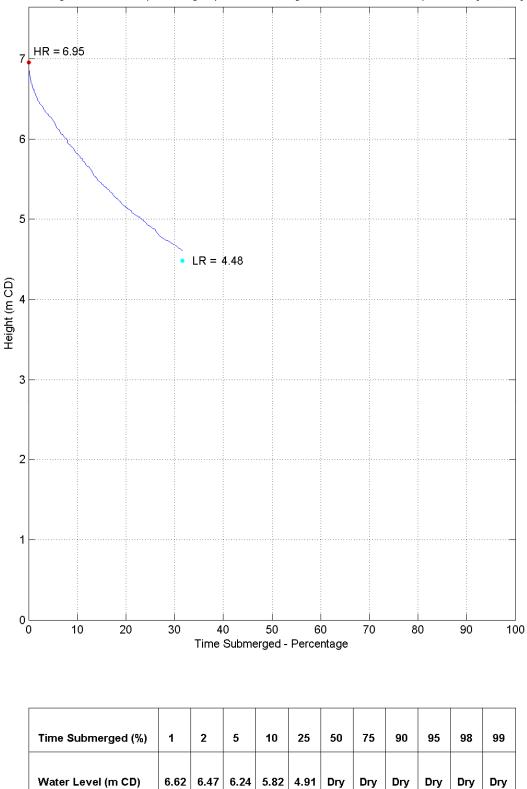
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [242,189]





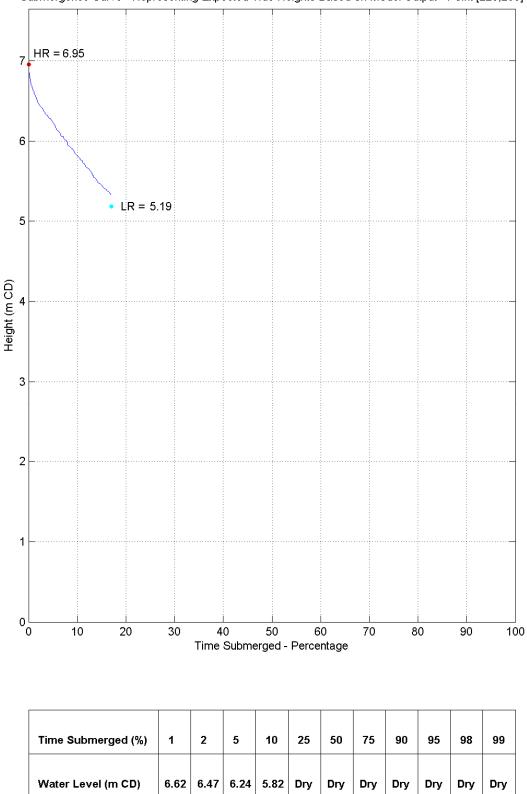






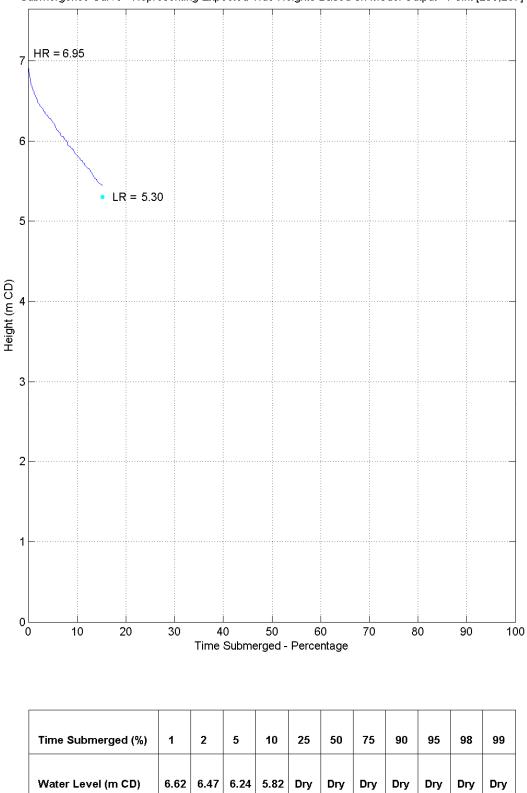
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [226,207]

Figure E-33: Submergence curve for 15 culvert simulation – Point 33



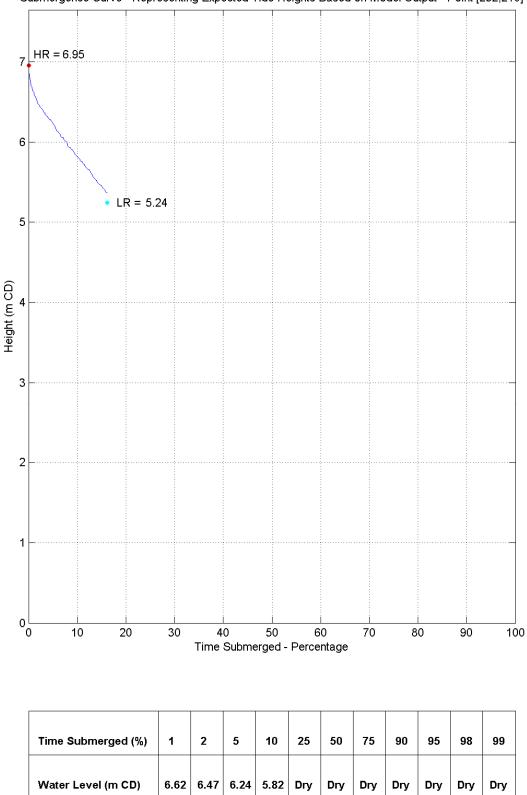
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [228,206]

Figure E-34: Submergence curve for 15 culvert simulation – Point 34



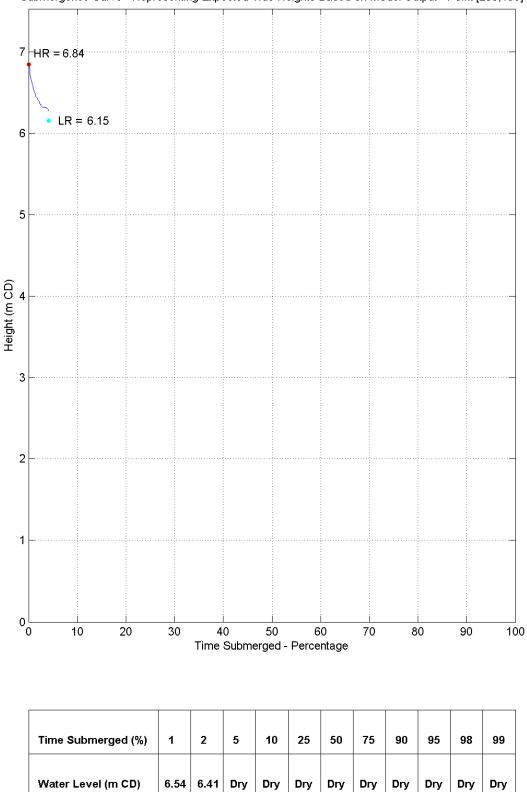
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [230,207]

Figure E-35: Submergence curve for 15 culvert simulation – Point 35



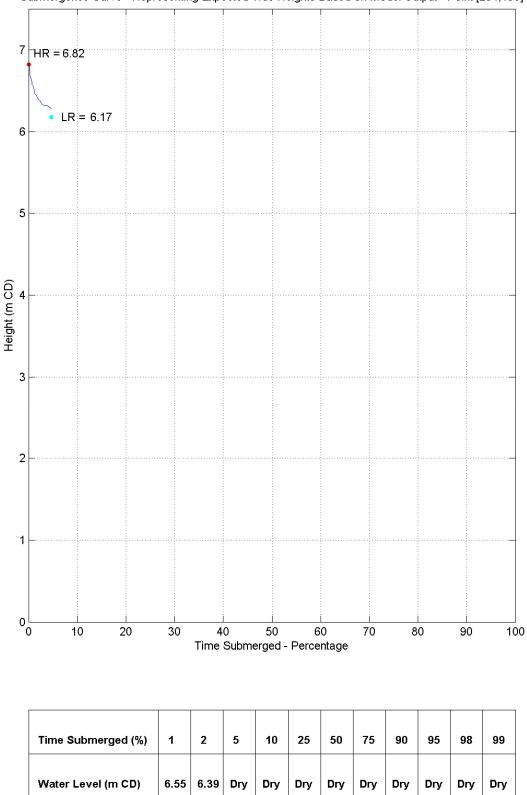
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [232,210]

Figure E-36: Submergence curve for 15 culvert simulation – Point 36



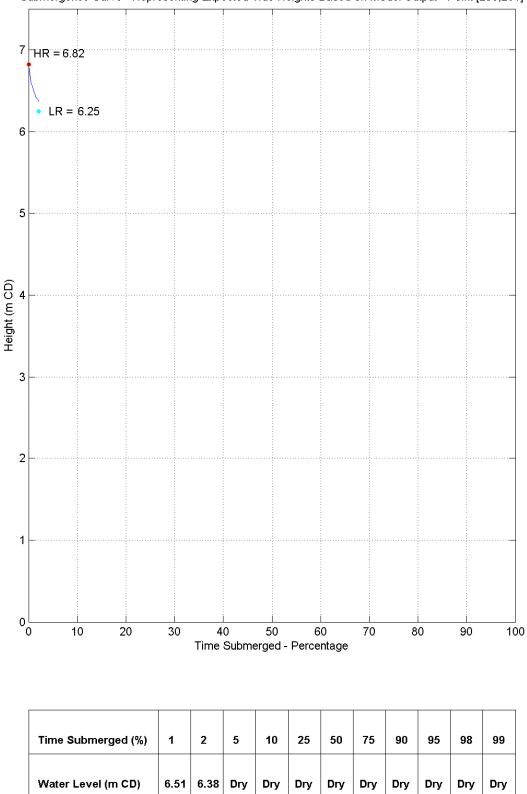
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [233,196]

Figure E-37: Submergence curve for 15 culvert simulation – Point 37



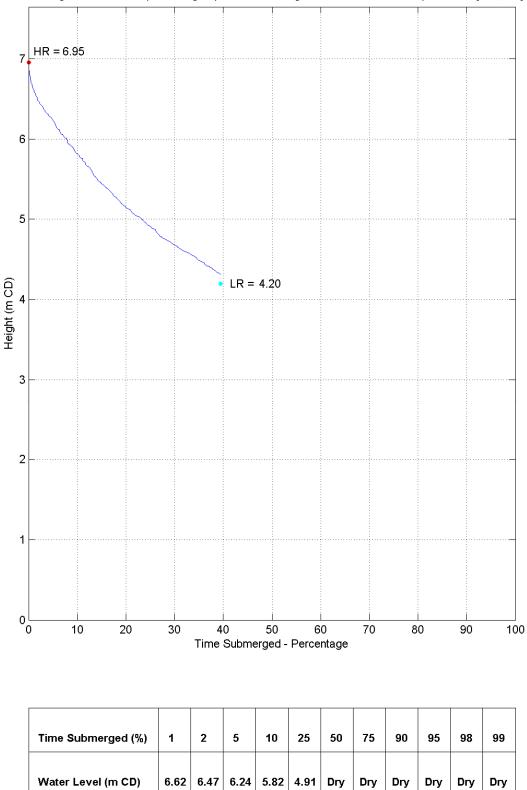
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [234,198]

Figure E-38: Submergence curve for 15 culvert simulation – Point 38



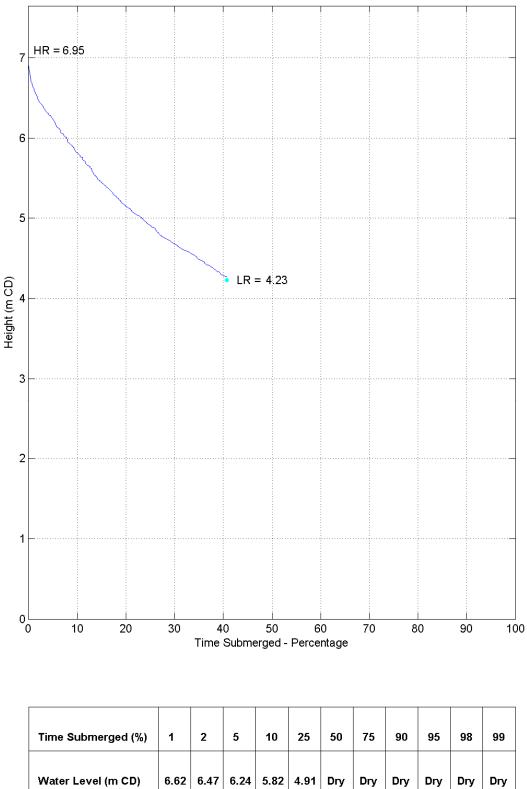
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [236,201]

Figure E-39: Submergence curve for 15 culvert simulation – Point 39



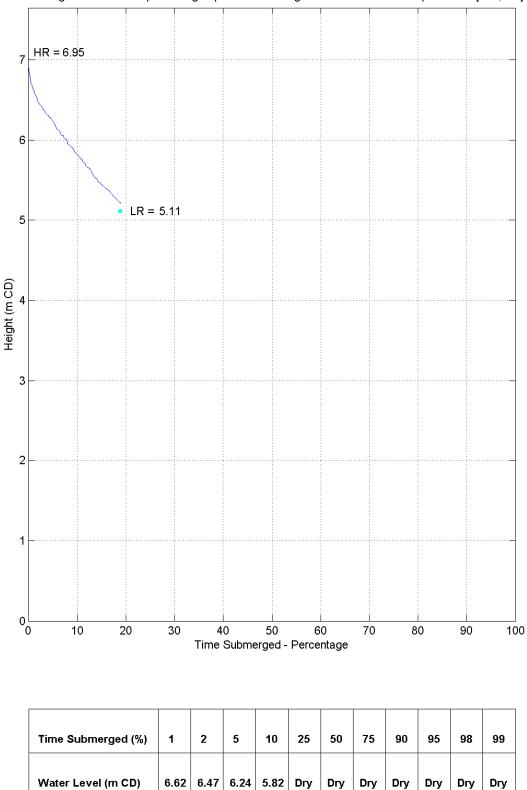
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [221,215]

Figure E-40: Submergence curve for 15 culvert simulation – Point 40



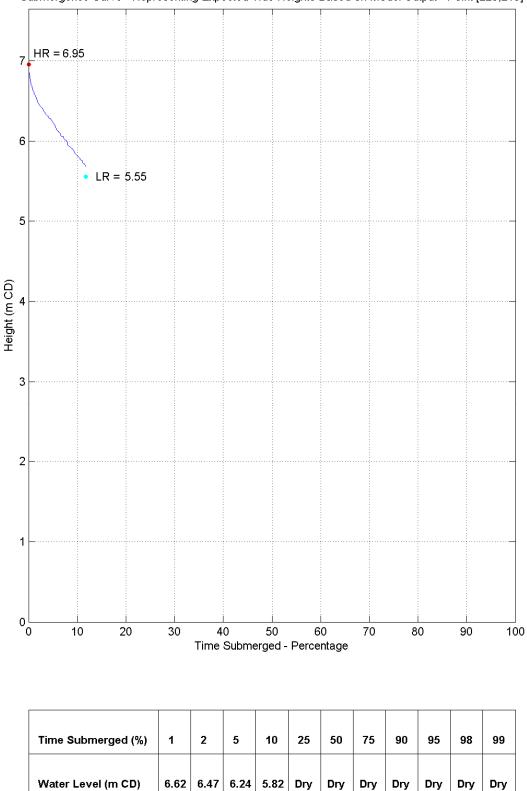
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [222,216]

Figure E-41: Submergence curve for 15 culvert simulation – Point 41



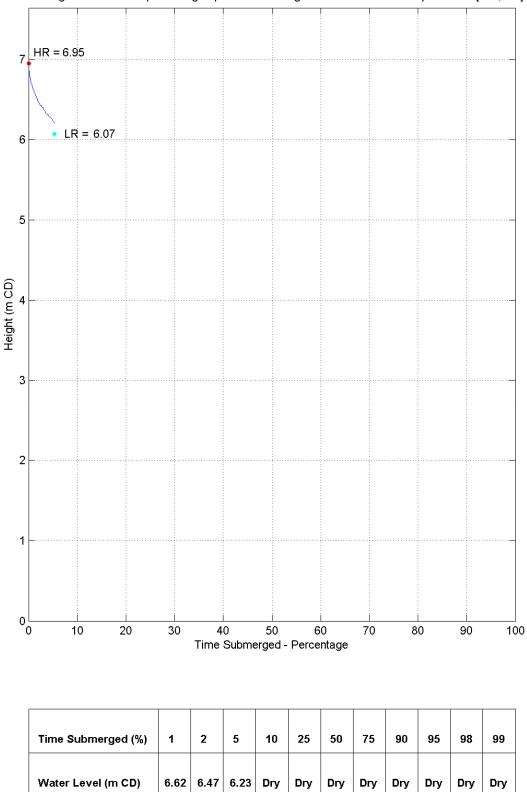
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [223,218]

Figure E-42: Submergence curve for 15 culvert simulation – Point 42



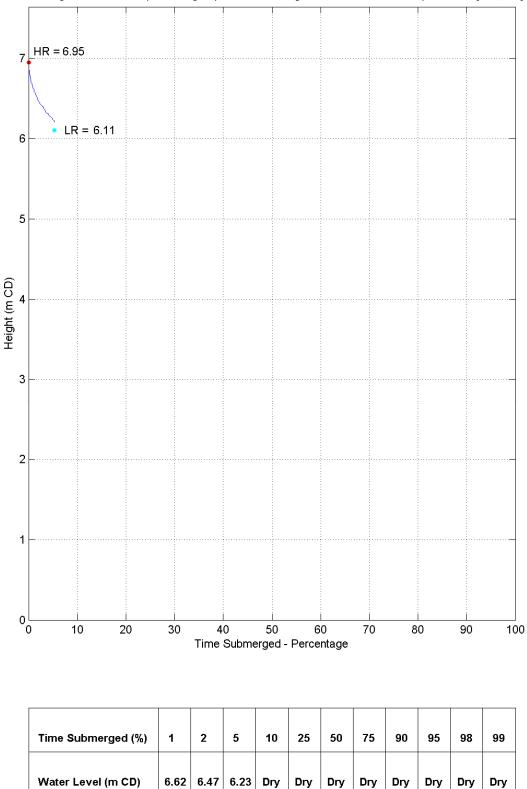
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [225,219]

Figure E-43: Submergence curve for 15 culvert simulation – Point 43



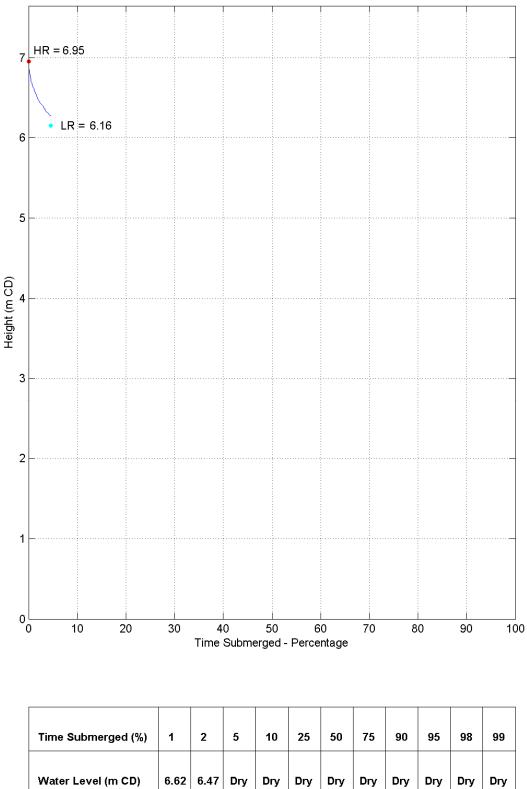
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [228,220]

Figure E-44: Submergence curve for 15 culvert simulation – Point 44



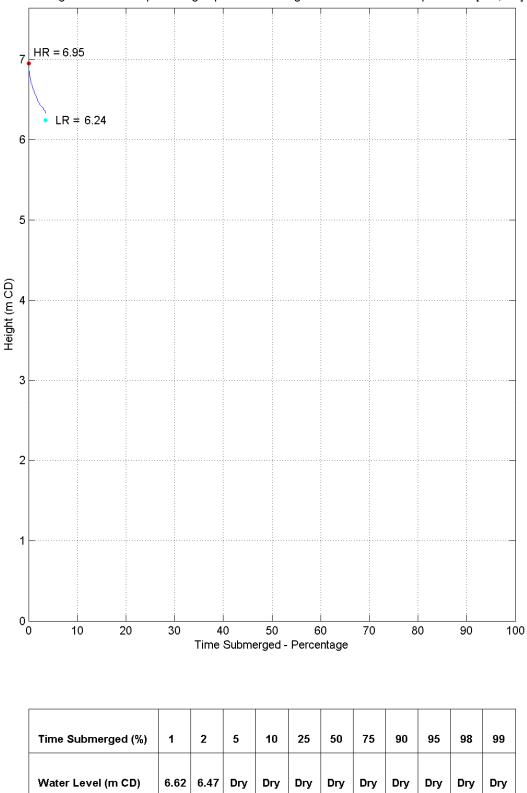
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [230,220]

Figure E-45: Submergence curve for 15 culvert simulation – Point 45

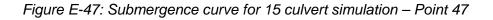


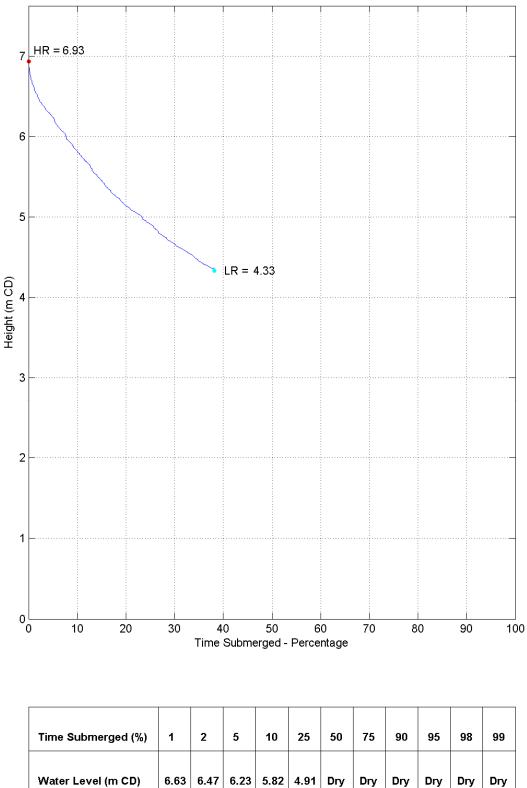
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [232,220]

Figure E-46: Submergence curve for 15 culvert simulation – Point 46



Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [234,221]





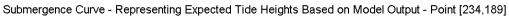
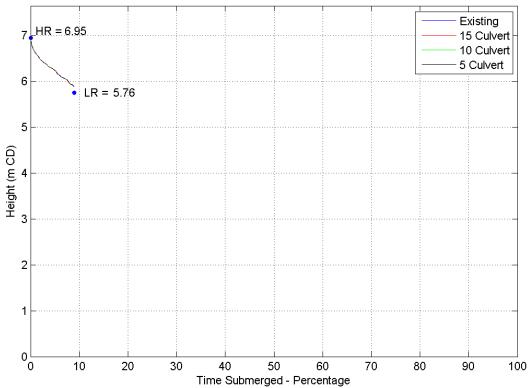


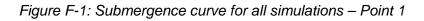
Figure E-48: Submergence curve for 15 culvert simulation – Point 48

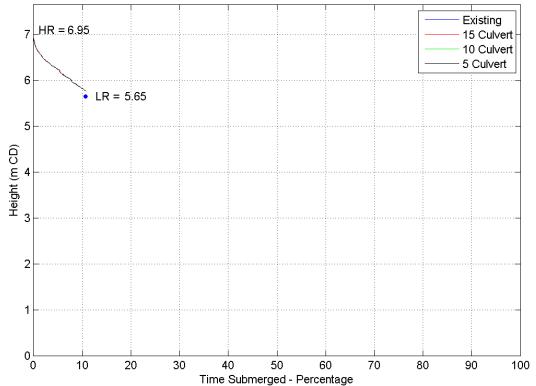
F. APPENDIX – COMPARISON SUBMERGENCE CURVES FOR ALL SIMULATIONS



Submergence Curve - Representing E	Eveneted Tide Unighte Deced or	n Madal Output Daint [210.172]
Submerdence Curve - Representing E	EXDECTED THE REPUBLIC DASED OF	

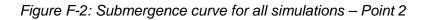
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	Dry							
15 Culvert WL (m CD)	6.62	6.47	6.23	Dry							
10 Culvert WL (m CD)	6.62	6.47	6.23	Dry							
5 Culvert WL (m CD)	6.62	6.47	6.23	Dry							

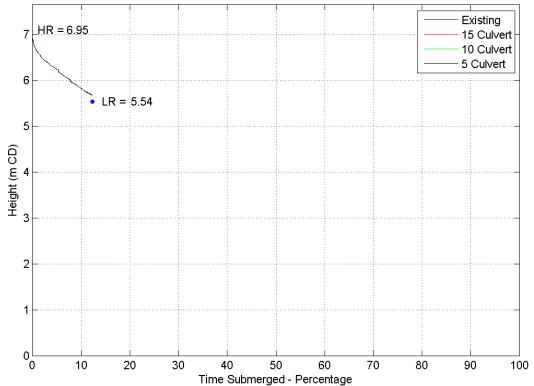




	Description of the second seco	d The Link where Design data and	Mandal Outrant Delight Odd 4701
Sunmergence Cun/e	- Representing Expected	1 LIDE HEIDDIS KASED OD	Model Output - Point [212,173]

Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	Dry						
15 Culvert WL (m CD)	6.62	6.47	6.23	5.82	Dry						
10 Culvert WL (m CD)	6.62	6.47	6.23	5.82	Dry						
5 Culvert WL (m CD)	6.62	6.47	6.23	5.82	Dry						





	Description of the second second second	The literation Description	Marcal at A start at	D - :-+ (044 470)
Sunmergence Curve	 Representing Expected 	LIDE HEIDDIS BASED OD	i Model Olithut -	Point 1214 1721
Gabinergenee Garve	rtoprosonting Expositod	That he for the second of the	i modol odipai	

Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	Dry						
15 Culvert WL (m CD)	6.62	6.47	6.23	5.82	Dry						
10 Culvert WL (m CD)	6.62	6.47	6.23	5.82	Dry						
5 Culvert WL (m CD)	6.62	6.47	6.23	5.82	Dry						

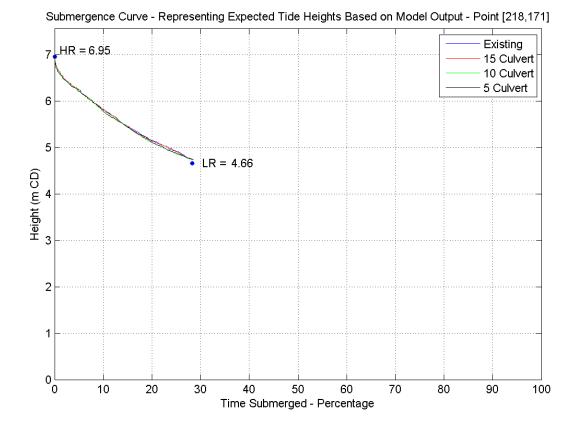


Figure F-3: Submergence curve for all simulations – Point 3

Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	4.92	Dry	Dry	Dry	Dry	Dry	Dry
15 Culvert WL (m CD)	6.63	6.48	6.23	5.81	4.91	Dry	Dry	Dry	Dry	Dry	Dry
10 Culvert WL (m CD)	6.61	6.45	6.23	5.78	4.85	Dry	Dry	Dry	Dry	Dry	Dry
5 Culvert WL (m CD)	6.58	6.46	6.2	5.79	4.87	Dry	Dry	Dry	Dry	Dry	Dry

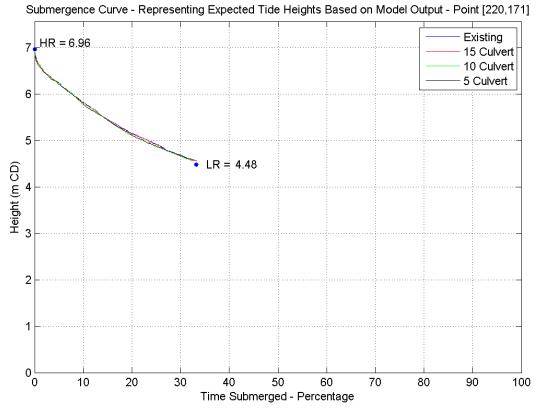
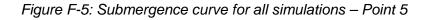
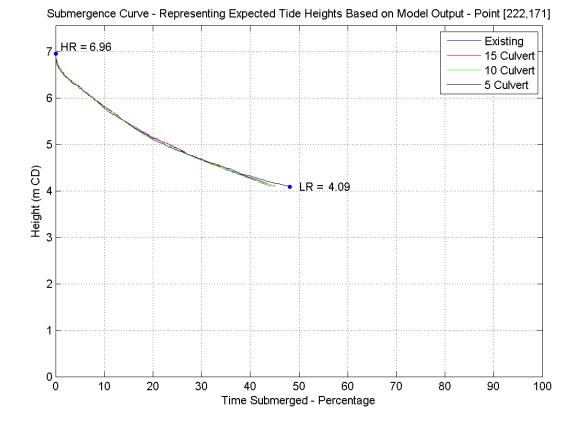


Figure F-4: Submergence curve for all simulations – Point 4

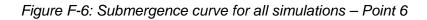
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	4.92	Dry	Dry	Dry	Dry	Dry	Dry
15 Culvert WL (m CD)	6.63	6.48	6.23	5.81	4.91	Dry	Dry	Dry	Dry	Dry	Dry
10 Culvert WL (m CD)	6.61	6.45	6.23	5.78	4.86	Dry	Dry	Dry	Dry	Dry	Dry
5 Culvert WL (m CD)	6.58	6.46	6.2	5.79	4.87	Dry	Dry	Dry	Dry	Dry	Dry

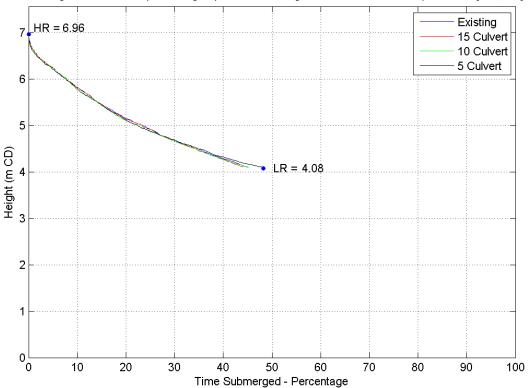




Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	4.92	Dry	Dry	Dry	Dry	Dry	Dry
15 Culvert WL (m CD)	6.63	6.48	6.23	5.81	4.91	Dry	Dry	Dry	Dry	Dry	Dry
10 Culvert WL (m CD)	6.61	6.45	6.23	5.78	4.86	Dry	Dry	Dry	Dry	Dry	Dry
5 Culvert WL (m CD)	6.58	6.45	6.2	5.79	4.88	Dry	Dry	Dry	Dry	Dry	Dry

West Creek tidal inundation Appendix.doc





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Submerdence Curve	 Representing Expected 	a fide Heidnis Based d	n wodel Ouldul -	- POINLIZZS, 17 11

Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	4.92	Dry	Dry	Dry	Dry	Dry	Dry
15 Culvert WL (m CD)	6.63	6.48	6.23	5.81	4.91	Dry	Dry	Dry	Dry	Dry	Dry
10 Culvert WL (m CD)	6.61	6.45	6.23	5.78	4.86	Dry	Dry	Dry	Dry	Dry	Dry
5 Culvert WL (m CD)	6.58	6.46	6.2	5.79	4.88	Dry	Dry	Dry	Dry	Dry	Dry

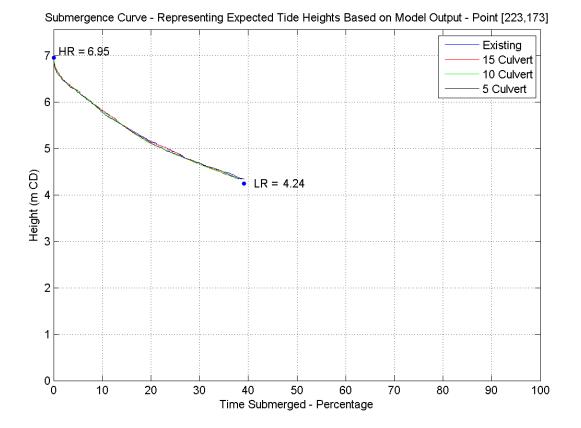
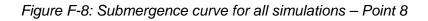
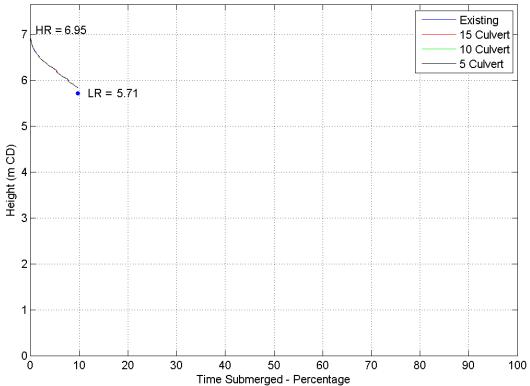


Figure F-7: Submergence curve for all simulations – Point 7

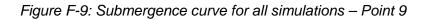
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	4.92	Dry	Dry	Dry	Dry	Dry	Dry
15 Culvert WL (m CD)	6.63	6.48	6.23	5.81	4.91	Dry	Dry	Dry	Dry	Dry	Dry
10 Culvert WL (m CD)	6.61	6.45	6.23	5.78	4.86	Dry	Dry	Dry	Dry	Dry	Dry
5 Culvert WL (m CD)	6.58	6.45	6.2	5.79	4.88	Dry	Dry	Dry	Dry	Dry	Dry

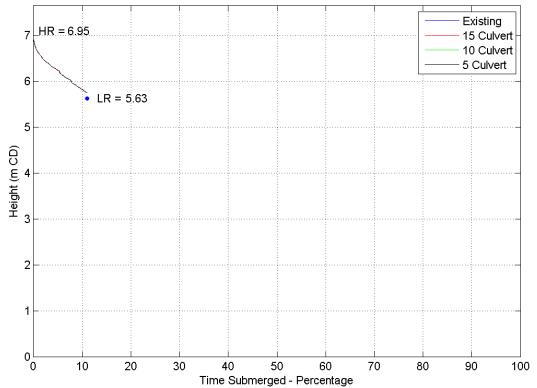




Cubmorgonoo Cun/o	 Representing Expected 	d Tida I laimhta Daoad ar		Daint [207 477]
	- Representing Expecter	a fide Heidnis Based of	n Model Ouldul ·	- POINLIZU7.1771
5		5		

Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	Dry							
15 Culvert WL (m CD)	6.62	6.47	6.23	Dry							
10 Culvert WL (m CD)	6.62	6.47	6.23	Dry							
5 Culvert WL (m CD)	6.62	6.47	6.23	Dry							

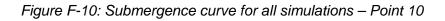


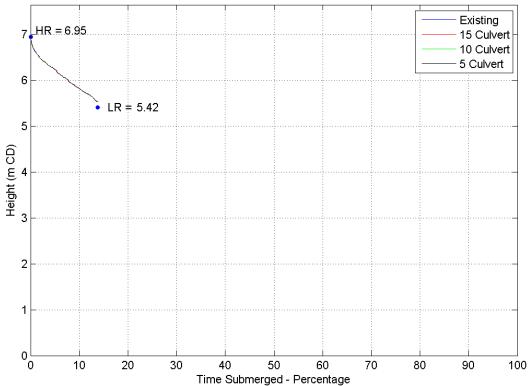


Submergence Curve -	. Representing Ex	nected Tide Hei	ights Based on M	Indel Output -	Point [209 176]
Submorgence Surve	Representing Ex	poolog ngo no	ignis busca on iv	nouci output	

Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	Dry						
15 Culvert WL (m CD)	6.62	6.47	6.23	5.82	Dry						
10 Culvert WL (m CD)	6.62	6.47	6.23	5.82	Dry						
5 Culvert WL (m CD)	6.62	6.47	6.23	5.82	Dry						

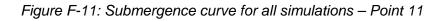
West Creek tidal inundation Appendix.doc

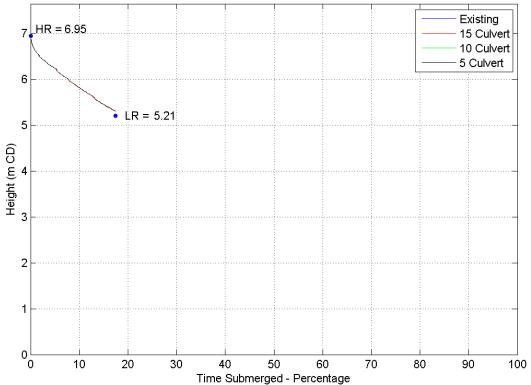




Outeman and Outeman F	Demande a mélim au Essan a céla d	Tide Haimble Deserter		D-1-4 [040 477]
Submergence Curve - F	Representing Expected	TIDE HEIDNIS Based of	1 Model Outbut ·	- Point 1212.1771

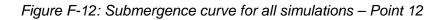
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	Dry						
15 Culvert WL (m CD)	6.62	6.47	6.23	5.82	Dry						
10 Culvert WL (m CD)	6.62	6.47	6.23	5.82	Dry						
5 Culvert WL (m CD)	6.62	6.47	6.23	5.82	Dry						

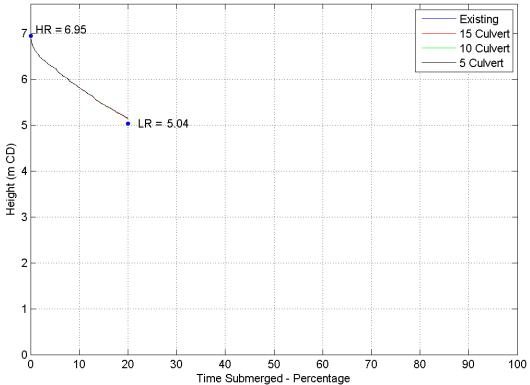




Cubmorgenee Cun/o	Depresenting Eveste	d Tida Haiabta Daaad a		Daint [04.4.470]
Submergence Curve -	- Representing Expecte	d Tide Heights Based o	n Model Ouldul ·	- POINLIZIA.1/01
		- · · · · · · · · · · · · · · · · · · ·		[=,]

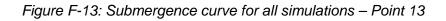
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	Dry						
15 Culvert WL (m CD)	6.62	6.47	6.23	5.82	Dry						
10 Culvert WL (m CD)	6.62	6.48	6.24	5.82	Dry						
5 Culvert WL (m CD)	6.62	6.47	6.24	5.82	Dry						

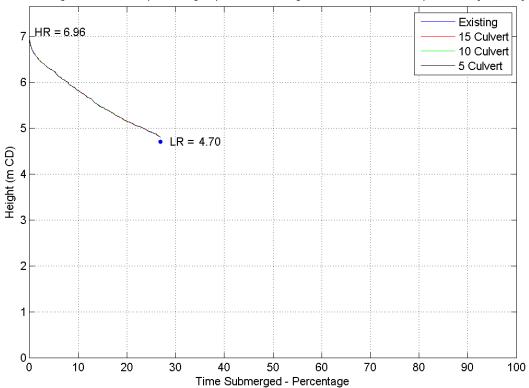




Cubmorgenee Cunic Der	waaanting Evalated Tid	la Ilaimhta Daaad an		Daint [045 470]
Submergence Curve - Rep	presentina Expectea Ha	ie Heidnis Based on	Wodel Ouldul -	POINLIZ15.1791
				[=,]

Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	Dry						
15 Culvert WL (m CD)	6.62	6.47	6.23	5.82	Dry						
10 Culvert WL (m CD)	6.62	6.48	6.24	5.82	Dry						
5 Culvert WL (m CD)	6.62	6.47	6.24	5.82	Dry						





	Description of the second second second	al That a liter adapted to be a set of a set	Madel Output Delet 1047 4001
Sunmergence Curve	- Representing Expected	n Lide Heidnis Based or	n Model Output - Point [217,180]
Gabinioi gonoo Gaivo	Troprosonting Exposito	a mao monginto babba or	

Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	4.92	Dry	Dry	Dry	Dry	Dry	Dry
15 Culvert WL (m CD)	6.62	6.47	6.24	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry
10 Culvert WL (m CD)	6.62	6.48	6.24	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry
5 Culvert WL (m CD)	6.62	6.47	6.24	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry

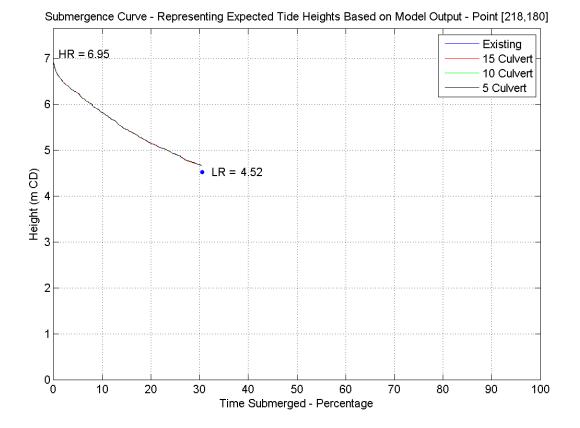
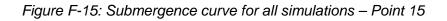
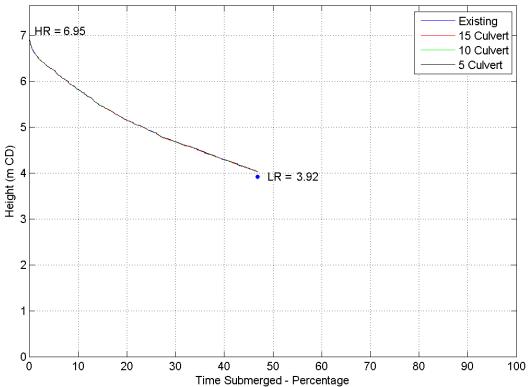


Figure F-14: Submergence curve for all simulations – Point 14

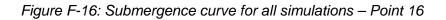
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	4.92	Dry	Dry	Dry	Dry	Dry	Dry
15 Culvert WL (m CD)	6.62	6.47	6.24	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry
10 Culvert WL (m CD)	6.62	6.48	6.24	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry
5 Culvert WL (m CD)	6.62	6.47	6.24	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry

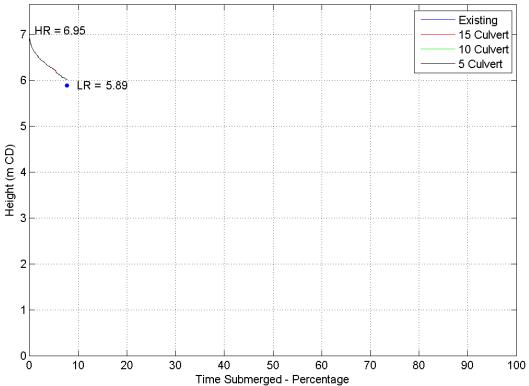




Submergence Curve -	 Representing Expects 	ed Tide Heights Based o	on Model Output - Point	: [220 180]
Capition geneo Calivo	Roprosonting Expose	ba mao morginto Dabba (. [220, 100]

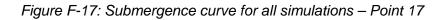
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	4.92	Dry	Dry	Dry	Dry	Dry	Dry
15 Culvert WL (m CD)	6.62	6.47	6.24	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry
10 Culvert WL (m CD)	6.62	6.48	6.24	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry
5 Culvert WL (m CD)	6.62	6.47	6.24	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry

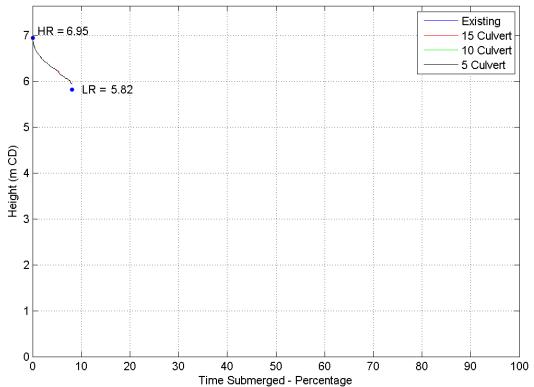




Cubmorgenee Cum/e	Depresenting Evester	Tida Llaighta Daaad ar		Deint [202 400]
Submergence Curve -	 Representing Expected 	I Fide Heidnis Based of	i Model Oulbul -	POINT 1202, 1901
5		9		

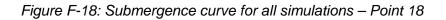
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	Dry							
15 Culvert WL (m CD)	6.62	6.47	6.23	Dry							
10 Culvert WL (m CD)	6.62	6.47	6.23	Dry							
5 Culvert WL (m CD)	6.62	6.47	6.23	Dry							

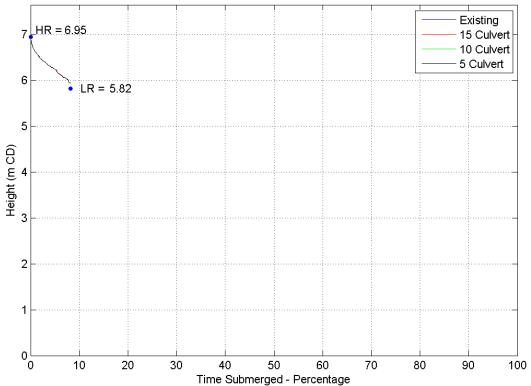




	Demonstration of Demonstrates	The I have been presented as	Mandal Ordered	D-1-1 1004 4041
Sunmergence Cun/e -	 Representing Expected 	LIDE HEIDDIS Kased of	NINDAEL OUTDUT -	

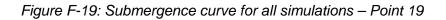
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	Dry							
15 Culvert WL (m CD)	6.62	6.47	6.23	Dry							
10 Culvert WL (m CD)	6.62	6.47	6.23	Dry							
5 Culvert WL (m CD)	6.62	6.47	6.23	Dry							

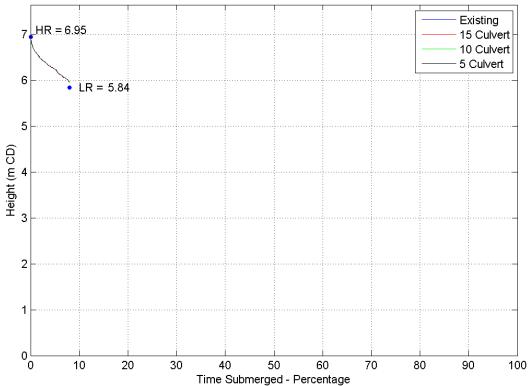




Outors and a Outor Description	while a Francische d'Tide I	Laterate Description Manufact	Output Delations (001
Submergence Curve - Represe	entina Expectea. Lide E	Helants Based on Miodel	UUIDUI - POIDI 1705 1971
Gabinioi genee Gaire Treprese	mang Exposed maon	Telgino Duoca en model	output i onit[Loo, ioz]

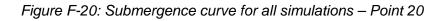
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	Dry							
15 Culvert WL (m CD)	6.62	6.47	6.23	Dry							
10 Culvert WL (m CD)	6.62	6.47	6.23	Dry							
5 Culvert WL (m CD)	6.62	6.47	6.23	Dry							

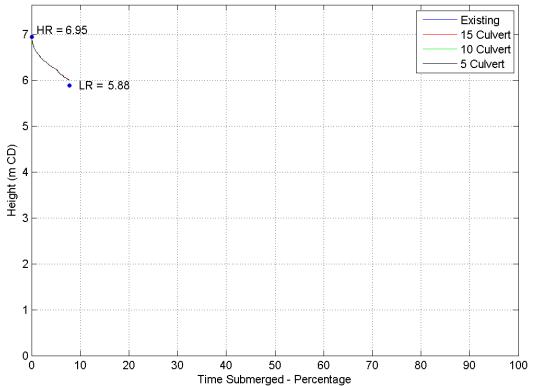




Submergence Curve -	Depresenting Dyna	ated Tide Llaighte	Deced on Medal Outry	4 Detet [200 402]
Submerdence Curve -	Representing Expe	clea nae Helanis.	Based on Model Outpu	i - Point 1200, 1931
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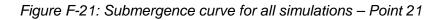
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	Dry							
15 Culvert WL (m CD)	6.62	6.47	6.23	Dry							
10 Culvert WL (m CD)	6.62	6.47	6.24	Dry							
5 Culvert WL (m CD)	6.62	6.47	6.24	Dry							

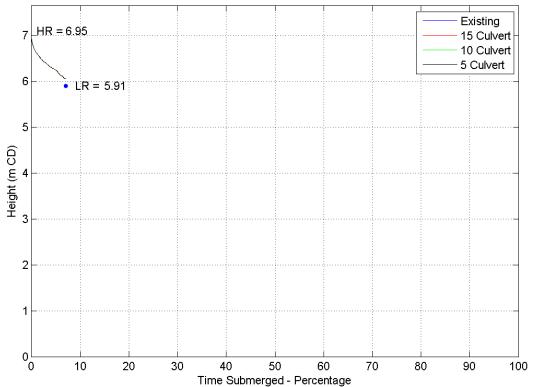




Submergence Curve -	Representing Evr	ected Tide Height	s Based on Model Outpi	it - Doint [209 194]
Oubmorgence ourve -	поргозопану шлр	oolou nuo noigiili	a Daacu on mouch outp	at = 1 Onit [200, 10+]

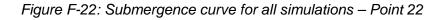
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	Dry							
15 Culvert WL (m CD)	6.62	6.47	6.24	Dry							
10 Culvert WL (m CD)	6.62	6.47	6.24	Dry							
5 Culvert WL (m CD)	6.62	6.47	6.24	Dry							

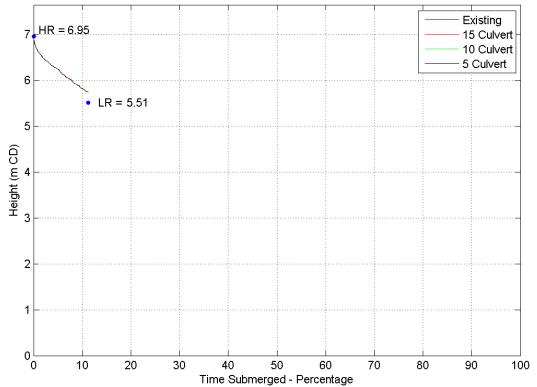




Submergence Curve -	Representing Evpe	cted Tide Heights	Based on Model Output	- Doint [210 193]
Oubmorgence ourve -	representing Expe	olou nuo noigilia	Dascu on Mouci Output	- i onic [210,100]

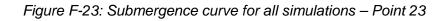
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	Dry							
15 Culvert WL (m CD)	6.62	6.47	6.24	Dry							
10 Culvert WL (m CD)	6.62	6.47	6.24	Dry							
5 Culvert WL (m CD)	6.62	6.47	6.24	Dry							

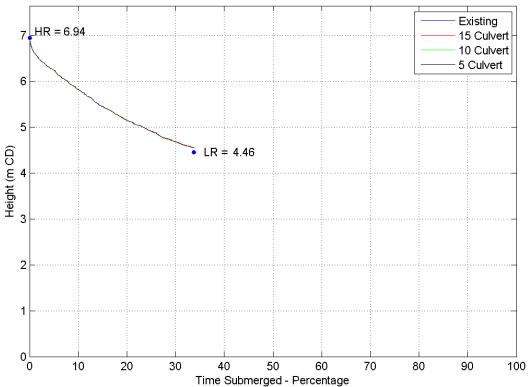




Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [211,193]

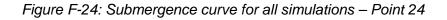
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	Dry						
15 Culvert WL (m CD)	6.62	6.47	6.24	5.82	Dry						
10 Culvert WL (m CD)	6.62	6.47	6.24	5.82	Dry						
5 Culvert WL (m CD)	6.62	6.47	6.24	5.82	Dry						

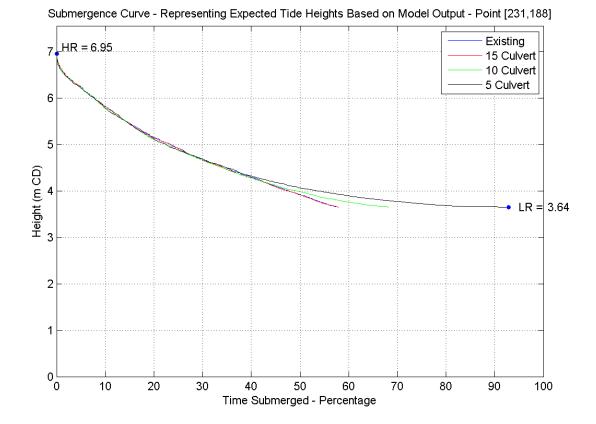




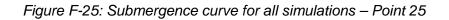
Submergence Curve - R	Demuse e embine en Esse e ebe el	Tide Haimble Deesel as	المنصفين المامية	D -:-+ [040 400]
Submergence Curve - R	kepresentina Expectea	TIDE HEIDNIS BASED OF	1 IVIODEI UUTDUT ·	• Point 1212 1931
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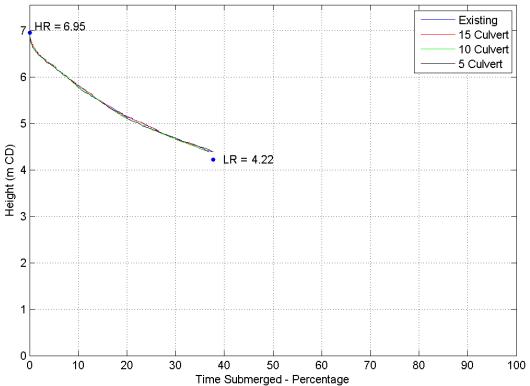
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	4.92	Dry	Dry	Dry	Dry	Dry	Dry
15 Culvert WL (m CD)	6.62	6.47	6.24	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry
10 Culvert WL (m CD)	6.62	6.47	6.24	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry
5 Culvert WL (m CD)	6.62	6.47	6.24	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry





Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	4.92	3.92	Dry	Dry	Dry	Dry	Dry
				5.04							
15 Culvert WL (m CD)	6.63	6.48	6.23	5.81	4.91	3.91	Dry	Dry	Dry	Dry	Dry
10 Culvert WL (m CD)	6.61	6.45	6.23	5.78	4.87	3.98	Dry	Dry	Dry	Dry	Dry
5 Culvert WL (m CD)	6.58	6.46	6.2	5.79	4.88	4.06	3.72	3.66	Dry	Dry	Dry





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Submergence Curve - Re	ebresentina Expectea	Tide Heidnis Based on	i iviodei Uutbut -	• Point 1232.1871
	ohi e e e i i i i i i i i i i i i i i i i			[,]

Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	4.92	Dry	Dry	Dry	Dry	Dry	Dry
15 Culvert WL (m CD)	6.63	6.48	6.23	5.81	4.91	Dry	Dry	Dry	Dry	Dry	Dry
10 Culvert WL (m CD)	6.61	6.45	6.23	5.78	4.87	Dry	Dry	Dry	Dry	Dry	Dry
5 Culvert WL (m CD)	6.58	6.46	6.2	5.79	4.88	Dry	Dry	Dry	Dry	Dry	Dry

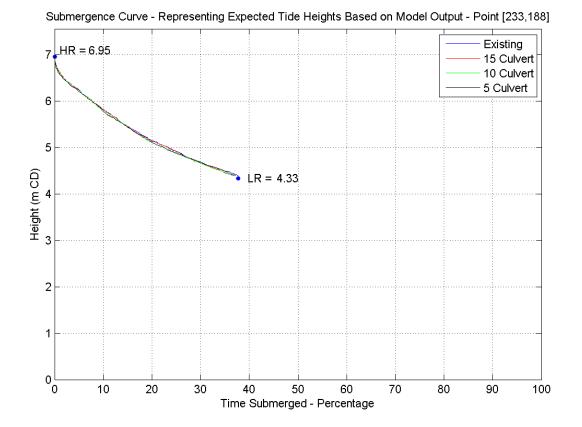
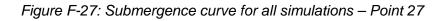
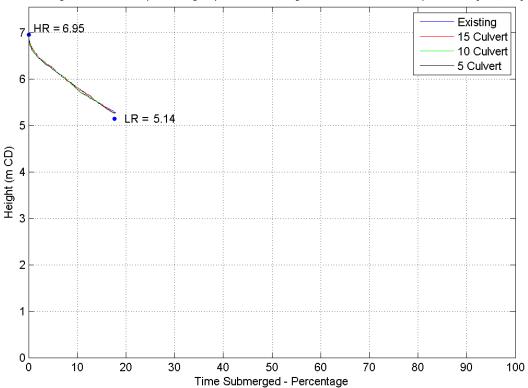


Figure F-26: Submergence curve for all simulations – Point 26

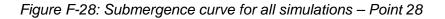
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	4.92	Dry	Dry	Dry	Dry	Dry	Dry
15 Culvert WL (m CD)	6.63	6.47	6.23	5.81	4.91	Dry	Dry	Dry	Dry	Dry	Dry
10 Culvert WL (m CD)	6.61	6.45	6.22	5.78	4.87	Dry	Dry	Dry	Dry	Dry	Dry
5 Culvert WL (m CD)	6.58	6.45	6.2	5.79	4.88	Dry	Dry	Dry	Dry	Dry	Dry

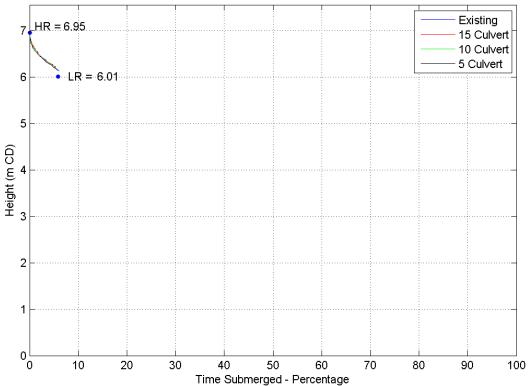




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Submergence Curve - Re	ebresenund Expected.	The Heldnis Based o	n iviodel Ouldul ·	- POINLIZSB. 1091
	-p			[,]

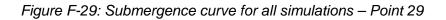
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.63	6.47	6.23	5.82	Dry						
15 Culvert WL (m CD)	6.63	6.48	6.23	5.82	Dry						
10 Culvert WL (m CD)	6.61	6.45	6.22	5.78	Dry						
5 Culvert WL (m CD)	6.58	6.46	6.2	5.79	Dry						

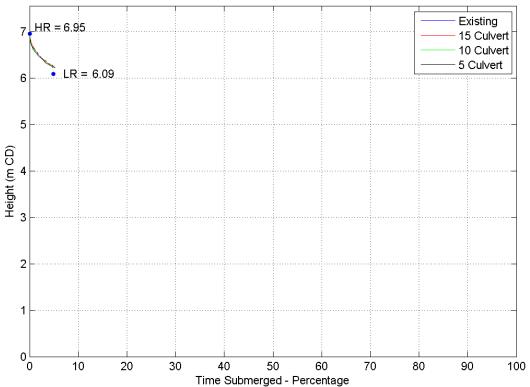




Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.63	6.47	6.24	Dry							
15 Culvert WL (m CD)	6.63	6.48	6.24	Dry							
10 Culvert WL (m CD)	6.61	6.45	6.22	Dry							
5 Culvert WL (m CD)	6.58	6.46	6.2	Dry							

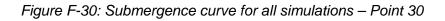
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [239,189]

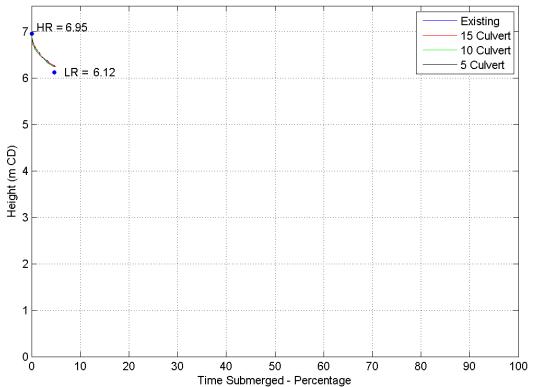




Submergence Curve	 Representing Expected 	d Tide Heights Rased (on Model Output - Point [240,189]
oubmorgenee ourve	Representing Expecte	a machioigints basea (

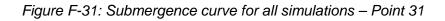
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.63	6.47	6.24	Dry							
15 Culvert WL (m CD)	6.63	6.48	6.24	Dry							
10 Culvert WL (m CD)	6.61	6.45	6.22	Dry							
5 Culvert WL (m CD)	6.58	6.46	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry

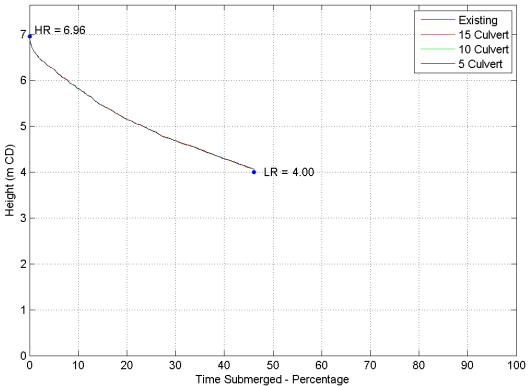




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Sunmergence Curve - Ren	resenting Expected Lide	Heidnis Based on Mi	odel Output - Point [242,189]

Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.63	6.47	Dry								
15 Culvert WL (m CD)	6.63	6.48	Dry								
10 Culvert WL (m CD)	6.61	6.45	Dry								
5 Culvert WL (m CD)	6.58	6.46	Dry								





Cubmarganaa Cunva	Depresenting Expected	LTida Llaighta Dagad ai		Date [225 200]
Submerdence Curve -	Representing Expected	I The Heights Based of	n Model Ouldul -	- POINLIZZS.ZUDI
		· · · · · · · · · · · · · · · · · · ·		

Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	4.92	Dry	Dry	Dry	Dry	Dry	Dry
15 Culvert WL (m CD)	6.62	6.47	6.24	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry
10 Culvert WL (m CD)	6.62	6.47	6.24	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry
5 Culvert WL (m CD)	6.62	6.47	6.24	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry

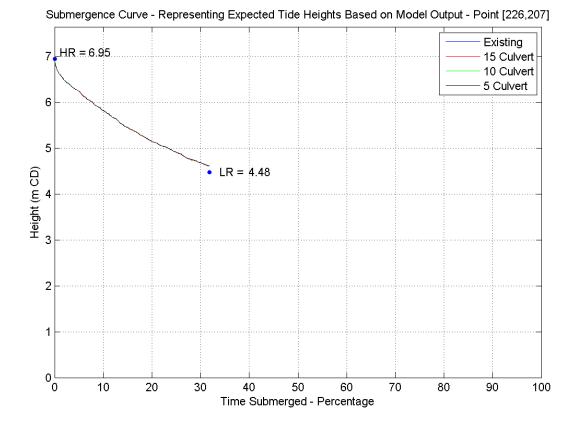
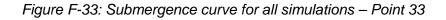
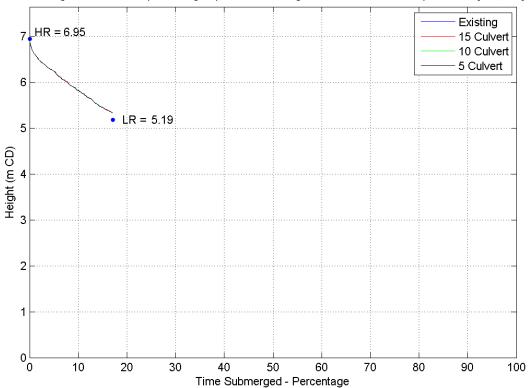


Figure F-32: Submergence curve for all simulations – Point 32

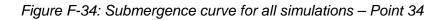
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	4.92	Dry	Dry	Dry	Dry	Dry	Dry
15 Culvert WL (m CD)	6.62	6.47	6.24	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry
10 Culvert WL (m CD)	6.62	6.47	6.24	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry
5 Culvert WL (m CD)	6.62	6.47	6.24	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry

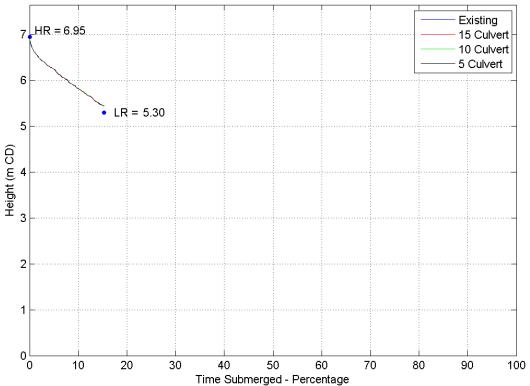




Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	Dry	Dry	Dry	Dry	Dry	Dry	Dry
15 Culvert WL (m CD)	6.62	6.47	6.24	5.82	Dry	Dry	Dry	Dry	Dry	Dry	Dry
	0.02		0.2.1	0.02	<i></i>	.,					
10 Culvert WL (m CD)	6.62	6.47	6.24	5.82	Dry	Dry	Dry	Dry	Dry	Dry	Dry
5 Culvert WL (m CD)	6.62	6.47	6.24	5.82	Dry	Dry	Dry	Dry	Dry	Dry	Dry

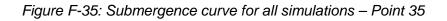
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [228,206]

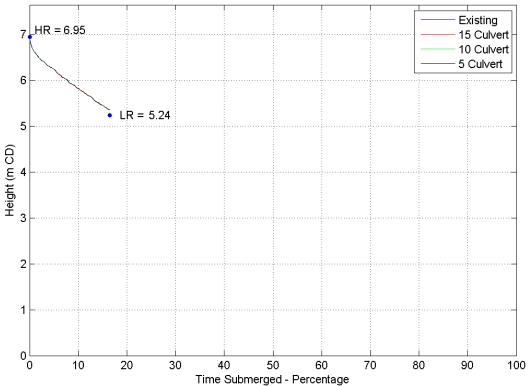




	and a section of Example of a HTML	. I I a Sanda Ala Dia a a al any Kata ata	LOUIS D. S. 1000 0071
Submergence Curve - Re	enresenting Expected Lig	e Heldhis Based on Miode	el Olifolit - Point 1230 2071
Gabinioi genee Gaive Tre	prosoning Exposion na	s noights based on meat	

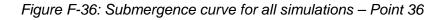
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	Dry						
15 Culvert WL (m CD)	6.62	6.47	6.24	5.82	Dry						
10 Culvert WL (m CD)	6.62	6.47	6.24	5.82	Dry						
5 Culvert WL (m CD)	6.62	6.47	6.24	5.82	Dry						

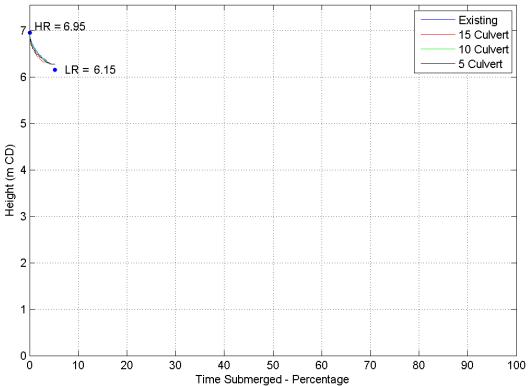




Cubmonanan Cum/a	 Representing Expected 	Tida Ilaiahta Daaada		Deint [222 240]
Submerdence Curve	- Representing Expected	I FIDE HEIDINS BASED O	n Model Ouldul -	- POINLIZSZ.ZTUL
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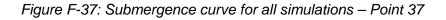
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	Dry						
15 Culvert WL (m CD)	6.62	6.47	6.24	5.82	Dry						
10 Culvert WL (m CD)	6.62	6.47	6.24	5.82	Dry						
5 Culvert WL (m CD)	6.62	6.47	6.24	5.82	Dry						

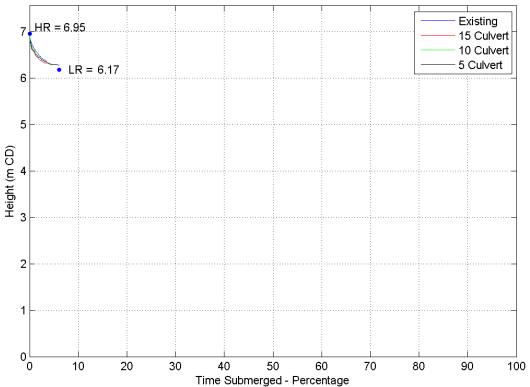




Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry
15 Culvert WL (m CD)	6.54	6.41	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry
10 Culvert WL (m CD)	6.61	6.45	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry
5 Culvert WL (m CD)	6.57	6.45	6.28	Dry							

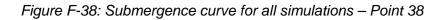
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [233,196]

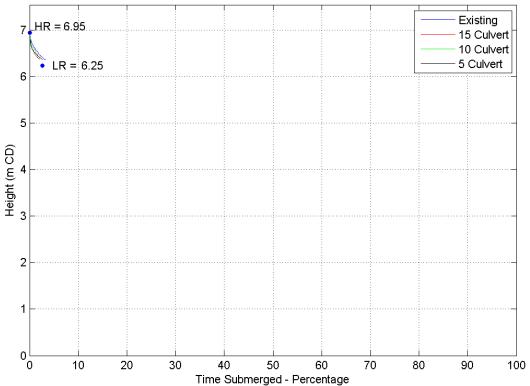




Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry
15 Culvert WL (m CD)	6.55	6.39	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry
10 Culvert WL (m CD)	6.59	6.45	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry	Dry
5 Culvert WL (m CD)	6.56	6.44	6.28	Dry							

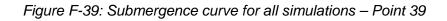
Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [234,198]

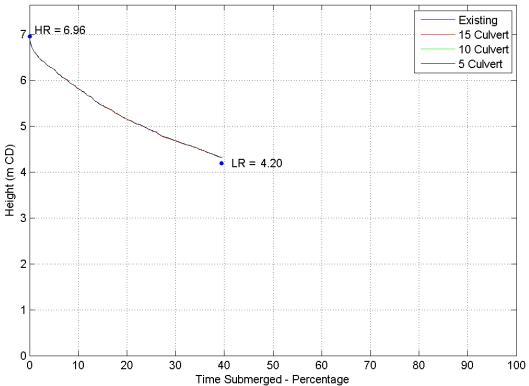




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Submergence Curve - Repre	esenting Expected Lide	e Heldnis Based on Mior	1el Outnut - Point 1236 2011
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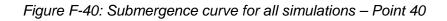
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	Dry								
15 Culvert WL (m CD)	6.51	6.38	Dry								
10 Culvert WL (m CD)	6.57	6.43	Dry								
5 Culvert WL (m CD)	6.52	6.42	Dry								

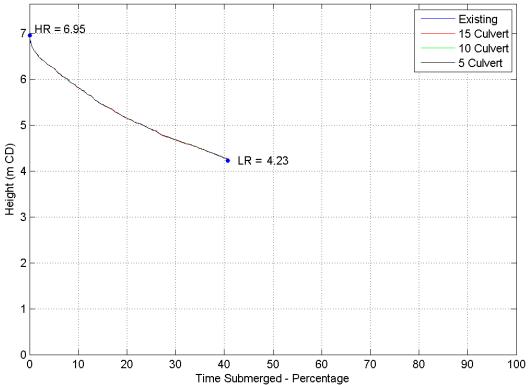




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Submergence Curve -	 Representing Expected 	a i ide Heidnis Based o	n woael Ouldul -	POINLIZZI.ZIDI
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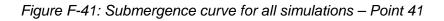
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry
15 Culvert WL (m CD)	6.62	6.47	6.24	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry
10 Culvert WL (m CD)	6.62	6.47	6.24	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry
5 Culvert WL (m CD)	6.62	6.47	6.24	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry

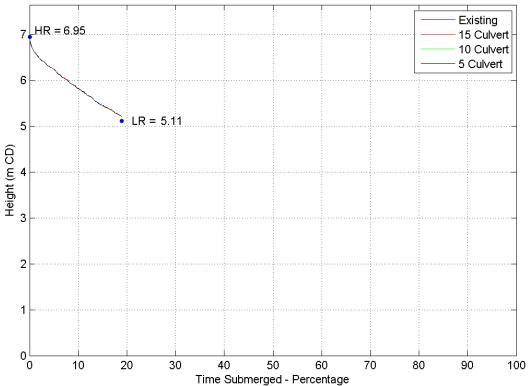




Cubmorgenee Cun/o	Depresenting Evester	d Tida I laiabta Daaad a	n Madal Outnut	Deint [222.246]
Submergence Curve -	 Representing Expected 	a i ide Heidnis Based o	n woael Ouldul -	
				[,]

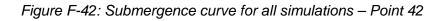
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry
15 Culvert WL (m CD)	6.62	6.47	6.24	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry
10 Culvert WL (m CD)	6.62	6.47	6.24	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry
5 Culvert WL (m CD)	6.62	6.47	6.24	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry

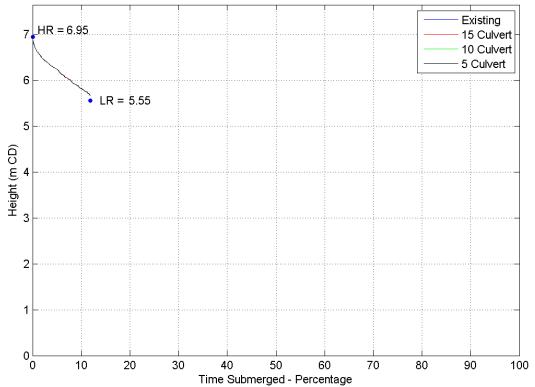




Submergence Curve	 Representing Expected 	ed Tide Heights Based c	n Model Output .	Point [223 218]
Submicigence Surve	Representing Expeets	a machioigints busca a		1 0111 [220,210]

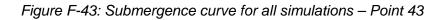
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	Dry						
15 Culvert WL (m CD)	6.62	6.47	6.24	5.82	Dry						
10 Culvert WL (m CD)	6.62	6.47	6.24	5.82	Dry						
5 Culvert WL (m CD)	6.62	6.47	6.24	5.82	Dry						

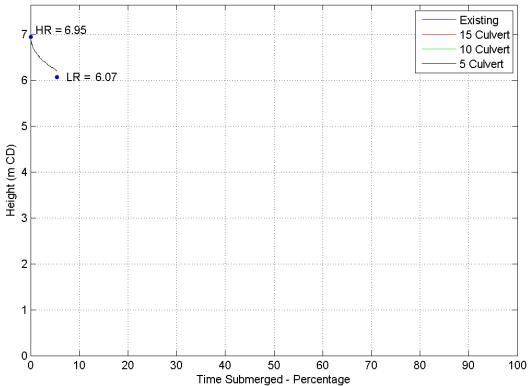




Submergence Curve -	Representing P	Evnected Tide	Heights Based or	n Model Output .	- Point [225 219]
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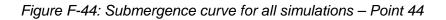
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	5.82	Dry						
15 Culvert WL (m CD)	6.62	6.47	6.24	5.82	Dry						
10 Culvert WL (m CD)	6.62	6.47	6.24	5.82	Dry						
5 Culvert WL (m CD)	6.62	6.47	6.24	5.82	Dry						

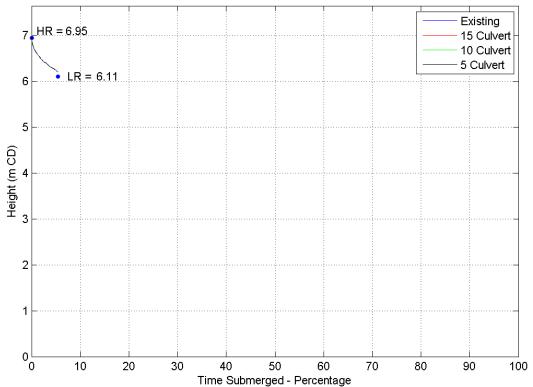




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Sunmergence Cun/e -	- Representing Expected	n Lide Heidnis Kased or	NINDREI CHITCHT - POINT 1778 7700

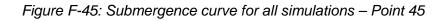
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	Dry							
15 Culvert WL (m CD)	6.62	6.47	6.23	Dry							
10 Culvert WL (m CD)	6.62	6.47	6.23	Dry							
5 Culvert WL (m CD)	6.62	6.47	6.24	Dry							

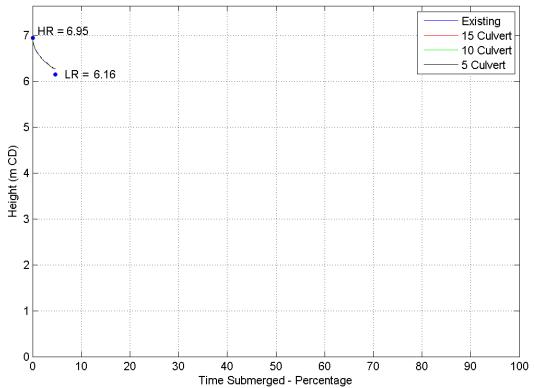




	Demonstration of Error sector	d Tide Heights Based o	March Order March	D - :-+ (000 000)
Sunmergence Cun/e	- Representing Expecte	a lide Heidnis Kased o	n iviodel Cilitolit.	

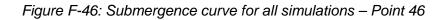
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	6.23	Dry							
15 Culvert WL (m CD)	6.62	6.47	6.23	Dry							
10 Culvert WL (m CD)	6.62	6.47	6.23	Dry							
5 Culvert WL (m CD)	6.62	6.47	6.23	Dry							

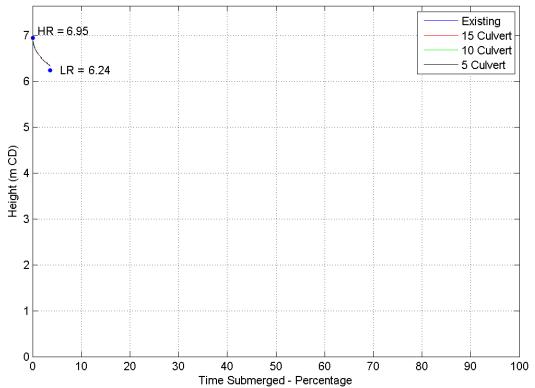




	Demonstration of Examples	d Tide Heights Based or	- Maralal Ordered	D - :-+ (000 000)
Sunmergence Curve	- Representing Expecte	a lude Heidnis Based of	n iviodel Olithilt -	- Point 1232 2201
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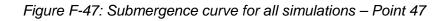
Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	Dry								
15 Culvert WL (m CD)	6.62	6.47	Dry								
10 Culvert WL (m CD)	6.62	6.47	Dry								
5 Culvert WL (m CD)	6.62	6.47	Dry								

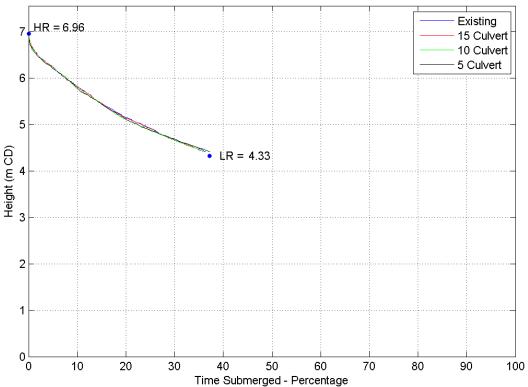




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Submergence Curve - Representing Expected Tide Heights Based on Model Output - Point [23-	. 7711
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Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.62	6.47	Dry								
15 Culvert WL (m CD)	6.62	6.47	Dry								
10 Culvert WL (m CD)	6.62	6.47	Dry								
5 Culvert WL (m CD)	6.62	6.47	Dry								





Cubmorgenee Cunic	Depresenting Expected	I Tida I laiabta Daaad a		Daint [224 400]
Submergence Curve -	 Representing Expected 	I FIGE HEIGHLS Based O	n Model Ouldul -	- POINLIZ34, 1091

Time Submerged (%)	1	2	5	10	25	50	75	90	95	98	99
Existing WL (m CD)	6.63	6.47	6.23	5.82	4.92	Dry	Dry	Dry	Dry	Dry	Dry
15 Culvert WL (m CD)	6.63	6.47	6.23	5.82	4.91	Dry	Dry	Dry	Dry	Dry	Dry
10 Culvert WL (m CD)	6.61	6.45	6.22	5.78	4.87	Dry	Dry	Dry	Dry	Dry	Dry
5 Culvert WL (m CD)	6.58	6.46	6.2	5.79	4.88	Dry	Dry	Dry	Dry	Dry	Dry

Figure F-48: Submergence curve for all simulations – Point 48